VOID FOR BIDDING

DO NOT UNSTAPLE THIS BOOKLET....ENTER ALL REQUIRED INFORMATION
------------------------    EITHER BY HAND OR BY STAMP.

DATE OF OPENING : March 19, 2010            CALL ORDER : 001

CONTRACT ID : B13668-10-000-0

PCN             PROJECTS AND CONTRACT NO.
-------------      ------------------------------
0006043.01000      CSNHS-0006-00(043)

COUNTY : CHEROKEE

CODE__________ ISSUED TO:

1. __________________________________________________________________
2. __________________________________________________________________
3. __________________________________________________________________

PART 1 OF 2
THE CONTRACTOR SHALL RETURN ALL PARTS OF THE PROPOSAL
WITH HIS BID UNLESS BIDDING ELECTRONICALLY
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

CONTRACT ID : B13668-10-000-0  
DESIGN BUILD PROJECT CONSISTING OF CONSTRUCTION OF AN  
INTERCHANGE ON I-575/SR 417 AT RIDGEWALK PKWY.  
(FOS)  

PROPOSAL GUARANTY : 5%  

DBE GOAL : 12.00 %  

<table>
<thead>
<tr>
<th>SITE</th>
<th>COMPLETION DATE</th>
<th>CONTRACT TIME</th>
<th>LIQUIDATED DAMAGES</th>
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<tr>
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<td>12/31/12</td>
<td>COMPLETION DATE</td>
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<td>01</td>
<td>AVAILABLE DAYS</td>
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<td>02</td>
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<td>90 CALENDAR DAYS</td>
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<td>04</td>
<td>160 CALENDAR DAYS</td>
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<td>05</td>
<td>270 CALENDAR DAYS</td>
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NOTICE TO BIDDERS  
If a DBE goal is specified, the bidder shall submit with this bid proposal a list of all proposed DBE participants. A form for this purpose is provided in this proposal. Please refer to the following specifications:  

102.07 Rejection of Proposals  
Disadvantaged Business Enterprise Program (Special Provision)  

BIDDERS SHALL ENTER ALL UNIT PRICES, MAKE ALL EXTENSIONS AND TOTAL THE BID.
<table>
<thead>
<tr>
<th>LINE NO</th>
<th>ITEM DESCRIPTION</th>
<th>APPROX. QUANTITY</th>
<th>UNIT PRICE AND UNITS</th>
<th>BID AMOUNT AND UNITS</th>
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<tbody>
<tr>
<td>150-9011 TRAFFIC CONTROL</td>
<td>- WORKZONE LAW ENFORCEMENT (CONTRACTOR BIDS)</td>
<td>2,500.000 HR</td>
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<tr>
<td>158-1000 TRAINING HOURS</td>
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<td>10,000.000 HR</td>
<td>0.80000 DOLLARS</td>
<td>8,000.00 DOLLARS</td>
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<td>999-2010 DESIGN COMPLETE</td>
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<td>LUMP</td>
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<tr>
<td>999-2015 CONSTRUCTION</td>
<td>COMPLETE</td>
<td>LUMP</td>
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SECTION 0001 ROADWAY

SECTION 0001 TOTAL

ENTER BID TOTAL ON NEXT PAGE

TOTAL BID

VOID FOR BIDDING
<table>
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<tr>
<th>LINE NO</th>
<th>ITEM DESCRIPTION</th>
<th>APPROX. QUANTITY AND UNITS</th>
<th>UNIT PRICE</th>
<th>BID AMOUNT</th>
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TOTAL BID

VOID FOR BIDDING
**DBE GOALS**

VENDOR ID: ____________________  BIDDER’S COMPANY NAME: ____________________

PROJECT NO. & COUNTY: CSNHS-0006-00(043)  **CHEROKEE**

LET NO: 001  LET DATE: March 19, 2010  TOTAL BID: ____________

THE REQUIRED DBE GOAL ON THIS CONTRACT IS: 12%

I PROPOSE TO UTILIZE THE FOLLOWING DBE’S:

**LIST OF DBE PARTICIPANTS**

<table>
<thead>
<tr>
<th>*VENDOR NUMBER</th>
<th>DBE NAME/ADDRESS (CITY, STATE)</th>
<th>TYPE OF WORK</th>
<th>Race Neutral</th>
<th>Race Conscious</th>
<th>*WORK CODE</th>
<th>AMOUNT</th>
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</table>

*For Departmental use only. Do not fill in Work codes.

PLEASE NOTE: Only 60% of the participation of a DBE Supplier who does not manufacture or install the product will be counted toward the goal. See below for further instructions.
INSTRUCTIONS FOR LIST OF DBE PARTICIPANTS

If a DBE Goal is indicated, you must propose to achieve a goal that is equal or greater than the percentage required. If no goal is indicated, you may propose your own goal.

The DBE Firms to be utilized as counting toward the proposed goal must be listed on this form, along with their addresses, type of work and the amount to be paid to each of the minority firms. The amount entered will not necessarily be the contract amount, but must be the actual amount that will be paid to the DBE firm. In the case of a DBE supplier, the amount paid and 60% of that amount both will be entered; and only the 60% figure should be added to the total. An example of this is shown in the example chart:

<table>
<thead>
<tr>
<th>Vendor Number</th>
<th>Company Name And Address (City and State)</th>
<th>Type of Work</th>
<th>* Work Code</th>
<th>Race Neutral</th>
<th>Race Conscious</th>
<th>Amount</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>ABC Oil Company Atlanta, GA</td>
<td>Diesel Fuel Supplier</td>
<td></td>
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<td></td>
<td>$80,000.00</td>
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<tr>
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<td>(60% = $48,000.00)</td>
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</tbody>
</table>

* For Departmental use ONLY. Do not fill in Work Codes.

The Contractor shall indicate for each DBE and Type of Work whether the DBE Participant is Race Neutral or Race Conscious by placing a checkmark in the appropriate column.

PLEASE NOTE: For 60% of the amount paid to a DBE supplier to be eligible to count toward fulfilling the DBE goal, the supplier must be an established “regular dealer” in the product involved, and not just a broker. A “regular dealer” would normally sell the product to several customers and would usually have product inventory on hand.
This information shall be submitted in accordance with Specification Section 102.16

| Prime Contractor/Consultant: | ____________________________ |
| Address/Telephone Number: | ____________________________ |
| Bid/Proposal Number: | ____________________________ |
| Quote Submitted MM/YY: | ____________________________ |

49 CRF Part 26.11 requires the Georgia Department of Transportation to develop and maintain a “bid opportunity list”. The list is intended to be a listing of all firms participating or attempting to participate, on DOT assisted contracts. The list must include all firms that bid on prime contracts, or bid or quote subcontracts and materials supplies on DOT-assisted projects, including both DBEs and non-DBEs. For consulting companies this list must include all subconsultants contacting you and expressing an interest in teaming with you on a specific DOT assisted project. Prime contractors and consultants must provide information for Nos. 1, 2, 3, and 4 and must provide information they have available on Numbers 5, 5.A, 6, 7, 8 and 9 for themselves, and their subcontractors and subconsultants.

1. Federal Tax ID Number: ____________________________ 6. ☐ DBE
2. Firm Name: ____________________________ ☐ Non-DBE
3. Phone: ____________________________ 7. ☐ Subcontractor
4. Address: ____________________________ 8. ☐ Subconsultant
5. Contact ______________ 9. ☐ Supplier
5.A. Company E mail address ____________________________

1. Federal Tax ID Number: ____________________________ 6. ☐ DBE
2. Firm Name: ____________________________ ☐ Non-DBE
3. Phone: ____________________________ 7. ☐ Subcontractor
4. Address: ____________________________ 8. ☐ Subconsultant
5. Contact ______________ 9. ☐ Supplier
5.A. Company E mail address ____________________________

1. Federal Tax ID Number: ____________________________ 6. ☐ DBE
2. Firm Name: ____________________________ ☐ Non-DBE
3. Phone: ____________________________ 7. ☐ Subcontractor
4. Address: ____________________________ 8. ☐ Subconsultant
5. Contact ______________ 9. ☐ Supplier
5.A. Company E mail address ____________________________
PRIME CONTRACTOR’S WORK AUTHORIZATION CERTIFICATION

Pursuant to O.C.G.A. § 13-10-91, all qualifying contractors and sub-contractors performing work within the State of Georgia on a contract with a public employer must register and participate in a federal work authorization program. Prime contractors may participate in any of the electronic verification of work authorization programs operated by the United States Department of Homeland Security or any equivalent federal work authorization program operated by the United States Department of Homeland Security to verify information of newly hired employees, pursuant to the Immigration Reform and Control Act of 1986 (“IRCA”).

The date by which a prime contractor must register and participate in a qualifying federal work authorization program depends on the number of employees in the prime contractor’s company. If the prime contractor’s company has 500 or more employees, it is required to register and participate in a qualifying federal work authorization program by July 1, 2007. If the prime contractor’s company has 100 or more employees, it is required to register for and participate in a qualifying federal work authorization program by July 1, 2008. If the prime contractor’s company has 99 employees or fewer, it is required to register for and participate in a qualifying federal work authorization program by July 1, 2009.

Certify compliance with O.C.G.A. § 13-10-91 by checking the appropriate line below:

_____ The undersigned has registered for and is participating in a qualifying federal work authorization program;

The undersigned further agrees that, should it employ or contract with any subcontractor(s) in connection with the physical performance of services within this state pursuant to this contract with a public employer, the undersigned will secure from such subcontractor(s) a verification of compliance with O.C.G.A. § 13-10-91 using the form “Subcontractor’s Work Authorization Certification” or a substantially similar form. The undersigned will maintain records of compliance and provide a copy of each subcontractor’s verification to the public employer at the time the sub-contractor is retained to perform such service.

[SIGNATURE ON NEXT PAGE]
BY: Authorized Officer or Agent

Title of Authorized Officer or Agent

Printed Name of Authorized Officer or Agent

With express authority on behalf of:

Printed Name of Prime Contractor

SUBSCRIBED AND SWORN BEFORE ME ON THIS THE ______ DAY OF ________, 20__. 

Notary Public

My Commission Expires: ________
Failure to complete appropriate certification requirements identified below or submission of a false certification shall render the bid non-responsive.

EQUAL EMPLOYMENT OPPORTUNITY

I further certify that I have ___/have not ___ participated in a previous contract or subcontract subject to the equal opportunity clause, as required by Executive Orders 10925, 11114, or 11246, and that I have ___/have not___ filed with the Joint Reporting Committee, the Director of the Office of Federal Contract Compliance, a Federal Government contracting or administering agency, or the former President's Committee on Equal Employment Opportunity, all reports due under the applicable filing requirements.

I understand that if I have participated in a previous Contract or Subcontract subject to the Executive Orders above and have not filed the required reports that 41 CFR 60-1.7(b)(1) prevents the award of this Contract unless I submit a report governing the delinquent period or such other period specified by the Federal Highway Administration or by the Director, Office of Federal Contract Compliance, U. S. Department of Labor.

Reports and notifications required under 41 CFR 604, including reporting subcontract awards in excess of $10,000.00 should be addressed to:

Ms. Carol Gaudin
Regional Director, U. S. Department of Labor
Office of Federal Contract Compliance Programs, Region 4
Rm. 7B75
61 Forsyth St. SW
Atlanta GA 30303

EXAMINATION OF PLANS AND SPECIFICATIONS

I acknowledge that this Project will be constructed in English units.

I certify that I have carefully examined the Plans for this Project and the Standard Specifications, 2001 Edition, and the Supplemental Specifications and Special Provisions included in and made a part of this Proposal, and have also personally examined the site of the work. On the basis of the said Specifications and Plans, I propose to furnish all necessary machinery, tools, apparatus and other means of construction, and do all the work and furnish all the materials in the manner specified.

I understand the quantities mentioned are approximate only and are subject to either increase or decrease and hereby propose to perform any increased or decreased quantities of work or extra work on the basis provided for in the Specifications.
I also hereby agree that the State, or the Department of Transportation, would suffer damages in a sum equal to at least the amount of the enclosed Proposal Guaranty, in the event my Proposal should be accepted and a Contract tendered me thereunder and I should refuse to execute same and furnish bond as herein required, in consideration of which I hereby agree that, in the event of such failure on my part to execute said Contract and furnish bond within fifteen (15) days after the date of the letter transmitting the Contract to me, the amount of said Proposal Guaranty shall be and is hereby, forfeited to the State, or to the Department of Transportation, as liquidated damages as the result of such failure on my part.

I further propose to execute the Contract agreement described in the Specifications as soon as the work is awarded to me, and to begin and complete the work within the time limit provided. I also propose to furnish a Contract Bond, approved by the State Transportation Board, as required by the laws of the State of Georgia. This bond shall not only serve to guarantee the completion of the work on my part, but also to guarantee the excellence of both workmanship and materials until the work is finally accepted, as well as to fully comply with all the laws of the State of Georgia.

**CONFLICT OF INTEREST**

By signing and submitting this Contract I hereby certify that employees of this company or employee of any company supplying material or subcontracting to do work on this Contract will not engage in business ventures with employees of the Georgia Department of Transportation (GA D.O.T.) nor shall they provide gifts, gratuities, favors, entertainment, loans or other items of value to employees of this department.

Also, by signing and submitting this Contract I hereby certify that I will notify the Georgia Department of Transportation through its District Engineer of any business ventures entered into between employees of this company or employees of any company supplying material or subcontracting to do work on this Contract with a family member of GA D.O.T. employees.

**DRUG FREE WORKPLACE**

The undersigned certifies that the provisions of Code Sections 50-24-1 through 50-24-6 of the Official Code of Georgia Annotated, relating to the "Drug-free Workplace Act", have been complied with in full. The undersigned further certifies that:

1. A drug-free workplace will be provided for the Contractor's employees during the performance of the Contract; and
2. Each Contractor who hires a Subcontractor to work in a drug-free workplace shall secure from that Subcontractor the following written certification:

   "As part of the subcontracting agreement with (Contractor's name) , (Subcontractor's name) certifies to the Contractor that a drug free workplace will be provided for the Subcontractor's employees during the performance of this Contract pursuant to paragraph (7) of subsection (b) of Code Section 50-24-3."

Also, the undersigned further certifies that he will not engage in the unlawful manufacture, sale distribution, dispensation, possession, or use of a controlled substance or marijuana during the performance of the Contract.
NON-COLLUSION CERTIFICATION

I hereby certify that I have not, nor has any member of the firm(s) or corporation(s), either directly or indirectly entered into any agreement, participated in any collusion, nor otherwise taken any action in restraint of free competitive bidding in connection with this submitted bid.

It is understood and agreed that this Proposal is one of several competitive bids made to the Department of Transportation, and in consideration of mutual agreements of the bidders, similar hereto, and in consideration of the sum of One Dollar cash in hand paid, receipt whereof is hereby acknowledged, the undersigned agrees that this Proposal shall be an option, which is hereby given by the undersigned to the Department of Transportation to accept or reject this Proposal at any time within thirty (30) calendar days from the date on which this sealed proposal is opened and read, unless a longer period is specified in the Proposal or the successful bidder agrees in writing to a longer period of time for the award, and in consideration of the premises, it is expressly covenanted and agreed that this Proposal is not subject to withdrawal by the Proposer or Bidder, during the term of said option.

I hereby acknowledge receipt of the following checked amendments of the Proposal, Plans, Specifications and/or other documents pertaining to the Contract.

Amendment Nos.: ______ ______ ______ ______ ______. I understand that failure to confirm the receipt of amendments is cause for rejection of bids.

Witness my hand and seal this the _____ day of ______________________, 20______.

The bidder(s) whose signature(s) appear on this document, having personally appeared before me, and being duly sworn, deposes and says that the above statements are true and correct.

Sworn to and subscribed before me this _____ day of ________________, 20______.

__________________________________
(Notary Public)

My Commission expires the ______ day of ________________, 20______.

__________________________________
(Federal ID No./IRS No.)

---

 VOID FOR BIDDING
Listed below are modifications and additions to the 2001 State of Georgia Standard Specifications Constructions of Transportation System and the 2008 Supplemental Specifications modifying the 2001 Standard Specifications.
NOTICE TO ALL BIDDERS

To report bid rigging activities call:

1-800-424-9071

The U.S. Department of Transportation (DOT) operates the above toll-free “hotline” Monday through Friday, 8:00 AM to 5:00 PM, Eastern Time. Anyone with the knowledge of possible bid rigging, bidder collusion, or other fraudulent activities should use the “hotline” to report such activities.

The “hotline” is part of the DOT’s continuing effort to identify and investigate highway construction contract fraud and abuse, and is operated under the direction of the DOT Inspector General. All information will be treated confidentially and caller anonymity will be respected.
Required Contract Provisions Federal-Aid Construction Contracts

I. General
II. Nondiscrimination
III. Nonsegregated Facilities
IV. Payment of Predetermined Minimum Wage
V. Statements and Payrolls
VI. Record of Materials, Supplies, and Labor
VII. Subletting or Assigning the Contract
VIII. Safety: Accident Prevention
IX. False Statements Concerning Highway Projects
X. Implementation of Clean Air Act and Federal Water Pollution Control Act
XI. Certification Regarding Debarment, Suspension Ineligibility, and Voluntary Exclusion
XII. Certification Regarding Use of Contract Funds for Lobbying

Attachments

A. Employment Preference for Appalachian Contracts (included in Appalachian contracts only)

I. GENERAL
1. These contract provisions shall apply to all work performed on the contract by the contractor's own organization and with the assistance of workers under the contractor's immediate superintendence and to all work performed on the contract by piecework, station work, or by subcontract.
2. Except as otherwise provided for in each section, the contractor shall insert in each subcontract all of the stipulations contained in these Required Contract Provisions, and further require their inclusion in any lower tier subcontract or purchase order that may in turn be made. The Required Contract Provisions shall not be incorporated by reference in any case. The prime contractor shall be responsible for compliance by any subcontractor or lower tier subcontractor with these Required Contract Provisions.
3. A breach of any of the stipulations contained in these Required Contract Provisions shall be sufficient grounds for termination of the contract.
4. A breach of the following clauses of the Required Contract Provisions may also be grounds for debarment as provided in 29 CFR 5.12:
   
   Section I, paragraph 2;
   Section IV, paragraphs 1, 2, 3, 4, and 7;
   Section V, paragraphs 1 and 2a through 2g.

5. Disputes arising out of the labor standards provisions of Section IV (except paragraph 5) and Section V of these Required Contract Provisions shall not be subject to the general disputes clause of this contract. Such disputes shall be resolved in accordance with the procedures of the U.S. Department of Labor (DOL) as set forth in 29 CFR 5, 6, and 7. Disputes within the meaning of this clause include disputes between the contractor (or any of its subcontractors) and the contracting agency, the DOL, or the contractor's employees or their representatives.
6. Selection of Labor: During the performance of this contract, the contractor shall not:
a. discriminate against labor from any other State, possession, or territory of the United States (except for employment preference for Appalachian contracts, when applicable, as specified in Attachment A), or

b. employ convict labor for any purpose within the limits of the project unless it is labor performed by convicts who are on parole, supervised release, or probation.

II. NONDISCRIMINATION

(Applicable to all Federal-aid construction contracts and to all related subcontracts of $10,000 or more.)

1. Equal Employment Opportunity: Equal employment opportunity (EEO) requirements not to discriminate and to take affirmative action to assure equal opportunity as set forth under laws, executive orders, rules, regulations (28 CFR 35, 29 CFR 1630 and 41 CFR 60) and orders of the Secretary of Labor as modified by the provisions prescribed herein, and imposed pursuant to 23 U.S.C. 140 shall constitute the EEO and specific affirmative action standards for the contractor's project activities under this contract. The Equal Opportunity Construction Contract Specifications set forth under 41 CFR 60-4.3 and the provisions of the American Disabilities Act of 1990 (42 U.S.C. 12101 et seq.) set forth under 28 CFR 35 and 29 CFR 1630 are incorporated by reference in this contract. In the execution of this contract, the contractor agrees to comply with the following minimum specific requirement activities of EEO:

a. The contractor will work with the State highway agency (SHA) and the Federal Government in carrying out EEO obligations and in their review of his/her activities under the contract.

b. The contractor will accept as his operating policy the following statement:

"It is the policy of this Company to assure that applicants are employed, and that employees are treated during employment, without regard to their race, religion, sex, color, national origin, age or disability. Such action shall include: employment, upgrading, demotion, or transfer; recruitment or recruitment advertising; layoff or termination; rates of pay or other forms of compensation; and selection for training, including apprenticeship, pre-apprenticeship, and/or on-the-job training."

2. EEO Officer: The contractor will designate and make known to the SHA contracting officers an EEO Officer who will have the responsibility for and must be capable of effectively administering and promoting an active contractor program of EEO and who must be assigned adequate authority and responsibility to do so.

3. Dissemination of Policy: All members of the contractor's staff who are authorized to hire, supervise, promote, and discharge employees, or who recommend such action, or who are substantially involved in such action, will be made fully cognizant of, and will implement, the contractor's EEO policy and contractual responsibilities to provide EEO in each grade and classification of employment. To ensure that the above agreement will be met, the following actions will be taken as a minimum:

a. Periodic meetings of supervisory and personnel office employees will be conducted before the start of work and then not less often than once
every six months, at which time the contractor's EEO policy and its implementation will be reviewed and explained. The meetings will be conducted by the EEO Officer.

b. All new supervisory or personnel office employees will be given a thorough indoctrination by the EEO Officer, covering all major aspects of the contractor's EEO obligations within thirty days following their reporting for duty with the contractor.

c. All personnel who are engaged in direct recruitment for the project will be instructed by the EEO Officer in the contractor's procedures for locating and hiring minority group employees.

d. Notices and posters setting forth the contractor's EEO policy will be placed in areas readily accessible to employees, applicants for employment and potential employees.

e. The contractor's EEO policy and the procedures to implement such policy will be brought to the attention of employees by means of meetings, employee handbooks, or other appropriate means.

4. **Recruitment:** When advertising for employees, the contractor will include in all advertisements for employees the notation: "An Equal Opportunity Employer." All such advertisements will be placed in publications having a large circulation among minority groups in the area from which the project work force would normally be derived.

   a. The contractor will, unless precluded by a valid bargaining agreement, conduct systematic and direct recruitment through public and private employee referral sources likely to yield qualified minority group applicants. To meet this requirement, the contractor will identify sources of potential minority group employees, and establish with such identified sources procedures whereby minority group applicants may be referred to the contractor for employment consideration.

   b. In the event the contractor has a valid bargaining agreement providing for exclusive hiring hall referrals, he is expected to observe the provisions of that agreement to the extent that the system permits the contractor's compliance with EEO contract provisions. (The DOL has held that where implementation of such agreements have the effect of discriminating against minorities or women, or obligates the contractor to do the same, such implementation violates Executive Order 11246, as amended.)

   c. The contractor will encourage his present employees to refer minority group applicants for employment. Information and procedures with regard to referring minority group applicants will be discussed with employees.

5. **Personnel Actions:** Wages, working conditions, and employee benefits shall be established and administered, and personnel actions of every type, including hiring, upgrading, promotion, transfer, demotion, layoff, and termination, shall be taken without regard to race, color, religion, sex, national origin, age or disability. The following procedures shall be followed:

   a. The contractor will conduct periodic inspections of project sites to insure that working conditions and employee facilities do not indicate discriminatory treatment of project site personnel.

   b. The contractor will periodically evaluate the spread of wages paid within each classification to determine any evidence of discriminatory wage practices.
c. The contractor will periodically review selected personnel actions in depth to determine whether there is evidence of discrimination. Where evidence is found, the contractor will promptly take corrective action. If the review indicates that the discrimination may extend beyond the actions reviewed, such corrective action shall include all affected persons.

d. The contractor will promptly investigate all complaints of alleged discrimination made to the contractor in connection with his obligations under this contract, will attempt to resolve such complaints, and will take appropriate corrective action within a reasonable time. If the investigation indicates that the discrimination may affect persons other than the complainant, such corrective action shall include such other persons. Upon completion of each investigation, the contractor will inform every complainant of all of his avenues of appeal.

6. **Training and Promotion:**
   a. The contractor will assist in locating, qualifying, and increasing the skills of minority group and women employees, and applicants for employment.
   
   b. Consistent with the contractor's work force requirements and as permissible under Federal and State regulations, the contractor shall make full use of training programs, i.e., apprenticeship, and on-the-job training programs for the geographical area of contract performance. Where feasible, 25 percent of apprentices or trainees in each occupation shall be in their first year of apprenticeship or training. In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision.
   
   c. The contractor will advise employees and applicants for employment of available training programs and entrance requirements for each.
   
   d. The contractor will periodically review the training and promotion potential of minority group and women employees and will encourage eligible employees to apply for such training and promotion.

7. **Unions:** If the contractor relies in whole or in part upon unions as a source of employees, the contractor will use his/her best efforts to obtain the cooperation of such unions to increase opportunities for minority groups and women within the unions, and to effect referrals by such unions of minority and female employees. Actions by the contractor either directly or through a contractor's association acting as agent will include the procedures set forth below:
   
   a. The contractor will use best efforts to develop, in cooperation with the unions, joint training programs aimed toward qualifying more minority group members and women for membership in the unions and increasing the skills of minority group employees and women so that they may qualify for higher paying employment.
   
   b. The contractor will use best efforts to incorporate an EEO clause into each union agreement to the end that such union will be contractually bound to refer applicants without regard to their race, color, religion, sex, national origin, age or disability.
   
   c. The contractor is to obtain information as to the referral practices and policies of the labor union except that to the extent such information is within the exclusive possession of the labor union and such labor union refuses to furnish such information to the contractor, the contractor shall so certify to the SHA and shall set forth what efforts have been made to obtain such information.
d. In the event the union is unable to provide the contractor with a 
reasonable flow of minority and women referrals within the time limit set 
forth in the collective bargaining agreement, the contractor will, through 
independent recruitment efforts, fill the employment vacancies without 
regard to race, color, religion, sex, national origin, age or disability; 
making full efforts to obtain qualified and/or qualifiable minority group 
persons and women. (The DOL has held that it shall be no excuse that 
the union with which the contractor has a collective bargaining 
agreement providing for exclusive referral failed to refer minority 
employees.) In the event the union referral practice prevents the 
contractor from meeting the obligations pursuant to Executive Order 
11246, as amended, and these special provisions, such contractor shall 
immediately notify the SHA.

8. **Selection of Subcontractors, Procurement of Materials and Leasing of 
Equipment:** The contractor shall not discriminate on the grounds of race, color, 
religion, sex, national origin, age or disability in the selection and retention of 
subcontractors, including procurement of materials and leases of equipment.

a. The contractor shall notify all potential subcontractors and suppliers of 
his/her EEO obligations under this contract.

b. Disadvantaged business enterprises (DBE), as defined in 49 CFR 23, 
shall have equal opportunity to compete for and perform subcontracts 
which the contractor enters into pursuant to this contract. The contractor 
will use his best efforts to solicit bids from and to utilize DBE 
subcontractors or subcontractors with meaningful minority group and 
female representation among their employees. Contractors shall obtain 
lists of DBE construction firms from SHA personnel.

c. The contractor will use his best efforts to ensure subcontractor 
compliance with their EEO obligations.

9. **Records and Reports:** The contractor shall keep such records as necessary to 
document compliance with the EEO requirements. Such records shall be 
retained for a period of three years following completion of the contract work and 
shall be available at reasonable times and places for inspection by authorized 
representatives of the SHA and the FHWA.

a. The records kept by the contractor shall document the following:

   1. The number of minority and non-minority group members and 
      women employed in each work classification on the project;
   2. The progress and efforts being made in cooperation with unions, 
      when applicable, to increase employment opportunities for 
      minorities and women;
   3. The progress and efforts being made in locating, hiring, training, 
      qualifying, and upgrading minority and female employees; and
   4. The progress and efforts being made in securing the services of 
      DBE subcontractors or subcontractors with meaningful minority 
      and female representation among their employees.

b. The contractors will submit an annual report to the SHA each July for the 
duration of the project, indicating the number of minority, women, and 
non-minority group employees currently engaged in each work 
classification required by the contract work. This information is to be 
reported on Form FHWA-1391. If on-the-job training is being required by 
special provision, the contractor will be required to collect and report 
training data.
III. NONSEGREGATED FACILITIES

(Applicable to all Federal-aid construction contracts and to all related subcontracts of $10,000 or more.)

a. By submission of this bid, the execution of this contract or subcontract, or the consummation of this material supply agreement or purchase order, as appropriate, the bidder, Federal-aid construction contractor, subcontractor, material supplier, or vendor, as appropriate, certifies that the firm does not maintain or provide for its employees any segregated facilities at any of its establishments, and that the firm does not permit its employees to perform their services at any location, under its control, where segregated facilities are maintained. The firm agrees that a breach of this certification is a violation of the EEO provisions of this contract. The firm further certifies that no employee will be denied access to adequate facilities on the basis of sex or disability.

b. As used in this certification, the term "segregated facilities" means any waiting rooms, work areas, restrooms and washrooms, restaurants and other eating areas, timeclocks, locker rooms, and other storage or dressing areas, parking lots, drinking fountains, recreation or entertainment areas, transportation, and housing facilities provided for employees which are segregated by explicit directive, or are, in fact, segregated on the basis of race, color, religion, national origin, age or disability, because of habit, local custom, or otherwise. The only exception will be for the disabled when the demands for accessibility override (e.g. disabled parking).

c. The contractor agrees that it has obtained or will obtain identical certification from proposed subcontractors or material suppliers prior to award of subcontracts or consummation of material supply agreements of $10,000 or more and that it will retain such certifications in its files.

IV. PAYMENT OF PREDETERMINED MINIMUM WAGE

(Applicable to all Federal-aid construction contracts exceeding $2,000 and to all related subcontracts, except for projects located on roadways classified as local roads or rural minor collectors, which are exempt.)

1. General:

   a. All mechanics and laborers employed or working upon the site of the work will be paid unconditionally and not less often than once a week and without subsequent deduction or rebate on any account [except such payroll deductions as are permitted by regulations (29 CFR 3) issued by the Secretary of Labor under the Copeland Act (40 U.S.C. 276c)] the full amounts of wages and bona fide fringe benefits (or cash equivalents thereof) due at time of payment. The payment shall be computed at wage rates not less than those contained in the wage determination of the Secretary of Labor (hereinafter "the wage determination") which is attached hereto and made a part hereof, regardless of any contractual relationship which may be alleged to exist between the contractor or its subcontractors and such laborers and mechanics.
The wage determination (including any additional classifications and wage rates conformed under paragraph 2 of this Section IV and the DOL poster (WH-1321) or Form FHWA-1495) shall be posted at all times by the contractor and its subcontractors at the site of the work in a prominent and accessible place where it can be easily seen by the workers. For the purpose of this Section, contributions made or costs reasonably anticipated for bona fide fringe benefits under Section 1(b)(2) of the Davis-Bacon Act (40 U.S.C. 276a) on behalf of laborers or mechanics are considered wages paid to such laborers or mechanics, subject to the provisions of Section IV, paragraph 3b, hereof. Also, for the purpose of this Section, regular contributions made or costs incurred for more than a weekly period (but not less often than quarterly) under plans, funds, or programs, which cover the particular weekly period, are deemed to be constructively made or incurred during such weekly period. Such laborers and mechanics shall be paid the appropriate wage rate and fringe benefits on the wage determination for the classification of work actually performed, without regard to skill, except as provided in paragraphs 4 and 5 of this Section IV.

b. Laborers or mechanics performing work in more than one classification may be compensated at the rate specified for each classification for the time actually worked therein, provided, that the employer's payroll records accurately set forth the time spent in each classification in which work is performed.

c. All rulings and interpretations of the Davis-Bacon Act and related acts contained in 29 CFR 1, 3, and 5 are herein incorporated by reference in this contract.

2. Classification:

a. The SHA contracting officer shall require that any class of laborers or mechanics employed under the contract, which is not listed in the wage determination, shall be classified in conformance with the wage determination.

b. The contracting officer shall approve an additional classification, wage rate and fringe benefits only when the following criteria have been met:

1. the work to be performed by the additional classification requested is not performed by a classification in the wage determination;
2. the additional classification is utilized in the area by the construction industry;
3. the proposed wage rate, including any bona fide fringe benefits, bears a reasonable relationship to the wage rates contained in the wage determination; and
4. with respect to helpers, when such a classification prevails in the area in which the work is performed.

c. If the contractor or subcontractors, as appropriate, the laborers and mechanics (if known) to be employed in the additional classification or
their representatives, and the contracting officer agree on the classification and wage rate (including the amount designated for fringe benefits where appropriate), a report of the action taken shall be sent by the contracting officer to the DOL, Administrator of the Wage and Hour Division, Employment Standards Administration, Washington, D.C. 20210. The Wage and Hour Administrator, or an authorized representative, will approve, modify, or disapprove every additional classification action within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

d. In the event the contractor or subcontractors, as appropriate, the laborers or mechanics to be employed in the additional classification or their representatives, and the contracting officer do not agree on the proposed classification and wage rate (including the amount designated for fringe benefits, where appropriate), the contracting officer shall refer the questions, including the views of all interested parties and the recommendation of the contracting officer, to the Wage and Hour Administrator for determination. Said Administrator, or an authorized representative, will issue a determination within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

e. The wage rate (including fringe benefits where appropriate) determined pursuant to paragraph 2c or 2d of this Section IV shall be paid to all workers performing work in the additional classification from the first day on which work is performed in the classification.

3. Payment of Fringe Benefits:
   a. Whenever the minimum wage rate prescribed in the contract for a class of laborers or mechanics includes a fringe benefit which is not expressed as an hourly rate, the contractor or subcontractors, as appropriate, shall either pay the benefit as stated in the wage determination or shall pay another bona fide fringe benefit or an hourly case equivalent thereof.
   b. If the contractor or subcontractor, as appropriate, does not make payments to a trustee or other third person, he/she may consider as a part of the wages of any laborer or mechanic the amount of any costs reasonably anticipated in providing bona fide fringe benefits under a plan or program, provided, that the Secretary of Labor has found, upon the written request of the contractor, that the applicable standards of the Davis-Bacon Act have been met. The Secretary of Labor may require the contractor to set aside in a separate account assets for the meeting of obligations under the plan or program.

4. Apprentices and Trainees (Programs of the U.S. DOL) and Helpers:
   a. Apprentices:
      A. Apprentices will be permitted to work at less than the predetermined rate for the work they performed when they are employed pursuant to and individually registered in a bona fide apprenticeship program registered with the DOL, Employment and Training Administration, Bureau of Apprenticeship and Training, or with a State apprenticeship agency recognized by the Bureau, or if a person is employed in his/her first 90 days of probationary employment as an apprentice in such an apprenticeship program, who is not individually registered in the
program, but who has been certified by the Bureau of Apprenticeship and Training or a State apprenticeship agency (where appropriate) to be eligible for probationary employment as an apprentice.

B. The allowable ratio of apprentices to journeyman-level employees on the job site in any craft classification shall not be greater than the ratio permitted to the contractor as to the entire work force under the registered program. Any employee listed on a payroll at an apprentice wage rate, who is not registered or otherwise employed as stated above, shall be paid not less than the applicable wage rate listed in the wage determination for the classification of work actually performed. In addition, any apprentice performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed. Where a contractor or subcontractor is performing construction on a project in a locality other than that in which its program is registered, the ratios and wage rates (expressed in percentages of the journeyman-level hourly rate) specified in the contractor's or subcontractor's registered program shall be observed.

C. Every apprentice must be paid at not less than the rate specified in the registered program for the apprentice's level of progress, expressed as a percentage of the journeyman-level hourly rate specified in the applicable wage determination. Apprentices shall be paid fringe benefits in accordance with the provisions of the apprenticeship program. If the apprenticeship program does not specify fringe benefits, apprentices must be paid the full amount of fringe benefits listed on the wage determination for the applicable classification. If the Administrator for the Wage and Hour Division determines that a different practice prevails for the applicable apprentice classification, fringes shall be paid in accordance with that determination.

D. In the event the Bureau of Apprenticeship and Training, or a State apprenticeship agency recognized by the Bureau, withdraws approval of an apprenticeship program, the contractor or subcontractor will no longer be permitted to utilize apprentices at less than the applicable predetermined rate for the comparable work performed by regular employees until an acceptable program is approved.

b. Trainees:

A. Except as provided in 29 CFR 5.16, trainees will not be permitted to work at less than the predetermined rate for the work performed unless they are employed pursuant to and individually registered in a program which has received prior approval, evidenced by formal certification by the DOL, Employment and Training Administration.

B. The ratio of trainees to journeyman-level employees on the job site shall not be greater than permitted under the plan approved by the Employment and Training Administration. Any employee listed on the payroll at a trainee rate who is not registered and participating in a training plan approved by the Employment and
Training Administration shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any trainee performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed.

C. Every trainee must be paid at not less than the rate specified in the approved program for his/her level of progress, expressed as a percentage of the journeyman-level hourly rate specified in the applicable wage determination. Trainees shall be paid fringe benefits in accordance with the provisions of the trainee program. If the trainee program does not mention fringe benefits, trainees shall be paid the full amount of fringe benefits listed on the wage determination unless the Administrator of the Wage and Hour Division determines that there is an apprenticeship program associated with the corresponding journeyman-level wage rate on the wage determination which provides for less than full fringe benefits for apprentices, in which case such trainees shall receive the same fringe benefits as apprentices.

D. In the event the Employment and Training Administration withdraws approval of a training program, the contractor or subcontractor will no longer be permitted to utilize trainees at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

c. Helpers:

Helpers will be permitted to work on a project if the helper classification is specified and defined on the applicable wage determination or is approved pursuant to the conformance procedure set forth in Section IV.2. Any worker listed on a payroll at a helper wage rate, who is not a helper under a approved definition, shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed.

5. Apprentices and Trainees (Programs of the U.S. DOT):

Apprentices and trainees working under apprenticeship and skill training programs which have been certified by the Secretary of Transportation as promoting EEO in connection with Federal-aid highway construction programs are not subject to the requirements of paragraph 4 of this Section IV. The straight time hourly wage rates for apprentices and trainees under such programs will be established by the particular programs. The ratio of apprentices and trainees to journeymen shall not be greater than permitted by the terms of the particular program.

6. Withholding:

The SHA shall upon its own action or upon written request of an authorized representative of the DOL withhold, or cause to be withheld, from the contractor or subcontractor under this contract or any other Federal contract with the same
prime contractor, or any other Federally-assisted contract subject to Davis-Bacon prevailing wage requirements which is held by the same prime contractor, as much of the accrued payments or advances as may be considered necessary to pay laborers and mechanics, including apprentices, trainees, and helpers, employed by the contractor or any subcontractor the full amount of wages required by the contract. In the event of failure to pay any laborer or mechanic, including any apprentice, trainee, or helper, employed or working on the site of the work, all or part of the wages required by the contract, the SHA contracting officer may, after written notice to the contractor, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds until such violations have ceased.

7. Overtime Requirements:

No contractor or subcontractor contracting for any part of the contract work which may require or involve the employment of laborers, mechanics, watchmen, or guards (including apprentices, trainees, and helpers described in paragraphs 4 and 5 above) shall require or permit any laborer, mechanic, watchman, or guard in any workweek in which he/she is employed on such work, to work in excess of 40 hours in such workweek unless such laborer, mechanic, watchman, or guard receives compensation at a rate not less than one-and-one-half times his/her basic rate of pay for all hours worked in excess of 40 hours in such workweek.

8. Violation:

Liability for Unpaid Wages; Liquidated Damages: In the event of any violation of the clause set forth in paragraph 7 above, the contractor and any subcontractor responsible thereof shall be liable to the affected employee for his/her unpaid wages. In addition, such contractor and subcontractor shall be liable to the United States (in the case of work done under contract for the District of Columbia or a territory, to such District or to such territory) for liquidated damages. Such liquidated damages shall be computed with respect to each individual laborer, mechanic, watchman, or guard employed in violation of the clause set forth in paragraph 7, in the sum of $10 for each calendar day on which such employee was required or permitted to work in excess of the standard work week of 40 hours without payment of the overtime wages required by the clause set forth in paragraph 7.

9. Withholding for Unpaid Wages and Liquidated Damages:

The SHA shall upon its own action or upon written request of any authorized representative of the DOL withhold, or cause to be withheld, from any monies payable on account of work performed by the contractor or subcontractor under any such contract or any other Federal contract with the same prime contractor, or any other Federally-assisted contract subject to the Contract Work Hours and Safety Standards Act, which is held by the same prime contractor, such sums as may be determined to be necessary to satisfy any liabilities of such contractor or subcontractor for unpaid wages and liquidated damages as provided in the clause set forth in paragraph 8 above.
V. STATEMENTS AND PAYROLLS

(Applicable to all Federal-aid construction contracts exceeding $2,000 and to all related
subcontracts, except for projects located on roadways classified as local roads or rural
collectors, which are exempt.)

1. Compliance with Copeland Regulations (29 CFR 3):

The contractor shall comply with the Copeland Regulations of the Secretary of
Labor which are herein incorporated by reference.

2. Payrolls and Payroll Records:

   a. Payrolls and basic records relating thereto shall be maintained by the
      contractor and each subcontractor during the course of the work and
      preserved for a period of 3 years from the date of completion of the
      contract for all laborers, mechanics, apprentices, trainees, watchmen,
      helpers, and guards working at the site of the work.

   b. The payroll records shall contain the name, social security number, and
      address of each such employee; his or her correct classification; hourly
      rates of wages paid (including rates of contributions or costs anticipated
      for bona fide fringe benefits or cash equivalent thereof the types
      described in Section 1(b)(2)(B) of the Davis Bacon Act); daily and weekly
      number of hours worked; deductions made; and actual wages paid. In
      addition, for Appalachian contracts, the payroll records shall contain a
      notation indicating whether the employee does, or does not, normally
      reside in the labor area as defined in Attachment A, paragraph 1.

      Whenever the Secretary of Labor, pursuant to Section IV, paragraph 3b,
      has found that the wages of any laborer or mechanic include the amount
      of any costs reasonably anticipated in providing benefits under a plan or
      program described in Section 1(b)(2)(B) of the Davis Bacon Act, the
      contractor and each subcontractor shall maintain records which show
      that the commitment to provide such benefits is enforceable, that the
      plan or program is financially responsible, that the plan or program has
      been communicated in writing to the laborers or mechanics affected, and
      show the cost anticipated or the actual cost incurred in providing
      benefits. Contractors or subcontractors employing apprentices or
      trainees under approved programs shall maintain written evidence of the
      registration of apprentices and trainees, and ratios and wage rates
      prescribed in the applicable programs.

   c. Each contractor and subcontractor shall furnish, each week in which any
      contract work is performed, to the SHA resident engineer a payroll of
      wages paid each of its employees (including apprentices, trainees, and
      helpers, described in Section IV, paragraphs 4 and 5, and watchmen and
      guards engaged on work during the preceding weekly payroll period).
      The payroll submitted shall set out accurately and completely all of the
      information required to be maintained under paragraph 2b of this Section
      V. This information may be submitted in any form desired. Optional Form
      WH-347 is available for this purpose and may be purchased from the
      Superintendent of Documents (Federal stock number 029-005-0014-1),
The prime contractor is responsible for the submission of copies of payrolls by all subcontractors.

d. Each payroll submitted shall be accompanied by a "Statement of Compliance," signed by the contractor or subcontractor or his/her agent who pays or supervises the payment of the persons employed under the contract and shall certify the following:

1. that the payroll for the payroll period contains the information required to be maintained under paragraph 2b of this Section V and that such information is correct and complete;
2. that such laborer or mechanic (including each helper, apprentice, and trainee) employed on the contract during the payroll period has been paid the full weekly wages earned, without rebate, either directly or indirectly, and that no deductions have been made either directly or indirectly from the full wages earned, other than permissible deductions as set forth in the Regulations, 29 CFR 3;
3. that each laborer or mechanic has been paid not less that the applicable wage rate and fringe benefits or cash equivalent for the classification of worked performed, as specified in the applicable wage determination incorporated into the contract.

e. The weekly submission of a properly executed certification set forth on the reverse side of Optional Form WH-347 shall satisfy the requirement for submission of the "Statement of Compliance" required by paragraph 2d of this Section V.

f. The falsification of any of the above certifications may subject the contractor to civil or criminal prosecution under 18 U.S.C. 1001 and 31 U.S.C. 231.

g. The contractor or subcontractor shall make the records required under paragraph 2b of this Section V available for inspection, copying, or transcription by authorized representatives of the SHA, the FHWA, or the DOL, and shall permit such representatives to interview employees during working hours on the job. If the contractor or subcontractor fails to submit the required records or to make them available, the SHA, the FHWA, the DOL, or all may, after written notice to the contractor, sponsor, applicant, or owner, take such actions as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds. Furthermore, failure to submit the required records upon request or to make such records available may be grounds for debarment action pursuant to 29 CFR 5.12.

VI. RECORD OF MATERIALS, SUPPLIES, AND LABOR

1. On all Federal-aid contracts on the National Highway System, except those which provide solely for the installation of protective devices at railroad grade crossings, those which are constructed on a force account or direct labor basis, highway beautification contracts, and contracts for which the total final construction cost for roadway and bridge is less than $1,000,000 (23 CFR 635) the contractor shall:
a. Become familiar with the list of specific materials and supplies contained in Form FHWA-47, "Statement of Materials and Labor Used by Contractor of Highway Construction Involving Federal Funds," prior to the commencement of work under this contract.

b. Maintain a record of the total cost of all materials and supplies purchased for and incorporated in the work, and also of the quantities of those specific materials and supplies listed on Form FHWA-47, and in the units shown on Form FHWA-47.

c. Furnish, upon the completion of the contract, to the SHA resident engineer on Form FHWA-47 together with the data required in paragraph 1b relative to materials and supplies, a final labor summary of all contract work indicating the total hours worked and the total amount earned.

2. At the prime contractor's option, either a single report covering all contract work or separate reports for the contractor and for each subcontract shall be submitted.

VII. SUBLETTING OR ASSIGNING THE CONTRACT

1. The contractor shall perform with its own organization contract work amounting to not less than 30 percent (or a greater percentage if specified elsewhere in the contract) of the total original contract price, excluding any specialty items designated by the State. Specialty items may be performed by subcontract and the amount of any such specialty items performed may be deducted from the total original contract price before computing the amount of work required to be performed by the contractor's own organization (23 CFR 635).

   a. "Its own organization" shall be construed to include only workers employed and paid directly by the prime contractor and equipment owned or rented by the prime contractor, with or without operators. Such term does not include employees or equipment of a subcontractor, assignee, or agent of the prime contractor.

   b. "Specialty Items" shall be construed to be limited to work that requires highly specialized knowledge, abilities, or equipment not ordinarily available in the type of contracting organizations qualified and expected to bid on the contract as a whole and in general are to be limited to minor components of the overall contract.

   c. The contract amount upon which the requirements set forth in paragraph 1 of Section VII is computed includes the cost of material and manufactured products which are to be purchased or produced by the contractor under the contract provisions.

   d. The contractor shall furnish (a) a competent superintendent or supervisor who is employed by the firm, has full authority to direct performance of the work in accordance with the contract requirements, and is in charge of all construction operations (regardless of who performs the work) and (b) such other of its own organizational resources (supervision, management, and engineering services) as the SHA contracting officer determines is necessary to assure the performance of the contract.

   e. No portion of the contract shall be sublet, assigned or otherwise disposed of except with the written consent of the SHA contracting officer, or authorized representative, and such consent when given shall
VIII. **SAFETY: ACCIDENT PREVENTION**

1. In the performance of this contract the contractor shall comply with all applicable Federal, State, and local laws governing safety, health, and sanitation (23 CFR 635). The contractor shall provide all safeguards, safety devices and protective equipment and take any other needed actions as it determines, or as the SHA contracting officer may determine, to be reasonably necessary to protect the life and health of employees on the job and the safety of the public and to protect property in connection with the performance of the work covered by the contract.

2. It is a condition of this contract, and shall be made a condition of each subcontract, which the contractor enters into pursuant to this contract, that the contractor and any subcontractor shall not permit any employee, in performance of the contract, to work in surroundings or under conditions which are unsanitary, hazardous or dangerous to his/her health or safety, as determined under construction safety and health standards (29 CFR 1926) promulgated by the Secretary of Labor, in accordance with Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 333).

3. Pursuant to 29 CFR 1926.3, it is a condition of this contract that the Secretary of Labor or authorized representative thereof, shall have right of entry to any site of contract performance to inspect or investigate the matter of compliance with the construction safety and health standards and to carry out the duties of the Secretary under Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 333).

IX. **FALSE STATEMENTS CONCERNING HIGHWAY PROJECTS**

In order to assure high quality and durable construction in conformity with approved plans and specifications and a high degree of reliability on statements and representations made by engineers, contractors, suppliers, and workers on Federal-aid highway projects, it is essential that all persons concerned with the project perform their functions as carefully, thoroughly, and honestly as possible. Willful falsification, distortion, or misrepresentation with respect to any facts related to the project is a violation of Federal law. To prevent any misunderstanding regarding the seriousness of these and similar acts, the following notice shall be posted on each Federal-aid highway project (23 CFR 635) in one or more places where it is readily available to all persons concerned with the project:

**NOTICE TO ALL PERSONNEL ENGAGED ON FEDERAL-AID HIGHWAY PROJECTS**

18 U.S.C. 1020 reads as follows:

"Whoever, being an officer, agent, or employee of the United States, or of any State or Territory, or whoever, whether a person, association, firm, or corporation, knowingly makes any false statement, false representation, or false report as to the character, quality, quantity, or cost of the material used or to be used, or the quantity or quality of the work performed or to be performed, or the cost thereof in connection with the submission of plans, maps, specifications, contracts, or costs of construction on any highway or related project submitted for approval to the Secretary of Transportation; or
Whoever knowingly makes any false statement, false representation, false report or false claim with respect to the character, quality, quantity, or cost of any work performed or to be performed, or materials furnished or to be furnished, in connection with the construction of any highway or related project approved by the Secretary of Transportation; or

Whoever knowingly makes any false statement or false representation as to material fact in any statement, certificate, or report submitted pursuant to provisions of the Federal-aid Roads Act approved July 1, 1916, (39 Stat. 355), as amended and supplemented;

Shall be fined not more that $10,000 or imprisoned not more than 5 years or both."

X. IMPLEMENTATION OF CLEAN AIR ACT AND FEDERAL WATER POLLUTION CONTROL ACT

(Applicable to all Federal-aid construction contracts and to all related subcontracts of $100,000 or more.)

By submission of this bid or the execution of this contract, or subcontract, as appropriate, the bidder, Federal-aid construction contractor, or subcontractor, as appropriate, will be deemed to have stipulated as follows:

1. That any facility that is or will be utilized in the performance of this contract, unless such contract is exempt under the Clean Air Act, as amended (42 U.S.C. 1857 et seq., as amended by Pub.L. 91-604), and under the Federal Water Pollution Control Act, as amended (33 U.S.C. 1251 et seq., as amended by Pub.L. 92-500), Executive Order 11738, and regulations in implementation thereof (40 CFR 15) is not listed, on the date of contract award, on the U.S. Environmental Protection Agency (EPA) List of Violating Facilities pursuant to 40 CFR 15.20.

2. That the firm agrees to comply and remain in compliance with all the requirements of Section 114 of the Clean Air Act and Section 308 of the Federal Water Pollution Control Act and all regulations and guidelines listed there under.

3. That the firm shall promptly notify the SHA of the receipt of any communication from the Director, Office of Federal Activities, EPA, indicating that a facility that is or will be utilized for the contract is under consideration to be listed on the EPA List of Violating Facilities.

4. That the firm agrees to include or cause to be included the requirements of paragraph 1 through 4 of this Section X in every nonexempt subcontract, and further agrees to take such action as the government may direct as a means of enforcing such requirements.

XI. CERTIFICATION REGARDING DEBARMENT, SUSPENSION, INELIGIBILITY AND VOLUNTARY EXCLUSION

1. Instructions for Certification - Primary Covered Transactions:

   (Applicable to all Federal-aid contracts - 49 CFR 29)

   a. By signing and submitting this proposal, the prospective primary participant is providing the certification set out below.

   b. The inability of a person to provide the certification set out below will not necessarily result in denial of participation in this covered transaction.
The prospective participant shall submit an explanation of why it cannot provide the certification set out below. The certification or explanation will be considered in connection with the department or agency's determination whether to enter into this transaction. However, failure of the prospective primary participant to furnish a certification or an explanation shall disqualify such a person from participation in this transaction.

c. The certification in this clause is a material representation of fact upon which reliance was placed when the department or agency determined to enter into this transaction. If it is later determined that the prospective primary participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause of default.

d. The prospective primary participant shall provide immediate written notice to the department or agency to whom this proposal is submitted if any time the prospective primary participant learns that its certification was erroneous when submitted or has become erroneous by reason of changed circumstances.

e. The terms "covered transaction," "debarred," "suspended," "ineligible," "lower tier covered transaction," "participant," "person," "primary covered transaction," "principal," "proposal," and "voluntarily excluded," as used in this clause, have the meanings set out in the Definitions and Coverage sections of rules implementing Executive Order 12549. You may contact the department or agency to which this proposal is submitted for assistance in obtaining a copy of those regulations.

f. The prospective primary participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency entering into this transaction.

g. The prospective primary participant further agrees by submitting this proposal that it will include the clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," provided by the department or agency entering into this covered transaction, without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions.

h. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant may decide the method and frequency by which it determines the eligibility of its principals. Each participant may, but is not required to, check the nonprocurement portion of the "Lists of Parties Excluded From Federal Procurement or Nonprocurement Programs" (Nonprocurement List) which is compiled by the General Services Administration.

i. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause.
The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

j. Except for transactions authorized under paragraph f of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause or default.

* * * * *

Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion--Primary Covered Transactions

1. The prospective primary participant certifies to the best of its knowledge and belief, that it and its principals:
   a. Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from covered transactions by any Federal department or agency;
   b. Have not within a 3-year period preceding this proposal been convicted of or had a civil judgment rendered against them for commission of fraud or a criminal offense in connection with obtaining, attempting to obtain, or performing a public (Federal, State or local) transaction or contract under a public transaction; violation of Federal or State antitrust statutes or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, or receiving stolen property;
   c. Are not presently indicted for or otherwise criminally or civilly charged by a governmental entity (Federal, State or local) with commission of any of the offenses enumerated in paragraph 1b of this certification; and
   d. Have not within a 3-year period preceding this application/proposal had one or more public transactions (Federal, State or local) terminated for cause or default.

2. Where the prospective primary participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

* * * * *

1. Instructions for Certification - Lower Tier Covered Transactions:

(Applicable to all subcontracts, purchase orders and other lower tier transactions of $25,000 or more - 49 CFR 29)

a. By signing and submitting this proposal, the prospective lower tier is providing the certification set out below.
b. The certification in this clause is a material representation of fact upon which reliance was placed when this transaction was entered into. If it is later determined that the prospective lower tier participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department, or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

c. The prospective lower tier participant shall provide immediate written notice to the person to which this proposal is submitted if at any time the prospective lower tier participant learns that its certification was erroneous by reason of changed circumstances.

d. The terms "covered transaction," "debarred," "suspended," "ineligible," "primary covered transaction," "participant," "person," "principal," "proposal," and "voluntarily excluded," as used in this clause, have the meanings set out in the Definitions and Coverage sections of rules implementing Executive Order 12549. You may contact the person to which this proposal is submitted for assistance in obtaining a copy of those regulations.

e. The prospective lower tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency with which this transaction originated.

f. The prospective lower tier participant further agrees by submitting this proposal that it will include this clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions.

g. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant may decide the method and frequency by which it determines the eligibility of its principals. Each participant may, but is not required to, check the Nonprocurement List.

h. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

i. Except for transactions authorized under paragraph e of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.
Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion--Lower Tier Covered Transactions:

0. The prospective lower tier participant certifies, by submission of this proposal, that neither it nor its principals is presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participation in this transaction by any Federal department or agency.

1. Where the prospective lower tier participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

* * * * *

VII. CERTIFICATION REGARDING USE OF CONTRACT FUNDS FOR LOBBYING

(Applicable to all Federal-aid construction contracts and to all related subcontracts which exceed $100,000 - 49 CFR 20)

1. The prospective participant certifies, by signing and submitting this bid or proposal, to the best of his or her knowledge and belief, that:

   a. No Federal appropriated funds have been paid or will be paid, by or on behalf of the undersigned, to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with the awarding of any Federal contract, the making of any Federal grant, the making of any Federal loan, the entering into of any cooperative agreement, and the extension, continuation, renewal, amendment, or modification of any Federal contract, grant, loan, or cooperative agreement.

   b. If any funds other than Federal appropriated funds have been paid or will be paid to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with this Federal contract, grant, loan, or cooperative agreement, the undersigned shall complete and submit Standard Form-LLL, "Disclosure Form to Report Lobbying," in accordance with its instructions.

2. This certification is a material representation of fact upon which reliance was placed when this transaction was made or entered into. Submission of this certification is a prerequisite for making or entering into this transaction imposed by 31 U.S.C. 1352. Any person who fails to file the required certification shall be subject to a civil penalty of not less than $10,000 and not more than $100,000 for each such failure.

3. The prospective participant also agrees by submitting his or her bid or proposal that he or she shall require that the language of this certification be included in all lower tier subcontracts, which exceed $100,000 and that all such recipients shall certify and disclose accordingly.

© FHWA

United States Department of Transportation - Federal Highway Administration - Infrastructure
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Required Contract Provisions Federal-Aid Construction Contracts

1. Subsections IV and V; Modify the following wording below the subheading to read as follows:
   
   "(Applicable to all Federal-aid construction contracts exceeding $2,000 and to all related subcontracts.)"

2. Subsections V.2.b.; Delete the wording referencing “social security number” in the first sentence and Substitute “and the last four digits of the social security number”.


APPENDIX A
NOTICE TO CONTRACTORS
COMPLIANCE WITH TITLE VI OF THE CIVIL RIGHTS ACT OF 1964
FOR
FEDERAL-AID CONTRACTS

During the performance of this Contract, the Contractor, for itself, its assignees and successors in interest (hereinafter referred to as the “Contractor”), agrees as follows:

1. Compliance with Regulations: The Contractor will comply with the Regulations of the Department of Transportation relative to nondiscrimination in Federally-assisted programs of the Department of Transportation (Title 49, Code of Federal Regulations, Part 21, hereinafter referred to as the “Regulations”), which are herein incorporated by reference and made a part of the Contract.

2. Nondiscrimination: The Contractor, with regard to the work performed by it afterward and prior to completion of the contract work, will not discriminate on the ground of race, color, national origin, disability, sex, or age in the selection and retention of subcontracts including procurements of materials and leases of equipment. This will be done in accordance with Title VI of the Civil Rights Act of 1964 and other Non-Discrimination Authorities i.e., Section 504 of the 1973 Rehabilitation Act, the 1973 Federal-Aid Highway Act, the 1975 Age Discrimination Act, and the Americans with Disabilities Act of 1990. The Contractor will not participate either directly or indirectly in the discrimination prohibited by Section 21.5 of the Regulations, including employment practices when contract covers a program set forth in Appendix B of the Regulations. In addition, the Contractor will not participate either directly or indirectly in discrimination prohibited by 23 CFR 710.405 (b).

3. Solicitations for subcontracts, including procurements of materials and equipment: In all solicitations, either by competitive bidding or negotiation made by the Contractor for work to be performed under a subcontract, including procurements of materials or equipment, each potential subcontractor or supplier shall be notified by the Contractor of the Contractor’s obligations under this Contract and the Regulations relative to nondiscrimination on the ground of race, color, national origin, disability, sex or age.
4. Information and Reports: The Contractor will provide all information and reports required by the Regulations, or orders and instructions issued pursuant thereto, and will permit access to its books, records, accounts, other sources of information, and its facilities as may be determined by the Department of Transportation or the Federal Highway Administration to be pertinent to ascertain compliance with such Regulations, orders and instructions. Where any information required of a Contractor is in the exclusive possession of another who fails or refuses to furnish this information, the Contractor shall so certify to the Department of Transportation, or the Federal Highway Administration as appropriate, and shall set forth what efforts it has made to obtain the information.

5. Sanctions for Noncompliance: In the event of the Contractor’s noncompliance with the nondiscrimination provisions of this Contract, the Department of Transportation shall impose such Contract sanctions as it or the Federal Highway Administration may determine to be appropriate, including, but not limited to:

   (a) withholding of payments to the Contractors under the Contract until the Contractor complies, and/or

   (b) Cancellation, termination or suspension of the Contract, in whole or in part.

6. Incorporation of Provisions: The Contractor will include the provisions of paragraph (1) through (6) in every subcontract, including procurements of materials and leases of equipment, unless exempt by the Regulations, orders or instruction issued pursuant thereto. The Contractor will take such action with respect to any subcontract or procurement as the Department of Transportation or the Federal Highway Administration may direct as a means of enforcing such provisions including sanctions for noncompliance. Provided, however, that in the event a Contractor becomes involved in, or is threatened with, litigation with a subcontractor or supplier as result of such direction, the Contractor may request the State to enter into such litigation to protect the interests of the State, and, in addition, the Contractor may request the United States to enter into such litigation to protect the interest of the United States.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

Revised 7/24/09

U. S. Department of Labor

GENERAL WAGE DECISION NO. GA080305 06/26/2009 GA305

State: GEORGIA

County(ies): ATKINSON

Construction Type: Highway

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Number Publication Date
0 06/26/2009

SUGA 2009-001 05/22/2009

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WELDER – Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080306 06/26/2009 GA306

State: GEORGIA

County(ies): BALDWIN

Construction Type: Highway

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).
SUGA 2009-002     05/22/2009

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LABORER: FLAGGER ............................................................................ 8.43  1.39
LABORER: PIPELAYER ......................................................................... 8.50  0.00
OPERATOR: ASPHALT PAVER ............................................................... 14.50  0.86
OPERATOR: BACKHOE/EXCAVATOR .................................................... 11.40  0.00
OPERATOR: BLADE/GRADER ............................................................... 13.90  1.40
OPERATOR: BULLDOZER ..................................................................... 13.01  0.00
OPERATOR: CRANE ............................................................................. 14.70  0.00
OPERATOR: MECHANIC ...................................................................... 15.78  1.03
OPERATOR: SWEEPER ....................................................................... 13.20  1.40
OPERATOR: ROLLER ........................................................................... 11.90  1.34
TRUCK DRIVER ................................................................................... 10.96  0.00

WELDER – Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080307  06/26/2009  GA307

State: GEORGIA

County(ies): FANNIN

Construction Type: Highway

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; and other major bridges).

Modification Number Publication Date
0 06/26/2009

SUGA 2009-003     05/22/2009

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47
### GENERAL WAGE DECISION NO. GA080308 06/26/2009 GA308

State: GEORGIA

Construction type: Highway

County(ies): LAURENS

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; and other major bridges).

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WELDER – Receive rate prescribed for craft performing operation to which welding is incidental.
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA

GENERAL WAGE DECISION No. GA080309  06/26/2009  GA309

State:        GEORGIA

Construction type: Highway

County(ies):  RANDOLPH

Construction Description:  HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; other major bridges).

Modification Number  Publication Date
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080310  06/26/2009  GA310

State:        GEORGIA

Construction type: Highway

County(ies):  TOWNS

Construction Description:  HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; other major bridges).
Modification Number   Publication Date
0             06/26/2009

SUGA2009-006     05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080311  06/26/2009  GA311

State: GEORGIA

Construction Type: Highway

County(ies): WHITE

Construction description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Number   Publication Date
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DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

OPERATOR: BACKHOE/EXCAVATOR 12.06 0.00
OPERATOR: BLADE/GRADER 13.90 1.40
OPERATOR: BULLDOZER 13.48 1.67
OPERATOR: CRANE 15.56 3.95
OPERATOR: MECHANIC 15.78 1.13
OPERATOR: SWEEPER 13.20 1.40
OPERATOR: ROLLER 11.86 1.34
TRUCK DRIVER 12.36 1.40

WELDERS: Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080312 06/26/2009 GA312

State: GEORGIA
Construction type: Highway
County(ies): WILCOX

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Number Publication Date
0 06/26/2009

SUGA2009-008 05/22/2009

| CARPENTER, EXCLUDING CABINET INSTAL. AND FORM WORK | 12.45 | 2.71 |
| CEMENT MASON/CONCRETE FINISHER | 11.07 | 2.40 |
| LABORER: COMMON OR GENERAL | 8.41 | 0.00 |
| LABORER: FLAGGER | 8.43 | 1.39 |
| LABORER: PIPELAYER | 8.50 | 0.00 |
| OPERATOR: ASPHALT PAVER | 14.50 | 0.86 |
| OPERATOR: BACKHOE/EXCAVATOR | 11.40 | 0.00 |
| OPERATOR: BLADE/GRADER | 13.90 | 1.40 |
| OPERATOR: BULLDOZER | 13.01 | 0.00 |
| OPERATOR: CRANE | 14.70 | 0.00 |
| OPERATOR: MECHANIC | 15.78 | 1.03 |
| OPERATOR: SWEEPER | 13.20 | 1.40 |
| OPERATOR: ROLLER | 11.90 | 1.34 |
| TRUCK DRIVER | 10.96 | 0.00 |

WELDERS: Receive rate prescribed for craft performing operation to which welding is incidental.
GENERAL WAGE DECISION NO. GA080313 06/26/2009 GA313

State: GEORGIA

Construction Type: Highway

County(ies): APPLING AND BACON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Number Publication Date
0 06/26/2009

SUGA2009-009 05/22/2009

RATES FRINGES
CEMENT MASON/CONCRETE FINISHER ......................................................... 11.55 0.00
LABORER: COMMON OR GENERAL ............................................................... 8.41 0.00
LABORER: FLAGGER ...................................................................................... 8.43 1.39
LABOR: PIPELAYER ...................................................................................... 8.50 0.00
OPERATOR: ASPHALT PAVER ....................................................................... 14.50 0.86
OPERATOR: BACKHOE/EXCAVATOR ............................................................ 11.86 0.00
OPERATOR: BLADE/GRADER ...................................................................... 13.90 1.40
OPERATOR: BULLDOZER ............................................................................. 15.75 0.00
OPERATOR: CRANE ........................................................................................ 13.89 0.00
OPERATOR: MECHANIC ............................................................................... 15.78 1.03
OPERATOR: SWEeper .................................................................................... 13.20 1.40
OPERATOR: ROLLER ..................................................................................... 11.90 1.34
TRUCK DRIVER ........................................................................................... 11.23 0.00

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080314 6/26/2009 GA314

State: GEORGIA

Construction Type: Highway

County(ies): BANKS, FRANKLIN, GILMER, GORDON, HABERSHAM, HART, LUMPIN, MORGAN, POLK, RABUN, STEPHENS AND UNION

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA 2009-010 05/22/2009
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

Carpenter: (Form Work Only) ................................................................. 12.02  2.55
Cement Mason/Concrete Finisher ............................................................. 12.03  0.00
Laborer: Asphalt Raker ............................................................................ 11.40  1.39
Laborer: Common or General ................................................................. 9.25   1.57
Laborer: Flagger ...................................................................................... 8.43   1.39
Laborer: Pipelayer .................................................................................... 8.50   0.00
Operator: Asphalt Paver ......................................................................... 14.50  0.86
Operator: Backhoe/Excavator ............................................................... 12.06  0.00
Operator: Blade/Grader ........................................................................... 13.90  1.40
Operator: Bulldozer .................................................................................. 13.48  1.67
Operator: Crane ....................................................................................... 15.56  3.95
Operator: Mechanic .................................................................................. 15.78   1.13
Operator: Sweeper .................................................................................... 13.20  1.40
Operator: Roller ....................................................................................... 11.86  1.34
Truck Driver .............................................................................................. 12.36  1.40

Welders - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080315  6/26/2009  GA315

State: GEORGIA

Construction type: Highway
County(ies): BEN HILL, BERRIEN, CALHOUN, CAMDEN, CLAY, CLINCH, COFFEE,
COLQUITT, COOK, DECATUR, GRADY, IRWIN, JEFF DAVIS,
MITCHELL, PIERCE, QUITMAN, TELFAIR, THOMAS, TIFT, TURNER,
WARE AND WAYNE

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels,
building structures in rest area projects, and railroad construction; bascule, suspension,
and spandrel arch bridges; bridges designed for commercial navigation; bridges involving
marine construction; other major bridges).

Modification Date Publication Date
0  06/26/2009

SUGA  2009-011  05/22/2009

Cement Mason/Concrete Finisher ............................................................. 11.55  0.00
Laborer: Common or General ................................................................. 8.41  0.00
Laborer: Flagger ...................................................................................... 8.43  1.39
Laborer: Pipelayer .................................................................................... 8.50  0.00
Operator: Asphalt Paver ......................................................................... 14.50  0.86
Operator: Backhoe/Excavator ............................................................... 11.86  0.00
Operator: Blade/Grader ........................................................................... 13.90  1.40
Operator: Bulldozer .................................................................................. 15.75  0.00
Operator: Crane ....................................................................................... 13.89  0.00
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

OPERATOR: MECHANIC................................................................. 15.78 1.03
OPERATOR: SWEEPER.............................................................. 13.20 1.40
OPERATOR: ROLLER................................................................. 11.90 1.34
TRUCK DRIVER....................................................................... 11.23 0.00

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080316 6/26/2009 GA316

State: GEORGIA

Construction type: Highway

County(ies): BLECKLEY, CRISP, DODGE, DOOLY, MACON, PEACH, PULASKI, PUTNAM, SCHLEY, SUMTER, TALBOT, TAYLOR, TROUP, WEBSTER AND WILKINSON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA 2009-012 05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
GENERAL WAGE DECISION NO. GA080317 6/26/2009 GA317

State: GEORGIA

Construction type: Highway

County(ies): BULLOCH, CANDLER, EMANUEL, EVANS, GLASCOCK, HANCOCK, JEFFERSON, JENKINS, JOHNSON, LINCOLN, MONTGOMERY, TATTNALL, TOOMBS, TREATLEN, WARREN AND WASHINGTON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

SUGA 2009-013 05/22/2009

RATES FRINGES
CEMENT MASON/CONCRETE FINISHER .......................................................... 12.03 0.00
LABORER: COMMON OR GENERAL ................................................................. 9.52 0.00
LABORER: FLAGGER ....................................................................................... 8.43 1.39
LABORER: PIPELAYER .................................................................................... 8.50 0.00
OPERATOR: ASPHALT PAVER ....................................................................... 14.50 0.86
OPERATOR: BACKHOE/EXCAVATOR ............................................................. 12.06 0.00
OPERATOR: BLADE/GRADER ....................................................................... 13.90 1.40
OPERATOR: BULLDOZER ............................................................................... 11.69 0.00
OPERATOR: CRANE ....................................................................................... 15.07 3.95
OPERATOR: MECHANIC ............................................................................... 15.78 1.13
OPERATOR: SWEEPER ................................................................................. 13.20 1.40
OPERATOR: ROLLER ...................................................................................... 11.86 1.34
TRUCK DRIVER ............................................................................................ 12.31 1.40

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080318 6/26/2009 GA318

State: GEORGIA

Construction type: Highway

County(ies): CHARLTON, EARLY, MILLER AND SEMINOLE

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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0 06/26/2009
SUGA 2009-014 05/22/2009
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080319 6/26/2009 GA319

State: GEORGIA

Construction type: Highway
County(ies): CHATTOOGA AND ELBERT,

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA 2009-015 05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
GENERAL WAGE DECISION NO. GA080320  6/26/2009  GA320

State: GEORGIA

Construction type: Highway

County(ies): GREENE AND JACKSON,

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
06/26/2009

SUGA  2009-016  05/22/2009

RATES   FRINGES

CARPENTER (FORM WORK ONLY) ................................................................. 12.02    2.55
CEMENT MASON/CONCRETE FINISHER ...................................................... 12.03    0.00
LABORER: ASPHALT RAKER ......................................................................... 11.40    1.39
LABORER: COMMON OR GENERAL ............................................................. 9.25    1.57
LABORER: FLAGGER ..................................................................................... 8.43    1.39
LABORER: PIPELAYER .................................................................................. 8.50    0.00
OPERATOR: ASPHALT PAVER ................................................................. 14.50    0.86
OPERATOR: BACKHOE ............................................................................ 12.06    0.00
OPERATOR: BLADE/GRADER .................................................................... 13.90    1.40
OPERATOR: BULLDOZER .......................................................................... 13.48    1.67
OPERATOR: CRANE .................................................................................... 15.56    3.95
OPERATOR: MECHANIC ............................................................................ 15.78    1.13
OPERATOR: SWEEPER ............................................................................ 13.20    1.40
OPERATOR: ROLLER .................................................................................. 11.86    1.34
TRUCK DRIVER ........................................................................................... 12.36    1.40

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080321  6/26/2009  GA321

State: GEORGIA

Construction type: Highway

County(ies): SCREVEN, TALIAFERRO, WHEELER AND WILKES

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
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SUGA  2009-017  05/22/2009

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## DEPARTMENT OF TRANSPORTATION
### STATE OF GEORGIA

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

## GENERAL WAGE DECISION NO. GA080322 6/26/2009 GA322

**State:** GEORGIA

**Construction type:** Highway

**County(ies):** STEWART AND UPSON

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

**Modification Date** 0  **Publication Date** 06/26/2009

**SUGA 2009-018 05/22/2009**

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**GENERAL WAGE DECISION NO. GA080323  6/26/2009  GA323**

State: GEORGIA  
Construction type: Highway  
County(ies): BAKER

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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**GENERAL WAGE DECISION NO. GA080324  6/26/2009  GA324**

State: GEORGIA  
Construction type: Highway  
County(ies): BARROW

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
### DEPARTMENT OF TRANSPORTATION
### STATE OF GEORGIA

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#### GENERAL WAGE DECISION NO. GA080325  7/24/2009 GA325

**State:** GEORGIA  
**Construction type:** Highway  
**County(ies):** BRANTLEY  

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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OPERATOR: SCREED .......................................................... 13.38  1.63
TRUCK DRIVER .......................................................... 9.00  0.00

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080326  6/26/2009 GA326

State: GEORGIA

Construction type: Highway

County(ies): BROOKS

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date: 0
Publication Date: 06/26/2009

SUGA  2009-022  05/22/2009

RATES  FRINGES
CARPENTER, (Including form work) ........................................ 11.75  0.66
CEMENT MASON/CONCRETE FINISHER ......................... 12.00  1.32
IRONWORKER, REINFORCING ........................................ 11.80  1.85
LABORER: COMMON OR GENERAL ................................... 8.54  0.00
OPERATOR: ASPHALT PAVER ........................................ 12.00  0.00
OPERATOR: ASPHALT SPREADER ................................. 10.36  1.57
OPERATOR: BACKHOE/EXCAVATOR ......................... 12.71  1.99
OPERATOR: BULLDOZER ............................................. 12.50  1.34
OPERATOR: CRANE ..................................................... 14.49  0.00
OPERATOR: GRADER/BLADE ........................................ 12.00  0.00
OPERATOR: LOADER .................................................. 11.43  0.00
OPERATOR: ROLLER .................................................... 10.92  1.35
OPERATOR: SCREED ................................................... 13.38  1.63
TRUCK DRIVER ......................................................... 11.05  1.22

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080327  7/24/2009 GA327

State: GEORGIA

Construction type: Highway

County(ies): BRYAN

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

Modification Date Publication Date
0 06/26/2009
1 07/24/2009

SUGA 2009-023 05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080328 6/26/2009 GA328

State: GEORGIA

Construction type: Highway
County(ies): BUTTS

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA 2009-024 05/22/2009

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GENERAL WAGE DECISION NO. GA080329  6/26/2009  GA329

State: GEORGIA

Construction type: Highway
County(ies): CATOOSA

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
06/26/2009

SUGA  2009-025  05/22/2009

RATES   FRINGES
CARPENTER, (Including form work).................................................................11.29  0.52
CEMENT MASON/CONCRETE FINISHER .......................................................11.66  0.52
IRONWORKER, REINFORCING...................................................................11.80  1.85
LABORER: COMMON OR GENERAL............................................................8.66  0.52
OPERATOR: ASPHALT PAVER......................................................................12.00  0.00
OPERATOR: ASPHALT SPREADER...............................................................10.36  1.57
OPERATOR: BACKHOE/EXCAVATOR.........................................................14.79  0.51
OPERATOR: BULLDOZER...........................................................................13.68  1.72
OPERATOR: CRANE..................................................................................13.57  0.53
OPERATOR: GRADER/BLADE.....................................................................12.00  0.00
OPERATOR: LOADER................................................................................11.43  0.00
OPERATOR: ROLLER................................................................................10.92  1.35
OPERATOR: SCREED................................................................................13.38  1.63
TRUCK DRIVER......................................................................................10.99  0.00

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

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DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

GENERAL WAGE DECISION NO. GA080330 7/24/2009 GA330

State: GEORGIA
Construction type: Highway
County(ies): CHATHAM

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009
1 07/24/2009

SUGA 2009-026 05/22/2009

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<td>TRUCK DRIVER</td>
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080331 6/26/2009 GA331

State: GEORGIA
Construction type: Highway
County(ies): CHATTAHOOCHEE

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA 2009-027 05/22/2009

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<td>10.36</td>
<td>1.57</td>
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<tr>
<td>Operator: Backhoe/Excavator</td>
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

**GENERAL WAGE DECISION NO. GA080332 6/26/2009 GA332**

State: GEORGIA

Construction type: Highway

County(ies): COWETA

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date: 0  Publication Date: 06/26/2009

SUGA 2009-028 05/22/2009

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<td>Ironworker, Reinforcing</td>
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<td>1.57</td>
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
State: GEORGIA

Construction type: Highway

County(ies): DADE

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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SUGA  2009-029  05/22/2009

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State: GEORGIA

Construction type: Highway

County(ies): DAWSON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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SUGA  2009-030  05/22/2009
### DEPARTMENT OF TRANSPORTATION
### STATE OF GEORGIA

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

### GENERAL WAGE DECISION NO. GA080335 6/26/2009 GA335

State: GEORGIA

Construction type: Highway
County(ies): DEKALB

Construction
Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
06/26/2009

SUGA 2009-031 05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
GENERAL WAGE DECISION NO. GA080336  6/26/2009  GA336

State: GEORGIA

Construction type: Highway
County(ies): DOUGHERTY

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date  Publication Date
0  06/26/2009

SUGA  2009-032  05/22/2009

RATES  FRINGES
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CEMENT MASON/CONCRETE FINISHER .......................................................... 12.30  0.00
IRONWORKER, REINFORCING ..................................................................... 11.80  1.85
LABORER: COMMON OR GENERAL ............................................................... 8.54  0.00
OPERATOR: ASPHALT PAVER ....................................................................... 12.00  0.00
OPERATOR: ASPHALT SPREADER ............................................................... 10.36  1.57
OPERATOR: BACKHOE/EXCAVATOR ........................................................... 11.42  0.00
OPERATOR: BULLDOZER ............................................................................... 12.50  1.34
OPERATOR: CRANE ..................................................................................... 14.49  0.00
OPERATOR: GRADER/BLADE ..................................................................... 12.00  0.00
OPERATOR: LOADER .................................................................................. 11.43  0.00
OPERATOR: ROLLER .................................................................................. 10.92  1.35
OPERATOR: SCREED .................................................................................. 13.38  1.63
TRUCK DRIVER .......................................................................................... 11.05  1.22
WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080337  6/26/2009  GA337

State: GEORGIA

Construction type: Highway
County(ies): ECHOLS

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date  Publication Date
0  06/26/2009

SUGA  2009-033  05/22/2009
CARPENTER, (Including form work)..........................12.04  0.00
CEMENT MASON/CONCRETE FINISHER..................12.00  1.32
IRONWORKER, REINFORCING..............................11.80  1.85
LABORER: COMMON OR GENERAL............................8.54  0.00
OPERATOR: ASPHALT PAVER.................................12.00  0.00
OPERATOR: ASPHALT SPREADER.............................10.36  1.57
OPERATOR: BACKHOE/EXCAVATOR.........................12.71  1.99
OPERATOR: BULLDOZER........................................12.50  1.34
OPERATOR: CRANE...............................................14.49  0.00
OPERATOR: GRADER/BLADE.................................12.00  0.00
OPERATOR: LOADER.............................................11.43  0.00
OPERATOR: ROLLER...............................................10.99  1.34
OPERATOR: SCREED..............................................13.38  1.63
TRUCK DRIVER....................................................11.05  1.22

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080338  7/24/2009  GA338

State: GEORGIA

Construction type: Highway
County(ies): EFFINGHAM

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

SUGA  2009-034  05/22/2009

CARPENTER, (Including form work)..........................13.19  0.26
CEMENT MASON/CONCRETE FINISHER..................8.72  0.00
IRONWORKER, REINFORCING..............................11.80  1.85
LABORER: COMMON OR GENERAL............................7.25  0.00
OPERATOR: ASPHALT PAVER.................................12.00  0.00
OPERATOR: ASPHALT SPREADER.............................10.36  1.57
OPERATOR: BACKHOE/EXCAVATOR.........................13.33  1.36
OPERATOR: BULLDOZER........................................13.68  1.72
OPERATOR: CRANE...............................................14.79  0.00
OPERATOR: GRADER/BLADE.................................12.00  0.00
OPERATOR: LOADER.............................................11.43  0.00
OPERATOR: ROLLER...............................................10.92  1.35
OPERATOR: SCREED..............................................13.38  1.63
TRUCK DRIVER....................................................13.80  1.22

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
### GENERAL WAGE DECISION NO. GA080339 6/26/2009 GA339

**State:** GEORGIA  
**Construction type:** Highway  
**County(ies):** FAYETTE  

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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**SUGA 2009-035 05/22/2009**

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

### GENERAL WAGE DECISION NO. GA080340 6/26/2009 GA340

**State:** GEORGIA  
**Construction type:** Highway  
**County(ies):** FLOYD  

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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**SUGA 2009-036 05/22/2009**
### GENERAL WAGE DECISION NO. GA080341  6/26/2009  GA341

**State:**  GEORGIA  
**Construction type:** Highway  
**County(ies):** FORSYTH  

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

**Modification Date**  Publication Date  
0  06/26/2009  

**SUGA  2009-037  05/22/2009**

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
GENERAL WAGE DECISION NO. GA080342  6/26/2009 GA342

State: GEORGIA

Construction type: Highway
County(ies): FULTON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA  2009-038  05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080343  7/24/2009 GA343

State: GEORGIA

Construction type: Highway
County(ies): GLYNN

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009
1 07/24/2009

SUGA  2009-039  05/22/2009
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<td>Operator: Backhoe/Excavator</td>
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

**GENERAL WAGE DECISION NO. GA080344  6/26/2009  GA344**

State: GEORGIA

Construction type: Highway

County(ies): GWINNETT

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date: 0
Publication Date: 06/26/2009

SUGA 2009-040  05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
**DEPARTMENT OF TRANSPORTATION**  
**STATE OF GEORGIA**

**GENERAL WAGE DECISION NO. GA080345  6/26/2009  GA345**

State: GEORGIA  
Construction type: Highway  
County(ies): HALL  

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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**SUGA  2009-041  05/22/2009**  

**GENERAL WAGE DECISION NO. GA080346  6/26/2009  GA346**

State: GEORGIA  
Construction type: Highway  
County(ies): HARRIS  

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080347  6/26/2009  GA347

State: GEORGIA

Construction type: Highway

County(tes): HOUSTON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date 0
Publication Date 06/26/2009

SUGA 2009-043  05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
GENERAL WAGE DECISION NO. GA080348  6/26/2009  GA348

State:          GEORGIA

Construction type: Highway
County(ies):   LANIER

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0       06/26/2009

SUGA  2009-044  05/22/2009

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LABORER: COMMON OR GENERAL................................................................. 8.54  0.00
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OPERATOR: ASPHALT SPREADER.................................................................... 10.36  1.57
OPERATOR: BACKHOE/EXCAVATOR............................................................... 12.71  1.99
OPERATOR: BULLDOZER.................................................................................... 12.50  1.34
OPERATOR: CRANE ............................................................................................ 14.49  0.00
OPERATOR: GRADER/BLADE............................................................................ 12.00  0.00
OPERATOR: LOADER........................................................................................... 11.43  0.00
OPERATOR: ROLLER............................................................................................ 10.92  1.35
OPERATOR: SCREED............................................................................................ 13.38  1.63
TRUCK DRIVER...................................................................................................... 11.05  1.22

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080349  7/24/2009  GA349

State:          GEORGIA

Construction type: Highway
County(ies):   LONG

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0       06/26/2009
1       07/24/2009

SUGA  2009-045  05/22/2009
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080350  6/26/2009  GA350

State: GEORGIA

Construction type: Highway

County(ies): LOWNDES

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date   Publication Date
0                  06/26/2009

SUGA  2009-046     05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
## GENERAL WAGE DECISION NO. GA080351  6/26/2009  GA351

**State:** GEORGIA  
**Construction type:** Highway  
**County(ies):** MCDUFFIE

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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**SUGA  2009-047  05/22/2009**

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

## GENERAL WAGE DECISION NO. GA080352  6/26/2009  GA352

**State:** GEORGIA  
**Construction type:** Highway  
**County(ies):** MONROE

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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**SUGA  2009-048  05/22/2009**
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

## GENERAL WAGE DECISION NO. GA080353  6/26/2009  GA353

State: GEORGIA

Construction type: Highway

County(ies): MURRAY

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date  Publication Date
0  06/26/2009

SUGA  2009-049  05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
## GENERAL WAGE DECISION NO. GA080354 6/26/2009 GA354

State: GEORGIA  
Construction type: Highway  
County(ies): ROCKDALE  

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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SUGA 2009-050 05/22/2009  

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

## GENERAL WAGE DECISION NO. GA080355 6/26/2009 GA355

State: GEORGIA  
Construction type: Highway  
County(ies): TERRELL  

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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SUGA 2009-051 05/22/2009  

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

State: GEORGIA

Construction type: Highway

County(ies): TWIGGS

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date 0  Publication Date 06/26/2009

SUGA  2009-052  05/22/2009

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
**GENERAL WAGE DECISION NO. GA080357 6/26/2009 GA357**

State: GEORGIA  
Construction type: Highway  
County(ies): WALKER  

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

**GENERAL WAGE DECISION NO. GA080358 6/26/2009 GA358**

State: GEORGIA  
Construction type: Highway  
County(ies): WHITFIELD  

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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SUGA 2009-054 05/22/2009  

82
### DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA

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**WELDERS** - Receive rate prescribed for craft performing operation to which welding is incidental.

### GENERAL WAGE DECISION NO. GA080359  6/26/2009  GA359

State: GEORGIA

Construction type: Highway

County(ties): BARTOW, CARROLL, CHEROKEE, CLAYTON, COBB, DOUGLAS, HARALSON, HEARD, HENRY, JASPER AND LAMAR

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date | Publication Date
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0 | 06/26/2009
SUGA 2009-055 | 05/22/2009

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**WELDERS** - Receive rate prescribed for craft performing operation to which welding is incidental.
# GENERAL WAGE DECISION NO. GA080360 6/26/2009 GA360

State: GEORGIA  
Construction type: Highway  
County(ies): BIBB, CRAWFORD AND JONES  

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

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# GENERAL WAGE DECISION NO. GA080361 6/26/2009 GA361

State: GEORGIA  
Construction type: Highway  
County(ies): BURKE, CLARKE AND COLUMBIA  

**Construction Description:** HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

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**GENERAL WAGE DECISION NO. GA080362  6/26/2009 GA362**

State: GEORGIA

Construction type: Highway

County(ies): LEE AND WORTH

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

**SUGA 2009-058  05/22/2009**

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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

GENERAL WAGE DECISION NO. GA080363 7/24/2009 GA363

State: GEORGIA

Construction type: Highway
County(ies): LIBERTY AND MCINTOSH

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009
1 07/24/2009

SUGA 2009-059 05/22/2009

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OPERATOR: CRANE .................................................. 14.79 0.00
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

GENERAL WAGE DECISION NO. GA080364 6/26/2009 GA364

State: GEORGIA

Construction type: Highway
County(ies): MADISON, OCONEE, OGLETHORPE AND RICHMOND

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009

SUGA 2009-060 05/22/2009
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WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

**GENERAL WAGE DECISION NO. GA080365 6/26/2009 GA3625**

State: GEORGIA

Construction type: Highway

County(tes): MARION AND MUSCOGEE

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date Publication Date
0 06/26/2009
SUGA 2009-061 05/22/2009

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

GENERAL WAGE DECISION NO. GA080366  6/26/2009  GA366

State: GEORGIA

Construction type: Highway

County(ies): MERIWETHER, NEWTON, PAULDING, PICKENS, PIKE, SPALDING AND WALTON

Construction Description: HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects, and railroad construction; bascule, suspension, and spandrel arch bridges; bridges designed for commercial navigation; bridges involving marine construction; other major bridges).

Modification Date   Publication Date
0                 06/26/2009

SUGA  2009-062  05/22/2009

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</tr>
</tbody>
</table>

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

Unlisted classifications needed for work not included within the scope of the classifications listed may be added after award only as provided in the labor standards contract clauses (29 CFR 5.5(a)(1)(ii)).
1. As used in these specifications:
   a. “Covered area” means the geographical area described in the solicitation from which this contract resulted;
   b. “Director” means Director, Office of Federal Contract Compliance Programs, United States Department of Labor, or any person to whom the Director delegated authority;
   d. “Minority” includes:
      (i) Black (all persons having origins in any of the Black African racial groups not of Hispanic origin);
      (ii) Hispanic (all persons of Mexican, Puerto Rican, Cuban, Central or South American or other Spanish Culture or origin, regardless of race);
      (iii) Asian and Pacific Islander (all persons having origins in any of the original peoples of the Far East, Southeast Asia, the Indian Subcontinent, or the Pacific Islands); and
      (iv) American Indian or Alaskan Native (all persons having origins in any of the original peoples of North America and maintaining identifiable tribal affiliations through membership and participation or community identification).

2. Whenever the Contractor, or any Subcontractor at any tier, subcontracts a portion of the work involving any construction trade, it shall physically include in each subcontract in excess of $10,000 the provisions of these specifications and the Notice which contains the applicable goals for minority and female participation and which is set forth in the solicitations from which this contract resulted.

3. If the Contractor is participating (pursuant to 41 CFR 60-4.5) in a Hometown Plan approved by the U.S. Department of Labor in the covered area either individually or through an association, its affirmative action obligations on all work in the Plan area (including goals and timetables) shall be in accordance with that Plan for those trades which have unions participating in the Plan. Contractors must be able to demonstrate their participation in and compliance with the provisions of any such Hometown Plan. Each Contractor or Subcontractor participating in an approved Plan is individually required to comply with its obligations under the EEO clause, and to make a good faith effort to achieve each goal under the Plan in each trade in which it has employees. The overall good faith performance by other Contractors or Subcontractors toward a goal in an approved Plan does not excuse any covered Contractor’s or Subcontractor’s failure to take good faith efforts to achieve the Plan goals and timetables.

4. The Contractor shall implement the specific affirmative action standards provided in paragraphs 7a through p of these specifications. The goals set forth in the solicitation from which this contact resulted are expressed as percentages of the total hours of employment and training of minority and female utilization the Contractor should reasonably be able to achieve in each construction trade in which it has employees in the covered area. The Contractor is expected to make substantially uniform progress toward its goals in each craft during the period specified.
5. Neither the provisions of any collective bargaining agreement, nor the failure by a union with whom the Contractor has a collective bargaining agreement, to refer either minorities or women shall excuse the Contractor’s obligations under these specifications, Executive Order 11246, nor the regulations promulgated pursuant thereto.

6. In order for the non-working training hours of apprentices and trainees to be counted in meeting the goals, such apprentices and trainees must be employed by the Contractor during the training period, and the Contractor must have made a commitment to employ the apprentices and trainees at the completion of their training, subject to the availability of employment opportunities. Trainees must be trained pursuant to training programs approved by the U.S. Department of Labor.

7. The Contractor shall take specific affirmative actions to ensure equal employment opportunity. The evaluation of the Contractor’s compliance with these specifications shall be based upon its effort to achieve maximum results from its actions. The Contractor shall document these efforts fully, and shall implement affirmative action steps at least as extensive as the following:

   a. Ensure and maintain a working environment free of harassment, intimidation, and coercion at all sites, and in all facilities at which the Contractor’s employees are assigned to work. The Contractor, where possible, will assign two or more women to each construction project. The Contractor shall specifically ensure that all foremen, superintendents, and other on-site supervisory personnel are aware of and carry out the Contractor’s obligation to maintain such a working environment, with specific attention to minority or female individuals working at such sites or in such facilities.

   b. Establish and maintain a current list of minority and female recruitment sources, provide written notification to minority and female recruitment sources and to community organizations when the Contractor or its unions have employment opportunities available, and maintain a record of the organization’s responses.

   c. Maintain a current file of the names, addresses and telephone numbers of each minority and female off-the-street applicant and minority or female referral from a union, a recruitment source or community organization and of what action was taken with respect to each such individual. If such individual was sent to the union hiring hall for referral and was not referred back to the Contractor by the union or, if referred, not employed by the Contractor, this shall be documented in the file with the reason therefore, along with whatever additional actions the Contractor may have taken.

   d. Provide immediate written notification to the Director when the union or unions with which the Contractor has a collective bargaining agreement has not referred to the Contractor a minority person or woman sent by the Contractor, or when the Contractor has other information that the union referral process has impeded the Contractor’s efforts to meet its obligations.

   e. Develop on-the-job training opportunities and/or participate in training programs for the area which expressly include minority and women, including upgrading programs and apprenticeship and trainee programs relevant to the Contractor’s employment needs, especially those programs funded or approved by the Department of Labor. The Contractor shall provide notice of these programs to the sources compiled under 7b above.

   f. Disseminate the Contractor’s EEO policy by providing the notice of the policy to unions and training programs and requesting their cooperation in assisting the Contractor in meeting its EEO obligations; by including it in any policy manual and collective bargaining agreement; by publicizing it in the company newspaper, annual report, etc.; by specific review of the policy with all management personnel and with all minority and female employees at least once a year, and by posting the company EEO policy on bulletin boards accessible to all employees at each location where construction work is performed.
g. Review, at least annually, the company’s EEO policy and affirmative action obligations under these specifications with all employees having any responsibility for hiring, assignment, layoff, termination or other employment decisions including specific review of these items with onsite supervisory personnel such as Superintendents, General Foremen, etc. prior to the initiation of construction work at any job site. A written record shall be made and maintained identifying the time and place of these meetings, persons attending, subject matter discussed, and disposition of the subject matter.

h. Disseminate the Contractor’s EEO policy externally by including it in any advertising in the news media, specifically including minority and female news media, and providing written notification to and discussing the Contractor’s EEO policy with other Contractors and Subcontractors with whom the Contractor does or anticipates doing business.

i. Direct its recruitment efforts, both oral and written, to minority, female and community organization, to schools with minority and female students and to minority and female recruitment and training organizations serving the Contractor’s area and employment needs. Not later than one month prior to the date for the acceptance of applications for apprenticeship or other training by any recruitment source, the Contractor shall send written notification to organizations such as the above, describing the openings, screening procedures, and test to be used in the selection process.

j. Encourage present minority and female employees to recruit other minority persons and women and, where reasonable, provide after school, summer and vacation employment to minority and female youth both on the site and in other areas of a Contractor’s workforce.

k. Validate all tests and other selection requirements where there is an obligation to do so under 41 CFR Part 60-3.

l. Conduct, at least annually, an inventory and evaluation of all minority and female personnel for promotional opportunities and encourage these employees to seek or to prepare for, through appropriate training, etc. such opportunities.

m. Ensure that seniority practices, job classifications, work assignments and other personnel practices, do not have a discriminatory effect by continually monitoring all personnel and employment related activities to ensure that the EEO policy and the Contractor’s obligations under these specifications are being carried out.

n. Ensure that all facilities and company activities are non-segregated except that separate or single-user toilet and necessary changing facilities shall be provided to assure privacy between the sexes.

o. Document and maintain a record of all solicitations of offers for subcontracts from minority and female construction contractors and suppliers, including circulation of solicitations to minority and female contractor associations and other business associations.

p. Conduct a review, at least annually of all supervisors’ adherence to and performance under the Contractor’s EEO policies and affirmative action obligations.

8. Contractors are encouraged to participate in voluntary associations which assist in fulfilling one or more of their affirmative action obligations (7a through p). The efforts of a contractor association, joint contractor-union, contactor-community, or other similar group of which the contractor is a member and participant, may be asserted as fulfilling any one or more of its obligations under 7a through p of these Specifications provided that the contractor actively participates in the group, makes every effort to assure that the group has a positive impact on the employment of minorities and women in the industry, ensures that the concrete
benefits of the program are reflected in the Contractor’s minority and female workforce participation, makes a good faith effort to meet its individual goals and timetables, and can provide access to documentation which demonstrates the effectiveness of actions taken on behalf of the Contractor. The obligation to comply, however, is the Contractor’s and failure of such a group to fulfill an obligation shall not be a defense for the Contractor’s noncompliance.

9. A single goal for minorities and a separate single goal for women have been established. The Contractor, however, is required to provide equal employment opportunity and to take affirmative action for all minority groups, both male and female, and all women, both minority and non-minority. Consequently, the Contractor may be in violation of the Executive Order if a particular group is employed in a substantially disparate manner (for example, even though the Contractor has achieved its goals for women generally, the Contractor may be in violation of the Executive Order if a specific minority group of women is underutilized).

10. The Contractor shall not use the goals and timetables or affirmative action standards to discriminate against any person because of race, color, religion, sex, or national origin.

11. The Contractor shall not enter into any subcontract with any person or firm debarred from Government contracts pursuant to Executive Order 11246.

12. The Contractor shall carry out such sanctions and penalties for violation of these specifications and of the Equal Opportunity Clause, including suspension, termination and cancellation of existing subcontracts as may be imposed or ordered pursuant to Executive Order 11246, as amended, and its implementing regulations, by the Office of Federal Contract Programs. Any contractor who fails to carry out such sanctions and penalties shall be in violation of these specifications and Executive Order 11246, as amended.

13. The Contractor, in fulfilling its obligations under these specifications, shall implement specific affirmative action steps, at least as extensive as those standards prescribed in paragraph 7 of these specifications, so as to achieve maximum results from its efforts to ensure equal employment opportunity. If the Contractor fails to comply with the requirements of the Executive Order, the implementing regulations, or these specifications, the Director shall proceed in accordance with 41 CFR 60-4.8.

14. The Contractor shall designate a responsible official to monitor all employment related activity to ensure that the company EEO policy is being carried out, to submit reports relating to the provisions hereof as may be required by the Government and to keep records. Records shall at least include for each employee the name, address, telephone numbers, construction trade, union affiliation if any, employee identification number when assigned, social security number, race, sex, status (e.g., mechanic, apprentice, trainee, helper, or laborer), dates of changes in status, hours worked per week in the indicated trade, rate of pay, and locations at which the work was performed. Records shall be maintained in an easily understandable and retrievable form; however, to the degree that existing records satisfy this requirement, contractors shall not be required to maintain separate records.

15. Nothing herein provided shall be construed as a limitation upon the application of other laws which establish different standards of compliance or upon the application of requirements for the hiring of local or other area residents (e.g., those under the Public Works Employment Act of 1977 and the Community Development Block Grant Program).
NOTICE OF REQUIREMENT FOR AFFIRMATIVE ACTION TO ENSURE EQUAL EMPLOYMENT OPPORTUNITY (EXECUTIVE ORDER 11246) (43 FR 14895)

1. The Offeror’s or Bidder’s attention is called to the “Equal Opportunity Clause” and the “Standard Federal Equal Employment Opportunity Construction Contract Specifications” set forth herein.

2. The goals and timetables for minority and female participation expressed in percentage terms for the Contractor’s aggregate workforce in each trade on all construction work in the covered areas, are as follows:

GOALS FOR FEMALE PARTICIPATION

APPENDIX A
(43 FR 19473)

The following goals and timetables for female utilization shall be included in all Federal and federally assisted construction contracts and subcontracts in excess of $10,000. The goals are applicable to the contractor’s aggregate on-site construction workforce whether or not part of that workforce is performing work on a Federal or federally-assisted construction contract or subcontract. Area covered: Goals for Women apply nationwide.

Goals and timetables

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<th>Timetable</th>
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<td>4-1-80 Until Further Notice</td>
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GOALS FOR MINORITY PARTICIPATION

Appendix B-80

Until further notice, the following goals for minority utilization in each construction craft and trade shall be included in all Federal or federally assisted construction contracts and subcontracts in excess of $10,000 to be performed in the respective geographical areas. The goals are applicable to each nonexempt contractor’s total onsite construction workforce, regardless of whether or not part of that workforce is performing work on a Federal, federally assisted or non-federally related project, contact or subcontract.
Construction contractors which are participating in an approved Hometown Plan (see 41 CFR 60-4-5) are required to comply with the goals of the Hometown Plan with regard to construction work they perform in the areas covered by the Hometown Plan. With regard to all their other covered construction work, such contractors are required to comply with the applicable SMSA or EA goal contained in this appendix B-80.

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<th>State</th>
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   Meriwether; GA Quitman; GA Schley; GA
   Stewart; GA Sumter; GA Talbot; GA Troup;
   GA Webster

038 Macon, GA:
   SMSA Counties:
   4680 Macon, GA ............................................................. 27.5
   GA Bibb; GA Houston; GA Jones; GA Twiggs
   Non-SMSA Counties .......................................................... 31.7
      GA Baldwin; GA Bleckley; Crawford; GA Crisp;
      GA Dodge; GA Dooly; GA Hancock; GA Johnson;
      GA Laurens; GA Macon; GA Monroe; GA Peach;
      GA Pulaski; GA Putman; GA Taylor; GA Telfair;
      GA Treutl; GA Washington; GA Wheeler;
      GA Wilcox; GA Wilkinson

039 Savannah, GA:
   SMSA Counties:
   7520 Savannah, GA .......................................................... 30.6
   GA Bryan; GA Chatham; GA Effingham
   Non-SMSA Counties .......................................................... 29.8
      GA Appling; GA Atkinson;
      GA Bacon, GA Bulloch; GA Candler; GA
      Coffee; GA Evans; GA Jeff Davis; GA Liberty;
      GA Long; GA McIntosh; GA Montgomery; GA
      Screven; GA Tattnall; GA Toombs; GA Wayne;
      SC Beaufort; SC Hampton; SC Jasper

040 Albany, GA:
   SMSA Counties:
   0120 Albany, GA ............................................................ 32.1
   GA Dougherty; GA Lee
   Non-SMSA Counties .......................................................... 31.1
      GA Baker; GA Ben Hill; GA Berrien; GA
      Brooks; GA Calhoun; GA Clay; GA Clinch;
      GA Colquitt; GA Cook; GA Decatur; GA
      Early; GA Echols; GA Grady; GA Irwin; GA
      Lanier; GA Lowndes; GA Miller; GA Mitchell;
      GA Randolph; GA Seminole; GA Terrell; GA
      Thomas; GA Tift; GA Turner; GA Worth

Florida:
041 Jacksonville FL:
   Non-SMSA Counties .......................................................... 22.2
      GA Brantley; GA Camden; GA Charlton; GA Glynn; GA Pierce; GA Ware
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA
DISADVANTAGED BUSINESS ENTERPRISE PROGRAM

CRITERIA FOR ACCEPTABILITY

The purpose of this special provision is to establish criteria for acceptability of DBE firms for work performed on this contract. The intent is to ensure all participation counted toward fulfillment of the DBE goals is (1) real and substantial, (2) actually performed by viable, independent DBE owned firms, and (3) in accordance with the spirit of the applicable laws and regulations.

The policy of the Georgia Department of Transportation is to ensure compliance with Title VI of the Civil Rights Act of 1964, 49 Code of Federal Register, Part 26 and related statutes and regulations in all program activities.

To this end the Georgia Department of Transportation shall not discriminate on the basis of race, color, sex or national origin in the award, administration and performance of any Georgia Department of Transportation assisted contract or in the administration of its Disadvantaged Business Enterprise Program. The Georgia Department of Transportation shall take all necessary and reasonable steps to ensure nondiscrimination.

DBE payments and commitments for Federal-aid projects shall be separate and distinct and cannot be transferred or combined in any matter.

The DBE Goal specified in the contract will be a percentage representing the DBE Race Conscious Participation. The Contractor will strive to achieve an additional percentage, cumulatively amounting but not limited to 2 percent in his/her contracts for all projects during the course of the current State Fiscal Year, in order to meet the overall Georgia Department of Transportation DBE goal.
**DBE PROCEDURES:** The Contractor shall develop techniques to facilitate DBE participation in subcontracting activities. These techniques include:

(A) Arranging solicitations, time for the presentation of quotes, quantities, specifications, and delivery schedules to facilitate the participation of DBEs.

(B) Providing assistance to DBEs in overcoming barriers such as the inability obtaining bonding, financing, or technical assistance.

**DBE DIRECTORY:** The Department has available a directory or source list to facilitate identifying DBEs with capabilities relevant to general contracting requirements and to particular solicitations. The Department will make the directory available to bidders and proposers in their efforts to meet the DBE requirements. The directory or listing includes firms which the Department has certified to be eligible DBEs in accordance with 49 CFR Part 26.

**GOAL FOR PARTICIPATION:** If a percentage goal for DBE participation in this contract is set forth elsewhere in this proposal, the Contractor shall complete the DBE GOALS Form included in the proposal. The Contractor is encouraged to make every effort to achieve the goal set by the Department. However, if the Contractor cannot find sufficient DBE participants to meet the goal established by the Department, the Department will consider for award a proposal with less participation than the established goal if:

(A) The bidder can demonstrate no greater participation could be obtained. This should be well documented by demonstrating the Contractor’s actions through good faith efforts.

The following is a list of types of actions which the Department will consider as part of the Contractor’s good faith efforts to obtain DBE participation. This is not intended to be a mandatory checklist nor intended to be exclusive or exhaustive. Other factors or types of efforts may be relevant in appropriate cases.

(1) Soliciting through all reasonable and available means (e.g. attendance at pre-bid meetings, advertising and/or written notices) the interest of all certified DBEs who have the capability to perform the work of the contract. The Contractor must solicit this interest within sufficient time to allow the DBEs to respond to the
solicitation. The Contractor must determine with certainty if the DBEs are interested by taking appropriate steps to follow up initial solicitations.

(2) Selecting portions of the work to be performed by DBEs in order to increase the likelihood the DBE goals will be achieved. This includes, where appropriate, breaking out contract work items into economically feasible units to facilitate DBE participation, even when the Contractor might otherwise prefer to perform these work items with its own forces.

(3) Providing interested DBEs with adequate information about the plans, specifications, and requirements of the contract in a timely manner to assist DBEs participants in responding to a solicitation.

(4) (a) Negotiating in good faith with interested DBEs. Contractor(s) are responsible to make a portion of the work available to DBE subcontractors and suppliers and to select those portions of the work or material needs consistent with the available DBE subcontractors and suppliers, so as to facilitate DBE participation. Evidence of such negotiation includes the names, addresses, and telephone numbers of DBEs that were considered; a description of the information provided regarding the plans and specifications for the work selected for subcontracting; and evidence as to why additional agreements could not be reached for DBEs to perform the work.

(b) Contractor(s) using good business judgment would consider a number of factors in negotiating with subcontractors, including DBE subcontractors, and would take a firm’s price and capabilities as well as contract goals into consideration. However, the fact there may be some additional costs involved in finding and using DBEs is not in itself sufficient reason for a bidder’s failure to meet the contract DBE goal, as long as such costs are reasonable. Also,
the ability or desire of a Contractor to perform the work of a contract with its own organization does not relieve the Contractor of the responsibility to make good faith efforts. Contractors are not, however, required to accept higher quotes from DBEs if the price difference is excessive or unreasonable.

(5) Not rejecting DBEs as being unqualified without sound reasons based on a thorough investigation of their capabilities. The Contractor’s standing within its industry, membership in specific groups, organizations, or associations and political or social affiliations (for example union vs. nonunion employee status) are not legitimate causes for the rejection or non solicitation of bids in the Contractor’s efforts to meet the project goal.

(6) Making efforts to assist interested DBEs in obtaining bonding, lines of credit, or insurance as required by the contractor.

(7) Making efforts to assist interested DBEs in obtaining necessary equipment, supplies, materials, or related assistance or services.

(8) Effectively using the services of available minority/women community organizations; minority/women Contractors’ groups; local, state, and Federal minority/women business assistance offices; and other organizations as allowed on a case-by-case basis to provide assistance in the recruitment and placement of DBE’s.

(B) The participation proposed by the low bidder is not substantially less than the participation proposed by the other bidders on the same contract.

If no percentage goal is set forth in the proposal, the contractor may enter a proposed DBE participation. This voluntary DBE participation will count as race neutral DBE participation. Prime Contractor shall report race-neutral participation in accordance with the DBE Quarterly Report requirements shown in this document.

To be eligible for award of this contract,

All bidders will be required to submit the following information to the Department by the close of business on the 3rd working day following opening of the bid as a matter of bidder responsibility.
(1) The names and addresses of DBE firms committed to participate in the Contract;
(2) A description of the work each DBE will perform;
(3) The dollar amount of the participation of each DBE firm participating;
(4) Written documentation of the bidder’s commitment to use a DBE subcontractor whose participation it submits to meet a contract goal;
(5) Written confirmation from the DBE committed to participating in the contract, as provided in the prime contractor’s commitment.
(6) If the contract goal is not met, evidence of good faith efforts must be provided.

Failure by a bidder to furnish the above information may subject the bid to disqualification. Also failure by the bidder to submit satisfactory evidence of good faith efforts may subject the bid to disqualification.

Award of a contract by the Department to a Prime Contractor who has listed DBE participants with the bid may not constitute final approval by the Department of the listed DBE. The Department reserves the right to approve or disapprove a Disadvantaged firm after a review of the Disadvantaged firm’s proposal participation. Payment to the Contractor under the contract may be withheld until final approval of the listed DBEs is granted by the Department.

If the Contractor desires to substitute a DBE in lieu of those listed in the proposal, a letter of concurrence shall be required from the listed DBE prior to approval of the substitution, unless this requirement is waived by the Department.

Agreements between bidder and a DBE in which promises not to provide Subcontracting quotations to other bidders are prohibited.

**DEFINITION:** For the purposes of this provision, the following definitions will apply:

Disadvantaged Business Enterprise or DBE means a for-profit small business concern –

(1) Ensuring at least 51 percent owned by one or more individuals who are both socially and economically disadvantaged or, in the case of a corporation, in which 51 percent of the stock is owned by one or more such individuals; and
(2) Whose management and daily business operations are controlled by one or more of the socially and economically disadvantaged individuals who own the business.

**Good Faith Efforts** means efforts to achieve a DBE goal or other requirement of this part which, by their scope, intensity, and appropriateness to the objective, can reasonably be expected to fulfill the program requirement.

**Joint Venture** means an association of a DBE firm and one or more other firms to carry out a single, for-profit business enterprise, for which the parties combine their property, capital, efforts, skills and knowledge, and in which the DBE is responsible for a distinct, clearly defined portion of the work of the contract and whose share in the capital contribution, control, management, risks, and profits of the joint venture are commensurate with its ownership interest.

**Socially and Economically Disadvantaged Individual** means any individual who is a citizen (or lawfully admitted permanent resident) of the United States and who is –

(1) Any individual who the Department finds to be a socially and economically disadvantaged individual on a case-by-case basis.

(2) Any individual in the following groups, members of which are reputedly presumed to be socially and economically disadvantaged.

(i) “Black Americans,” which includes persons having origins, in any of the Black racial groups of Africa;

(ii) “Hispanic Americans,” which includes persons of Mexican, Puerto Rican, Cuban, Dominican, Central or South American, or other Spanish or Portuguese culture or origin, regardless of race;

(iii) “Native Americans,” which includes persons who are American Indians, Eskimos, Aleuts, or Native Hawaiians;

(iv) “Asian-Pacific Americans,” which includes persons whose origins are from Japan, China, Taiwan, Korea, Burma (Myanmar), Vietnam, Laos, Cambodia (Kampuchea), Thailand, Malaysia, Indonesia, the Philippines, Brunei, Samoa, Guam, the U.S. Trust Territories of the Pacific Islands (Republic of Palau), the
Commonwealth of the Northern Marianas Islands, Macao, Fiji, Tonga, Kiribati, Juvalu, Nauru, Federated States of Micronesia, or Hong Kong;

(v) “Subcontinent Asian Americans,” which includes persons whose origins are from India, Pakistan, Bangladesh, Bhutan, the Maldives Islands, Nepal or Sri Lanka;

(vi) Women;

(vii) Any additional groups whose members are designated as socially and economically disadvantaged by the SBA, at such time as the SBA designation becomes effective.

(3) GDOT will presume that such persons are socially and economically disadvantaged only to the extent permitted by applicable federal law.

Race-conscious measure is one focused specifically on assisting only DBEs, including women-owned DBEs.

Race-neutral measure is one being, or can be, used to assist all small businesses. For the purposes of this part, race-neutral includes gender-neutrality.

DISCRIMINATION PROHIBITED: No person shall be excluded from participation in, denied the benefits of, or otherwise discriminated against in connection with the award and performance of this contract on the grounds of race, color, sex or national origin.

The following assurance becomes a part of this contract and must be included in and made a part of each subcontract the prime contractor enters into with their subcontractors (49 CFR 26.13):

“The contractor, and/or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR Part 26 in the award and administration of DOT – assisted contracts. Failure by the contractor to carry out these requirements is (breach) of this contract which may result in the termination of this contract or such other remedy as the Department deems appropriate.
**Failure to Achieve Requirements:** Periodic reviews shall be made by the Department to determine the extent of compliance with the requirements set forth in this provision. If the Contractor is found to be in noncompliance, further payments for any work performed may be withheld until corrective action is taken. If corrective action is not taken, it may result in termination of this contract.

Participation will be counted toward fulfillment of the DBE goal as follows:

(A) When a DBE participates in a contract, the Contractor counts only the value of the work actually performed by the DBE toward DBE goals.

(1) Count the entire amount of the portion of a construction contract (or other contract not covered by paragraph (A) (2) of this section) performed by the DBE’s own forces. Include the cost of supplies and materials obtained by the DBE for the work of the contract, including supplies purchased or equipment leased by the DBE (except supplies and equipment the DBE subcontractor purchases or leases from the prime contractor or its affiliate).

(2) Count the entire amount of fees or commissions charged by a DBE firm for providing a bona fide service, such as professional, technical consultant, or managerial services, or for providing bonds or insurance specifically required for the performance of a DOT-assisted contract, toward DBE goals, provided the Department determines the fee is reasonable and not excessive as compared with fees customarily allowed for similar services.

(3) When a DBE subcontracts part of the work of its contract to another firm, the value of the subcontracted work may be counted toward DBE goals only if the DBE’s subcontractor is itself a DBE. Work that a DBE subcontracts to a non-DBE firm does not count toward DBE goals.

(B) When a DBE performs as a participant in a joint venture, count a portion of the total dollar value of the contract equal to the distinct, clearly defined portion of
the work of the contract the DBE performs with own forces toward DBE goals.

(C) Count expenditures to a DBE contractor toward DBE goals only if the DBE is performing a commercially useful function on that contract.

(1) A DBE performs a commercially useful function when responsible for execution of the work of the contract and carrying out responsibilities by actually performing, managing, and supervising the work involved. To perform a commercially useful function, the DBE must also be responsible, with respect to materials and supplies used on the contract, for negotiating price, determining quality and quantity, ordering the material, and installing (where applicable) and paying for the material itself.

(2) A DBE does not perform a commercially useful function if their role is limited to being an extra participant in a transaction, contract, or project through which funds are passed in order to obtain the appearance of DBE participation.

(3) If a DBE does not perform or exercise responsibility for at least 30 percent of the total cost of their contract with their own work force, or the DBE subcontracts a greater portion of the work of a contract than would be expected on the basis of normal industry practice for the type of work involved, the Department will presume the DBE is not performing a commercially useful function.

(4) When a DBE is presumed not to be performing a commercially useful function as provided in paragraph (C) (3) of this section, the DBE may present evidence to rebut this presumption.

(5) The Department’s decisions on commercially useful function matters are subject to review by the US DOT, but are administratively appealable to the US DOT.

(D) The following factors are to be used in determining whether a DBE trucking company is performing a commercially useful function:

(1) The DBE must be responsible for the management and
supervision of the entire trucking operation for which they are responsible on a particular contract, and there cannot be a contrived arrangement for the purpose of meeting DBE goals.

(2) The DBE must itself own and operate at least one fully licensed, insured, and operational truck used on the contract.

(3) The DBE receives credit for the total value of the transportation services it provides on the contract using trucks it owns, insures, and operates using drivers it employs.

(4) The DBE may lease trucks from another DBE firm, including an owner/operator who is certified as a DBE. The DBE who leases trucks from another DBE receives credit for the total value of the transportation services the lessee DBE provided on the contract.

(5) The DBE may also lease trucks from a non-DBE and is entitled to credit only for the fee or commission it receives as a result of the lease arrangement. The DBE does not receive credit for the total value of the transportation services provided by the lessee, since these services are not provided by a DBE.

(6) For purposes of this paragraph (D), a lease must indicate the DBE has exclusive use of and control over the truck. This does not preclude the leased truck from working for others during the term of the lease with the consent of the DBE, so long as the lease gives the DBE absolute priority for use of the leased truck. Leased trucks must display the name and identification number of the DBE.

(E) Count expenditures with DBEs for materials or supplies toward DBE goals as provided in the following:

(1) (i) If the materials or supplies are obtained from a DBE manufacturer, count 100 percent of the cost of the materials or supplies toward DBE goals.

(ii) For purposes of this paragraph, a manufacturer is a firm that operates or maintains a factory or establishment that produces, on the premises, the materials, supplies, articles, or equipment required under the contract and of the general character...
described by the specifications.

(2) (i) If the materials or supplies are obtained from a DBE regular dealer, count 60 percent of the cost of the materials or supplies toward DBE goals.

(ii) For purposes of this section, a regular dealer is a firm owning, operating, or maintaining a store, warehouse, or other establishment in which the materials, supplies, articles or equipment of the general character described by the specifications and required under the contract are bought, kept in stock, and regularly sold or leased to the public in the usual course of business.

(A) To be a regular dealer, the firm must be an established, regular business engaging, as its principal business and under its own name, in the purchase and sale or lease of the products in question.

(B) A person may be a regular dealer in such bulk items as petroleum products, steel, cement, gravel, stone, or asphalt without owning, operating, or maintaining a place of business as provided in this paragraph (E)(2)(ii) if the person both owns and operates distribution equipment for the products. Any supplementing of regular dealers’ own distribution equipment shall be by a long-term lease agreement and not on an ad hoc or contract-by-contract basis.

(C) Packagers, brokers, manufacturers’ representatives, or other persons who arrange or expedite transactions are not regular dealers within the meaning of this paragraph (E)(2).

(3) With respect to materials or supplies purchased from a DBE which is neither a manufacturer nor a regular dealer, count the entire amount of fees or commissions charged for assistance in the procurement of the materials and supplies, or fees or transportation charges for the delivery of materials or supplies required on a job site, toward DBE goals, provided you determine the fees to be reasonable and not excessive as compared with fees customarily allowed for similar services. Do not count any portion of the cost of the materials and supplies themselves toward DBE goals, however.
(4) Do not count the participation of a DBE subcontractor toward the prime contractor’s DBE achievements until the amount being counted toward the goal has been paid to the DBE.

(5) No participation will be counted not in compliance with Special Provision entitled “Criteria for Acceptability” which is a part of this contract or with any provisions included in 49 CFR Part 26.

(6) If the contract amount overruns, the contractor will not be required to increase the dollar amount of DBE participation. If the contract amount under runs, the contractor will not be allowed to under run the dollar amount of DBE participation except when the DBE subcontracted items themselves under run.

REPORTS

A: The contractor shall submit a “DBE Participation Report” on this contract quarterly which shall include the following:

1. The name of each DBE participating in the contract.
2. A description of the work to be performed, materials, supplies, and services provided by each DBE.
3. Whether each DBE is a supplier, subcontractor, owner/operator, or other.
4. The dollar value of each DBE subcontract or supply agreement.
5. The actual payment to date of each DBE participating in the contract.
6. The report shall be updated by the Prime Contractor whenever the approved DBE has performed a portion of the work that has been designated for the contract. Copies of this report should be transmitted promptly to the Engineer. Failure to submit the report with 30 calendar days following the end of the quarter may cause payment to the contractor to be withheld.
7. The Prime Contractor shall notify the Project Engineer at least 24 hours prior to the time the DBE commences working on the project. The DBE must furnish supervision of the DBE portion of the work, and the person responsible for this supervision must report to the Project Engineer when they begin work on the project. They must also inform the project engineer when their forces will be doing work on the project.
B. In order to comply with 49 CFR 26.11, the Prime Contractor shall submit documentation regarding all payments made from the Prime to all DBE subcontractors on federal aid projects in the form of copies of cancelled checks or notarized electronic documentation which validates said payments made on the DBE Quarterly Participation Reports. This information shall be required quarterly and submitted with the DBE Quarterly Participation Report.

C. Failure to respond within the time allowed in the request will be grounds for withholding all payments on all Contracts.

**SUBSTITUTION OF DBEs:** The Contractor shall make reasonable efforts to replace a DBE Subcontractor unable to perform work for any reason with another DBE. The Department shall approve all substitutions of Subcontractors in order to ensure the substitute firms are eligible DBEs.

**CERTIFICATION OF DBEs:** To ensure the DBE Program benefits only firms owned and controlled by Disadvantaged Individuals, the Department shall certify the eligibility of DBEs and joint ventures involving DBEs named by bidders.

Questions concerning DBE Certification/Criteria should be directed to the EEO Office at (404) 631-1972.
SPECIAL PROVISION

PROMPT PAYMENT:
Prime Contractors, who sublet a portion of their work, shall pay their subcontractors for satisfactory performance of their contracts no later then 10 calendar days from receipt of each payment made to them.

Any delay or postponement of payment among the parties may take place only for good cause with prior written approval from the Department.

If the contractor is found to be in noncompliance with these provisions, it shall constitute a breach of contract and further payments for any work performed may be withheld until corrective action is taken. If corrective action is not taken, it may result in termination of the contract.

All subcontract agreements shall contain this requirement.
REQUIRED CONTRACT PROVISIONS FOR FEDERAL-AID CONTRACTS

BUY AMERICA

Revised: March 25, 1992
Revised: January 7, 1994
Revised: June 9, 1995
First Use 2001 Specifications: November 1, 2002

All manufacturing processes for steel and iron materials and steel and iron coatings permanently incorporated into this project must occur in the United States of America. However, pig iron and processed, pelletized, or reduced iron ore used in the production of these products may be manufactured outside the United States.

This requirement, however, does not prevent a minimal use of foreign materials and coatings, provided the cost of materials and coatings used does not exceed one-tenth of one percent (0.1 percent) of the total contract cost or $2,500.00, whichever is greater.

NOTE: Coatings include: epoxy coating, galvanizing, painting and any other coating that protects or enhances the value of the material.

CONVICT PRODUCED MATERIALS

March 25, 1992
Revised: September 6, 1993
First Use 2001 Specifications: November 1, 2002

Materials produced by convict labor after July 1, 1991, may not be used for Federal-Aid highway construction projects unless it meets the following criteria:

1. The materials must be produced by convicts who are on parole, supervised release or probation from a prison; or,

2. If produced in a qualified prison facility, the amount of such materials produced in any 12-month period shall not exceed the amount produced in such facility for such construction during the 12-month period ending July 1, 1987. A qualified prison is defined as one producing convict made materials prior to July 1, 1987.
Utility companies having known facilities that conflict with the construction of this project will be directed by the Department to adjust or relocate their facilities and will be notified of the contract award.

Conform to all the requirements of the Specifications as they relate to cooperation with utility owners and the protection of utility installations that exist on the project. Refer to the requirements of Section 107, Legal Regulations and Responsibility to the Public, with particular attention to Subsection 107.21.

Coordinate The Work with any work to be performed by others in any right of way clearance and arrange a schedule of operations that will allow for completion of the Project within the specified contract time. Where stage construction is required, notify the utility owner when each stage of work is completed and the site is available for utility work to proceed.

Information concerning utility facilities known to exist within the project limits, including the list of owners, is available for reference.

Under Georgia Code Section 32-6-171, utilities are required to remove or relocate their facilities. The Department is required to give the utility at least 60 days written notice directing the removal, relocation, or adjustment and the utility owner is required to begin work within the time specified in the utility’s work plan or revised work plan.

Upon request, copies of all agreements with utility companies having facilities on this project will be made available for examination by the Contractor at the Department’s District Office. Utility Adjustment Schedules, when submitted to the Department by the utilities, will be made available to the Contractor after the Notice to Contractors has been posted by the Office of Construction Bidding Administration. The Utility Adjustment Schedules are available on the Office of Construction Bidding Administration’s web site. Utility Adjustment Schedules may be included with the Utility Special Provision in the Contract Proposal on select projects. The Contractor is responsible for considering in its bid all existing and proposed utility locations and the removals, relocations, and adjustments specified in the Utility’s Work Plan.

For this Project, Utility Owners that are required to remove, relocate, or adjust their facility to accommodate the construction of this Project may be liable to the Contractor...
for damages or delay costs resulting from the Utility Owner’s failure to clear conflicts within the time specified in the approved Utility Work Plan. If the Utility Owner is unable to submit and obtain Department approval of a revised Work Plan or fails to complete the removal, relocation, or adjustment of its facilities in accordance with the approved Work Plan, the Utility Owner may be liable to the Department, or the Contractor, for damages or delay costs.

In accordance with Subsection 105.06 of the Specifications, the Department is not liable for payment of any claims due to utility delays, inconvenience or damage sustained by the Contractor due to interference of any utilities or appurtenances, or the operation of moving them.

Whenever the Contractor considers that it is or will be entitled to damages or delay costs from the Utility Owner in accordance with O.C.G.A. 32-6-171, the Contractor shall provide written notice to the Utility Owner and the Department within ten (10) days from the time of the dispute or potential dispute is identified. The Contractor shall follow the Procedures for Utility Damages or Delay Costs outlined in the latest edition of The Utility Accommodation Policy and Standards Manual. Failure to follow the above will result in waiver of the Contractor’s claim against the Utility Owner for damages or delay costs.

In accordance with Subsection 107.21.G delays by utilities will continue to be considered by the Department in charging Contract Time. For purposes of applying provisions of this paragraph, railroads and the Metropolitan Atlanta Rapid Transit Authority (MARTA) are considered utilities.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

SECTION 102-Bidding Requirements and Conditions

The use of the Electronic Bid Bond Form in Expedite will be accepted by the Department for compliance with the Notice To Contractors requirement to utilize the “Bid Bond Form DOT 564-Rev. Dec. 13, 2004”.

Office of Contract Administration
Delete Subsection 102.01 and Substitute the following:

**102.01 Prequalification of Bidders**

Before submitting a bid in excess of $2,000,000, the Bidder shall have been prequalified with the Department and received a Certificate of Qualification in accordance with the Rules and Regulations approved and adopted by the State Transportation Board. Bidders submitting bids of $2,000,000 or less may be exempt from prequalification requirements. In addition, the aggregate total amount a Non-prequalified Bidder may have under contract shall not exceed $4,000,000. Bidders intending to consistently submit Proposals shall prequalify at least once a year. However, qualifications may be changed during that period upon the submission of additional favorable reports or upon unsatisfactory performance. In addition, the Department reserves the right at any time to require the Contractor to furnish a current financial and experience statement.

Delete Subsection 102.03 and Substitute the following:

**102.03 Contents of Proposal Forms**

Upon request, the Department will furnish the prospective Bidder with a Valid for Bidding Proposal Form. Only one Valid for Bidding Proposal will be issued to a prospective Bidder for each proposal being considered to be bid. This form will state the location and description of the contemplated construction and will show the approximate estimate of the various quantities and kinds of work to be performed or materials to be furnished, and will have a schedule of Items for which Unit Bid prices are invited. The Proposal Form will state the time in which The Work must be completed, the amount of the Proposal Guaranty, and the date of the opening of Proposals. The Form will also include any Special Provisions or requirements that vary from or are not contained in the Specifications. Also included with each Proposal Form will be a Non-Collusion Certificate. All papers bound
with or attached to the Proposal Form are considered a part thereof and must not be detached or altered when
the Proposal is submitted. The Plans, Specifications, and other documents designated in the Proposal Form will
be considered a part of the Proposal whether attached or not. The prospective Bidder will be required to pay the
Department the sum stated in the Notice to Contractors for each Valid for Bidding Proposal Form and each set
of Plans.

Delete Subsection 102.06 and Substitute the following:

102.06 Preparation of Proposal

The Bidder shall submit its Proposal on the form furnished by the Department (GADOT). The blank spaces on the
Proposal shall be filled in correctly for each Pay Item (except alternate items) and the Bidder shall write in ink
the Unit Price or a Lump Sum Price as called for in the Proposal for each Pay Item listed therein. In addition, the
Bidder shall also show the products of the respective Unit Prices and quantities and the total amount of the Bid
by adding the amounts of all Bid Items. In the event of a discrepancy in any of the figures, the Unit Price will
govern and the Bid will be recalculated.

In addition, the Bidder shall submit a technical proposal which shall include, but is not limited to, the design
build firm’s detailed project schedule (including all submittals shown in Table 4-1 of Special Provision 999, and
estimated review periods, and in other areas of Special Provision 999 where submittals are mentioned),
permit requirements, expected Released for Construction, total contract time, mobilization assumptions, a
detailed Maintenance of Traffic (MOT) plan, construction staging assumptions, as well as, a detailed estimate
with all material quantities and price assumptions used to form the basis of the bid. The Bidder shall also
clearly document all assumptions in this technical proposal. There are no page limit restrictions for the
technical proposal.

As part of the detailed estimate (with all material quantities and price assumptions used to form the basis of
the bid) the Bidder shall clearly identify the detailed costs for both the design and construction necessary for
the City of Woodstock’s utility relocations. The Bidder shall provide a cost which reasonably and accurately
reflects the design and relocation of these facilities. Failure to provide this information in the technical
proposal will deem the Bidder non-responsive.

The items required in the technical proposal are the minimum requirements. The intent of the technical
proposal is to provide the Department insight into the Contractor’s approach to design and construction,
scheduling assumptions, and with the assumed quantities and costs used to formulate the bid. As noted in
section 999.1.A.2 “Bids on this project shall reflect designing and constructing the project as shown in the
Scope (999.1.A.3) and applicable portions of the Plans Package. No exceptions shall be assumed by the
Contractor. However, alternative proposals on portions of the work will be entertained once the project is
awarded.” Therefore, no deviations shall be included in the bid or technical proposal.

In the case of Alternate items, Unit Prices shall be entered for only one alternate.

The Non-Collusion Certificate on the Department’s standard form included in the Proposal shall be executed.
The Certificate of Current Capacity shall be executed under oath and substantiated by the report of Status of
Contracts on Hand.
The Construction Contractors Bid Opportunity List standard form shall be completed with the required information.

The Bidder shall purchase from the GADOT Office of Construction Bidding Administration, a Valid for Bidding Proposal Form for each Letting Call Order Number in which the Bidder intends to submit a bid.

If the Proposal is made by an individual, its name and post office address shall be shown; if by a partnership, the name and post office address of one member of the partnership shall be shown; if by a corporation, the Proposal shall show the name, title and business address of the officer signing the Proposal. The Bidder’s Proposal shall be signed in ink or by Digital Signature by the individual, by one or more members of a partnership, or by one or more of the officers of a corporation, whichever is applicable. In the event of a joint venture, the Proposal shall be signed in ink or by Digital Signature by each individual involved, by each partnership through one or more of its members, or by each corporation through one or more officers of the corporation, whichever is applicable. Proposals not properly signed may be disqualified and rejected.

All bids in excess of $500,000 shall be submitted using the GADOT/AASHTO (American Association of State Highway and Transportation Officials) Electronic Bidding System (Expedite). When submitting a bid electronically, the Bidder’s Proposal shall consist of the Bid pages generated by the Expedite software including the Cover page, Bid Item pages, Disadvantaged Business Enterprise (DBE) pages (if applicable), Miscellaneous Data pages and the Signature page. By submitting a bid electronically, the Bidder acknowledges that all requirements included in the hard copy proposal, amendments, plans, Standard Specifications, and Supplemental Specifications are a part of the Bid and Contract.

The electronic bid shall be submitted by one of the following methods:

A. **Hand delivery of the electronic bid to the Department at the place specified in the Notice To Contractors.**

   The bid shall include the 3 ½ inch (90 mm) electronic diskette or CD Rom and the Bid pages described in paragraph seven, above.

B. **Electronic Bid Submission via the Internet and Bid Express™.**

   (Note: The Bidder shall secure an account and a valid Digital Signature from Bid Express™ (www.bidx.com) in order to use this method.

Instructions for preparing and submitting bids by these two methods are as follows:

A. **Hand Delivery of Bid to the Department**

   1. Access to the electronic bidding information is available on Bid Express™ at [www.bidx.com](http://www.bidx.com) and the GADOT Construction Bidding Administration Internet Web Site at [wwwb.dot.state.ga.us/dot/construction/contractsadm/index.shtml](http://wwwb.dot.state.ga.us/dot/construction/contractsadm/index.shtml).

   2. Electronic bids shall be prepared through the use of a computer controlled printer.

   3. The Bidder shall sign the electronic bid in the appropriate areas.

   4. When installing the Bid program the Bidder shall enter their vendor code in the following format: 2D0900. Before running the electronic bidding programs, the Bidder shall read the on-line help documentation for the Expedite software.
5. Zero (0) is considered to be a valid bid. The Bidder shall not enter 0 in any Unit Price field unless zero is the intended bid for that item.

6. All addenda shall be included in the electronic bid submitted.

7. For “Joint Bids” the Bidder shall select tools from the Windows Expedite menu and mark the electronic bid as “Joint Bid”.

8. The Bidder shall select tools and then check bid to check the bid and assure there are no errors prior to printing the electronic bid. After final printing, the Bidder may make changes to the electronic bid by indicating the changes in ink and initialing prior to submitting the bid.

9. Once the Bidder has completed the bid and made all desired changes, the diskette/CD Rom, a printout of the Cover sheet, Bid Item pages, DBE pages (if applicable), Miscellaneous Data pages, and Signature page shall be submitted to the Department. In case of a discrepancy between the diskette and the hard copy of the Bid Item pages, the hard copy will govern.

10. Electronic Bid pages shall be 8 ½ inch (216 mm) horizontal by 11 inch (279 mm) vertical. Bid information shall be placed across the horizontal width on each page.

11. The paper used for an electronic bid shall be of sufficient quality and durability to maintain clear and concise images and to withstand frequent handling.

12. If originally printed on continuous roll paper, electronic bids shall be separated before submitting the Bid to the Department.

13. All computer printed characters shall be legible. The Electronic Bid pages shall be submitted in the bid envelope provided.

14. The diskette shall be submitted in a separate sealed envelope from the Bid pages. The Bidder shall submit all electronic bids on one diskette/CD Rom. The envelope containing the diskette shall include the Bidders name and the Letting Call Order Numbers for which electronic bids are submitted.

B. Electronic Bid Submission Via The Internet And Bid Express™

1. Access to the electronic bidding information is available on Bid Express™ at www.bidx.com and the GADOT Construction Bidding Administration Internet Web Site at wwwb.dot.state.ga.us/dot/construction/contractsadm/index.shtml.

2. When installing the Bid program the Bidder shall enter their vendor code in the following format: 2DO900. Before running the electronic bidding programs, the Bidder shall read the on-line help documentation for the Expedite software.

3. Zero (0) is considered to be a valid bid. The Bidder shall not enter 0 in any Unit Price field unless zero is the intended bid for that item.

4. All addenda shall be included in the electronic bid submitted.

5. “Joint Bids” are allowed with Electronic Bid Submission via the Internet and Bid Express™

6. The Bidder shall select tools and then check bid from the Windows Expedite menu to check the bid and assure there are no errors prior to submitting the electronic bid. The electronic bid may be changed and resubmitted electronically to Bid Express™ as many times as desired prior to the advertised cutoff time specified in the Notice To Contractors. The last bid submitted for a given Letting Call Order Number prior to the cutoff time will be the Bid.
7. The Bidder shall make no claim against the Department in the event it is unable to submit its bid to Bid Express™ and/or Bid Express™ is unable to submit the bid(s) to the Department. The Department reserves the right to postpone the public reading of bids in the event of technical difficulties.

8. A fully executed Proposal Guaranty and Power of Attorney for each Letting Call Order Number bid shall be submitted by one of the following methods:

   A. Delivery to the GADOT Office of Construction Bidding Administration, Room 1113, in a sealed envelope by 12:00 noon on the day prior to the Bid Opening. Each Proposal Guaranty shall be clearly and legibly marked with the Letting Call Order Number.

   B. Electronic submission via the Internet and Bid Express™ by the time and date set in the Notice To Contractors for submission of Proposals.

   The Proposal Guaranty for a “Joint Bid” shall include the names of all Joint Venture parties involved in the bid.

Delete Subsection 102.07 and Substitute the following:

102.07 Rejection of Proposals

Proposals may be rejected as irregular if their consideration is conditioned upon the acceptance or rejection of other Proposals submitted by the same Bidder, if the Certificate of Current Capacity is not executed under Oath and substantiated, if a Unit Price is not shown for each Pay Item, or if they fail to comply with the EBS bidding requirements. In the case of alternate items, Unit Prices shall be entered for only one alternate. The Department reserves the right to disqualify and reject any Proposal that is not properly signed in accordance with the requisite of subsection 102.06.

A. Collusion

Any and all Proposals will be rejected if the Department believes that collusion exists among the Bidders and no participant in such collusion may submit future Proposals for the same work. The Department reserves the right to review and to refuse to consider any Proposal if the Bidder fails to execute the Non-Collusion Certificate.

B. Single Proposals

Only one Proposal from any person, partnership, or corporation under the same or different names shall be submitted on any Project.

C. Unbalanced Bids

Proposals may be rejected if any of the Unit Prices are obviously unbalanced. The Department will decide whether any Unit Prices are unbalanced either excessively above or below a reasonable cost analysis value determined by the Engineer, particularly if these unbalanced amounts are substantial and contrary to the interest of the Department.
D. Omissions and Alterations

Proposals may be rejected as irregular if they show any omissions, alterations of form, additions or conditions not called for, unauthorized alternate bids, erasures or changes not initialed, or other irregularities.

E. Debts

The Department reserves the right to reject Proposals from Bidders who have not paid or satisfactorily settled all legal debts due on other Contracts at the time Proposals are received.

F. Technicalities

The Department reserves the right to reject any and all Proposals and to waive technicalities at any time before the Contract has been signed by the Department.

G. Non-Prequalified Bidders

Proposals submitted in excess of $2,000,000 by non-prequalified contractors under Rule 672-5 of the Department’s Rules and Regulations Governing the Prequalification of Prospective Bidders will be disqualified and rejected.

H. Failure to List Disadvantaged Business Enterprise (DBE) Participants

If the contract has an established DBE goal, the Department reserves the right to reject and disqualify any proposal if the bidder has failed to list bona fide DBE participants with sufficient participation to achieve at least the established goal. The Department may consider for award a proposal with less participation than the established goal if both:

- The bidder can demonstrate that no greater participation could be obtained and;
- The participation proposed by the low bidder is not substantially less than the participation proposed by the other bidders on the same contract.

I. Non-responsive technical proposal

A proposal will only be considered non-responsive if it does not contain the information noted in paragraph 2 of section 102.06, and any other information necessary to clearly demonstrate those assumptions used to form the basis of the bid.

The technical proposal may be considered non-responsive if the bid or technical proposal contains any deviations from those items shown in the Scope (999.1.03) and applicable portions of the Plans Package.

Delete Subsection 102.09 and Substitute the following:

102.09 Delivery of Proposals

Each Proposal, together with the Proposal Guaranty, shall be submitted in a sealed envelope so marked as to identify its contents without being opened, unless submitted electronically (See Section 102.06). In addition, Six (6) copies of the Bidder’s technical proposal and one (1) cd or dvd containing an electronic copy of the Bidder’s technical proposal shall be submitted in a sealed envelope so marked as to identify its contents without being opened. Proposal forms are not transferable. Proposals will be received until the time and date set in the Notice To Contractors and shall be in the hands of the officials indicated by that time. Proposals
received after the advertised cutoff time established for submission of Proposals will be returned unopened to
the Bidder.

Delete Subsection 102.10 and Substitute the following:

102.10 Withdrawal or Revision of Proposals

Any Bidder may withdraw his Proposal by submitting, by telegram, letter, or facsimile transmission received
prior to the advertised cutoff time specified in the Notice To Contractors and verified by the Department, a
DEPARTMENT OF TRANSPORTATION BID PROPOSAL WITHDRAWAL FORM, completed by an authorized officer of
the company, whose signature is legally binding upon said company.

Any Bidder may submit a Bid change, by telegram, letter, or facsimile transmission received prior to the
advertised cutoff time specified in the Notice To Contractors and verified by the Department, completed by an
authorized officer of the company, whose signature is legally binding upon said company. In which case, the
Department will change the Bid at the time of opening and at such time will announce that a change was
received.

Add the following:

102.15 Submittal of “Certificate of Current Capacity” and “Status of Contracts on Hand”

The apparent low Bidder for each Letting Call Number shall submit the executed “Certificate of Current
Capacity” and the “Status of Contracts on Hand” to the GADOT Office of Construction Bidding Administration,
Room 1113, in a sealed envelope by 12:00 noon on the first working day after the Bid Opening.

If the “Certificate of Current Capacity” and the “Status of Contracts on Hand” are not delivered to the GADOT
Office of Construction Bidding Administration, Room 1113, in a sealed envelope by 12:00 noon on the first
working day after the Bid Opening, the Bid may be subject to disqualification.

Add the following:

102.16 Submittal of “Construction Contractors Bid Opportunity List”

All Bidders for each Letting Call Number shall submit the completed “Construction Contractors Bidding List” to
the GADOT Office of Construction Bidding Administration, Room 1113, in a sealed envelope by 12:00 noon on
the third working day after the Bid Opening as a matter of Bidder responsibility.

If the “Construction Contractors Bidding List” is not delivered to the GADOT Office of Construction Bidding
Administration, Room 1113, in a sealed envelope by 12:00 noon on the third working day after the Bid Opening,
the Bid may be subject to rejection.
Delete Subsection 104.08 and Substitute the following:

104.08 Value Engineering Proposals

A. Applicability

This Section applies to those cost reduction proposals initiated and developed by the Contractor for changing the Plans, Specifications, or other requirements of the Contract. These provisions do not apply unless the proposal submitted by the Contractor is specifically identified as being presented as a Value Engineering Proposal (VEP) and the Contract amount is in excess of $50,000.

The cost-reduction Proposals contemplated are those discretionary changes which would require a Supplemental Agreement modifying the Contract and would produce a savings to the Department by providing less costly items or methods than those specified in the Contract without impairing essential functions and characteristics including, but not limited to: service life, reliability, economy of operations, ease of maintenance, and safety, both during and after construction. Proposals must provide a product comparable to the original design at a lower cost or improved quality, or both. No proposals will be accepted that lower the quality of the project.

These provisions are applicable to the prime Contract and include all subcontracts.

B. Documentation

Value Engineering Proposals (VEP) will be processed in the same manner as prescribed for any other alterations of the Contract requiring a Supplemental Agreement.

As a minimum, the following information shall be submitted by the Contractor with each Value Engineering Proposal:

1. A description of the difference between the existing Contract requirement and the proposed change and the comparative advantages and disadvantages of each.
2. An itemization of the requirements of the Contract which must be changed and a recommendation of how to make such change (e.g., a suggested revision).
3. A detailed estimate of the cost of performing the work under the Contract and under the proposed change.
4. A prediction of any effects the proposed changes would have on other costs to the Department, including cost of related items and costs of maintenance and operation.
5. A statement of the time showing the last date by which an agreement for adoption of the proposed changes must be executed in order to obtain the maximum cost reduction during the remainder of the Contract, noting any effect on the Contract completion time or delivery schedule.
6. The dates of any previous or concurrent submissions of the Proposal, the Contract number(s) under which submitted, and the outcome or the result of the proposal in previous projects and any previous actions by the Department, if known.
7. A life-cycle cost analysis.

NOTE: If a VEP is similar to a change in the Plans or Specifications for the Project that is under consideration by the Department at the time said VEP is submitted, or if such VEP is based upon or similar to Standard Specifications, Special Provisions, or Standard Plans adopted by the Department after the advertisement of the Contract, the Engineer will not accept such proposal and the Department reserves the right to make such changes without compensation to the Contractor under the provisions of this Section.

Proposed changes in the basic design of a pavement type (e.g., rigid to flexible or vice versa) or pavement thickness will not be considered as an acceptable VEP. Proposed changes to base/subbase courses may be considered as an acceptable VEP. If design alternates are shown in the plans, the Department will not consider a VEP substituting a design alternate on which the Contractor could have bid for one on which the Contractor has bid. The Department reserves the right to reject any VEP submitted requiring additional Right-of-Way.

C. Submission

Value Engineering Proposals submitted by the Contractor will be processed as expeditiously as possible; however, the Department will not be liable for any delay in acting upon proposals submitted. The Contractor may withdraw, wholly or in part, any VEP not accepted by the Department within the time specified in Subsection 104.08.B.5.

D. Acceptance

The decision of the Engineer as to the acceptance or rejection of a VEP shall be final and shall not be subject to the provisions of Subsection 105.13, “Claims for Adjustments and Disputes.”

The Engineer may accept, in whole or in part, before work has been completed, any VEP submitted pursuant to this Subsection and not withdrawn by the Contractor by giving the Contractor written notice thereof reciting acceptance under this Subsection.

E. Notification

The Contractor will be notified in writing of the Department’s decision or rejection of each VEP submitted under these provisions. If a proposal is accepted, the necessary Contract modifications will be affected by execution of a Supplemental Agreement. Unless and until a VEP is affected by such Supplemental Agreement, the Contractor shall remain obligated to perform The Work in accordance with the terms of the existing Contract.

Supplemental Agreements made as a result of this Subsection will state that they are made pursuant to it.

F. Sharing

In the event a VEP submitted by the Contractor under this Subsection is accepted, the Supplemental Agreement effecting the necessary modifications will establish the net savings agreed upon and will provide for an adjustment in Contract Prices that will divide the net savings between the Contractor and the Department in accordance with the following provisions:

1. Division of net savings in Contract Price Adjustment:
   - 50 percent of the net savings to the Contractor.
   - 50 percent of the net savings to the Department.

2. The Department reserves the right to include in the agreement any conditions it deems appropriate for consideration, approval, and implementation of the VEP. The Contractor’s 50 percent of the net savings shall constitute the full compensation for effecting all changes pursuant to the agreement.

   Development costs incurred by the Contractor and review costs incurred by the Department shall not be considered in computing the net savings of the VEP.

3. Restrictions and Disclosures: Upon acceptance and implementation of any VEP, any restrictions imposed by the Contractor on its use or disclosure of the information submitted shall be void.

   The Department shall thereafter have the right to use, duplicate, and disclose, in whole or in any part, all data necessary in the utilization of the proposal.

Office of Construction
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Section 108—Prosecution and Progress

Delete the fifth paragraph from Subsection 108.01 and substitute the following:

No Subcontracts, or transfer of Contract, shall in any case release the Prime Contractor of his/her liability under the Contract and Bonds. No Subcontractor shall commence work in advance of the written approval of the Subcontract by the Department. Except for certain items exempted by the State Transportation Board, each Subcontractor shall be prequalified or registered with the Department. Each Subcontract for a Registered Subcontractor shall not exceed $1,000,000.00 and Subcontracts for Prequalified Contractors shall not exceed their current capacity. Prequalified or Registered Subcontractors shall be qualified or registered with the Department in accordance with Chapter 672-5 of the Rules and Regulations Governing the Prequalification of Prospective Bidders adopted by the State Transportation Board.
Delete subsection 108.08 in its entirety and substitute the following:

108.08 Failure or Delay in Completing Work on Time

Time is an essential element of the Contract, and any delay in the prosecution of The Work may inconvenience the public, obstruct traffic, or interfere with business. In addition to the aforementioned inconveniences, any delay in completion of The Work will always increase the cost of engineering. For this reason, it is important that The Work be pressed vigorously to completion. Should the Contractor or, in case of default, the Surety fail to complete The Work within the time stipulated in the Contract or within such extra time that may be allowed, charges shall be assessed against any money due or that may become due the Contractor in accordance with the following schedule:

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For each Calendar Day or Available Day, as specified, that any work shall remain uncompleted after the contract time specified for the completion of the Work required by the Contract, the sum specified in the Contract will be deducted from any money due the Contractor, not as a penalty, but as liquidated damages; provided however, that due account shall be taken of any adjustment of the contract time for completion of the work granted under the provisions of Subsection 108.07.E.

The Department may waive such portions of the liquidated damages as may accrue after the work is in condition for safe and convenient use by the traveling public.
A. Liquidated Damages

The amount of such charges is hereby agreed upon as fixed liquidated damages due the Department after the expiration of the time for completion specified in the Contract. The Contractor and his Surety shall be liable for liquidated damages in excess of the amount due the Contractor on the final payment.

These fixed liquidated damages are not established as a penalty but are calculated and agreed upon in advance by the Department and the Contractor due the uncertainty and impossibility of making a determination as to the actual and consequential damages which are incurred by the Department as a result of the failure on the part of the Contractor to complete The Work on time.

1. **Deduction From Partial Payments:** Liquidated damages, as they accrue, will be deducted from periodic partial payments.

2. **Deduction From Final Payment:** The full amount of liquidated damages will be deducted from final payment to the Contractor and/or his Surety.

3. **No Liquidated Damages Charged for Delay by the Department:** In case of default of the Contract and the subsequent completion of The Work by the Department as hereinafter provided, the Contractor and his Surety shall be liable for the liquidated damages under the Contract, but no liquidated damages shall be chargeable for any delay in the final completion of The Work by the Department due to any unreasonable action, negligence, omission, or delay of the Department. In any suit for the collection of or involving the assessment of liquidated damages, the reasonableness of the amount shall be presumed. The liquidated damages referred to herein are intended to be and are cumulative and shall be in addition to every other remedy now or hereafter enforceable at law, in equity, by statute, or under the Contract.

B. No Waiver of Department’s Rights

Permitting the Contractor to continue and finish The Work or any part of it after the expiration of the time allowed for completion or after any extension of time, shall not operate as a waiver of the rights of the Department under the Contract.

Office of Construction
Delete Subsection 109.07A. and Substitute the following:

A. General

At the end of each calendar month, the total value of Items complete in place will be estimated by the Engineer and certified for payment. Such estimate is approximate only and may not necessarily be based on detailed measurements. Value will be computed on the basis of Contract Item Unit Prices or on percentage of completion of Lump Sum Items.

When so requested by the Contractor and approved by the Engineer, Gross Earnings of $500,000.00 or more for work completed within the first 15 days of any month will be certified for payment on a semi-monthly basis subject to the conditions and provisions of Subsection 109.07.A, Subsection 109.07.B.6, Subsection 109.07.C, Subsection 109.07.D, Subsection 109.07.E, and Subsection 109.07.F.

Delete Subsection 109.07B.4 and Substitute the following:

No materials allowance will be made for a material when the requested allowance for such material is less than $50,000.00.
Delete Subsection 149.3.05.I and substitute the following:

I. Maintain the Stakes

After construction has begun in any segment of the Project, maintain the stakes that identify construction station numbers and locations as follows:

1. Ensure that stakes are placed at intervals not to exceed 200 ft (60 m) and use even, 100 ft (30 m) stations. On asphalt shoulder widening and earth shoulder reconstruction projects use mile post numbers when stations are not used.

Mark and flag stakes so that they are visible to DOT Project personnel in that segment of the Project until construction is complete.

Projects utilizing GPS controlled fine grading equipment, place stakes at intervals not to exceed 300 ft (91 m) on English projects and 100 m (310 ft) on metric projects. Use even, 100 ft (30 m) or 100 m (310 ft), stations.

2. During grading activities in fills or cuts over 20 ft (6 m), extend slope stakes up or down the slopes in intervals of 10 ft (3 m) or less to achieve an accurate cross section.

3. Denote the offset distance to the construction centerline on the station number stakes, when the station number is maintained in a location other than on the construction centerline. On asphalt shoulder widening and earth shoulder reconstruction projects use the offset to the edge of pavement on the stakes.
Add the following:

**150.01 GENERAL**

This section as supplemented by the Plans, Specifications, and Manual on Uniform Traffic Control Devices (MUTCD) shall be considered the Temporary Traffic Control (TTC) Plan. Activities shall consist of furnishing, installing, maintaining, and removing necessary traffic signs, pedestrian signs, barricades, lights, signals, cones, pavement markings and other traffic control devices and shall include flagging and other means for guidance and protection of vehicular and pedestrian traffic through the Work Zone. This Work shall include both maintaining existing devices and installing additional devices as necessary in construction work zones.

When any provisions of this Specification or the Plans do not meet the minimum requirements of the MUTCD, the MUTCD shall control. The 2003 Edition of the MUTCD shall be in effect for the duration of the project.

The needs and control of all road users (motorists, bicyclists and pedestrians within the highway right-of-way and easements, including persons with disabilities in accordance with the Americans with Disabilities Act of 1990 (ADA), Title II, Paragraph 35.130) through a Temporary Traffic Control (TTC) zone shall be an essential part of highway construction, utility work, maintenance operations and management of traffic incidents.

The Worksite Traffic Control Supervisor (WTCS) shall have a copy of Part VI of the MUTCD and the Contract on the job site. Copies of the current MUTCD may be obtained from the FHWA web page at [http://mutcd.fhwa.dot.gov](http://mutcd.fhwa.dot.gov).

**A. WORKER SAFETY APPAREL**

All workers exposed to the risks of moving roadway traffic or construction equipment shall wear high-visibility safety apparel meeting the requirements of International Safety
Equipment Association (ISEA) American National Standard for High-Visibility Safety Apparel, or equivalent revisions, and labeled as ANSI-2004 Class 2 or 3 risk exposure.

B. WORKSITE TRAFFIC CONTROL SUPERVISOR

ALL HIGHWAYS (ADDITIONAL REQUIREMENTS BELOW FOR INTERSTATES): The Contractor shall designate a qualified individual as the Worksite Traffic Control Supervisor (WTCS) who shall be responsible for selecting, installing and maintaining all traffic control devices in accordance with the Plans, Specifications, Special Provisions and the MUTCD. A written resume documenting the experience and credentials of the WTCS shall be submitted and accepted by the Engineer prior to beginning any work that involves traffic control. The WTCS shall be available on a twenty-four (24) hour basis to perform his duties. If the work requires traffic control activities to be performed during the daylight and nighttime hours it may be necessary for the Contractor to designate an alternate WTCS. An alternate WTCS must meet the same requirements and qualifications as the primary WTCS and be accepted by the Engineer prior to beginning any traffic control duties. The Worksite Traffic Control Supervisor’s traffic control responsibilities shall have priority over all other assigned duties.

As the representative of the Contractor, the WTCS shall have full authority to act on behalf of the Contractor in administering the TTC Plan. The WTCS shall have appropriate training in safe traffic control practices in accordance with Part VI of the MUTCD. In addition to the WTCS all other individuals making decisions regarding traffic control shall meet the training requirements of the Part VI of the MUTCD.

The WTCS shall supervise the initial installation of traffic control devices. The Engineer prior to the beginning of construction will review the initial installation. Modifications to traffic control devices as required by sequence of operations or staged construction shall be reviewed by the WTCS.

The WTCS shall be available on a full-time basis to maintain traffic control devices with access to all personnel, materials, and equipment necessary to respond effectively to an emergency situation within forty-five (45) minutes of notification of the emergency.

The WTCS shall regularly perform inspections to ensure that traffic control is maintained. Unless modified by the special conditions or by the Engineer, routine deficiencies shall be corrected within a twenty-four (24) hour period. Failure to comply with these provisions shall be grounds for dismissal from the duties of WTCS and/or removal of the WTCS from the project. Failure of the WTCS to execute his duties shall be considered as non-performance under Subsection 150.08.

The Engineer will periodically review the work for compliance with the requirements of the TTC plan.

On projects where traffic control duties will not require full time supervision, the Engineer may allow the Contractor’s Project Superintendent to serve as the WTCS as long as satisfactory results are obtained.

CERTIFIED WORKSITE TRAFFIC CONTROL SUPERVISOR
ADDITIONAL REQUIREMENTS FOR INTERSTATE AND LIMITED ACCESS HIGHWAYS: In addition to the requirements above, the WTCS shall have a minimum of one year’s
experience directly related to work site traffic control in a supervisory or responsible
capacity. The WTCS shall be currently certified by the American Traffic Safety Services
Association (ATSSA) Work Site Traffic Supervisor Certification program or the National
Safety Council Certification program.

Any work performed on the interstate or limited access highway right-of-way that requires
traffic control shall be supervised by the Certified Worksite Traffic Control Supervisor. No
work requiring traffic control shall be performed unless the certified WTCS is on the
worksite. Failure to maintain a Certified Worksite Traffic Control Supervisor on the work
will be considered as non-performance under Subsection 150.08.

The WTCS shall perform, as a minimum, weekly traffic control inspections on all interstate
and limited access highways. The inspection shall be reported to the Engineer on a TC-1
report. The Engineer will furnish a blank copy of the TC-1 report to the Contractor prior to
the beginning of any work on the interstate or limited access right-of-way.

C. TRAFFIC CONTROL DEVICES

All traffic control devices used during the construction of a project shall meet the Standards
utilized in the MUTCD, and shall comply with the requirements of these Specifications,
Project Plans, and Special Provisions. All devices shall be tested at NCHRP Test Level III.
Reference is made to Subsections 104.05, 107.07, and 107.09.

D. REFLECTORIZATION REQUIREMENTS

All rigid fluorescent orange construction warning signs (black on fluorescent orange) shall
meet the reflectorization and color requirements of ASTM Type VII, VIII, IX or X regardless
of the mounting height.

Portable signs which have flexible sign blanks shall meet the reflectorization and color
requirements of ASTM Type VI.

Warning signs (W3-1a) for stop conditions that have rumble strips located in the travelway
shall be reflectorized with ASTM Type IX fluorescent yellow sheeting.

All other signs shall meet the requirements of ASTM Type III or IV except for “Pass With
Care“ and “Do Not Pass” signs which may be ASTM Type I unless otherwise specified.

CHANNELIZATION DEVICES: Channelization devices shall meet the requirements of ASTM
Type III or IV high intensity sheeting.

E. IMPLEMENTATION REQUIREMENTS

No work shall be started on any project phase until the appropriate traffic control devices
have been placed in accordance with the Project requirements. Changes to traffic flow
shall not commence unless all labor, materials, and equipment necessary to make the
changes are available on the Project.

When any shift or change is made to the location of traffic or to the flow patterns of traffic,
including pedestrian traffic, the permanent safety features shall be installed and fully
operational before making the change. If staging or site conditions prevent the installation
of permanent features then the equivalent interim devices shall be utilized. This work shall also include any necessary removal and reinstallation of guardrail panels to achieve the required panel lap to accommodate the appropriate shift and traffic flow including the final traffic flow configuration (The cost of performing this work shall be included in Traffic Control-Lump Sum).

Any section of the work that is on new location shall have all permanent safety features installed and fully operational before the work is opened to traffic. Safety features shall include but are not limited to the following items:

1. Guardrail including anchors and delineation with properly lapped panels
2. Impact attenuators
3. Traffic signals
4. Warning devices
5. Pavement markings including words, symbols, stop bars, and crosswalks
6. Roadway signs including regulatory, warning, and guide

Outdoor lighting shall be considered as a safety feature for welcome centers, rest areas, and weigh station projects. For typical roadway type projects new street lighting is not considered a safety feature unless specifically noted in the plans or in the special conditions.

F. MAINTENANCE OF TRAFFIC CONTROL DEVICES

Traffic control devices shall be in acceptable condition when first erected on the project and shall be maintained in accordance with Subsection 104.05 throughout the construction period. All unacceptable traffic control devices shall be replaced within 24 hours. When not in use, all traffic control devices shall be removed, placed or covered so as not to be visible to traffic. All construction warning signs shall be removed within seven calendar days after time charges are stopped or pay items are complete. If traffic control devices are left in place for more than ten days after completion of the Work, the Department shall have the right to remove such devices, claim possession thereof, and deduct the cost of such removal from any monies due, or which may become due, the Contractor.

G. TRAFFIC INTERRUPTION RESTRICTIONS

The Department reserves the right to restrict construction operations when, in the opinion of the Engineer, the continuance of the Work would seriously hinder traffic flow, be needlessly disruptive or unnecessarily inconvenience the traveling public. The Contractor shall suspend and/or reschedule any work when the Engineer deems that conditions are unfavorable for continuing the Work.

Advanced notification requirements to the Contractor to suspend work will be according to the events and the time restrictions outlined below:

- Incident management: No advanced notice required
- Threatening/Inclement weather: 24 hours
- Holidays, sporting events, unfavorable conditions: Three (3) calendar days
If the work is suspended, the Contractor may submit a request for additional contract time as allowed under Section 108. The Department will review the request and may grant additional contract time as justified by the impact to the Contractor’s schedule. Compensation for loss of productivity, rescheduling of crews, rental of equipment or delays to the Contractor’s schedule will not be considered for payment. Additional contract time will be the only consideration granted to the Contractor.

H. SEQUENCE OF OPERATIONS

Any Sequence of Operations provided in this Contract in conjunction with any staging details which may be shown in the plans, is a suggested sequence for performing the Work. It is intended as a general staging plan for the orderly execution of the work while minimizing the impact on pedestrian facilities, mainline, cross-streets and side streets. The Contractor shall develop detailed staging and temporary traffic control plans for performing specific areas of the Work including but not limited to all traffic shifts, detours, bridge widenings, paces, or other activities that disrupt traffic or pedestrian flow. The Engineer may require detailed staging and TTC plans for lane closures or disruption to pedestrian facilities. These plans shall be submitted for approval at least two weeks prior to the scheduled date of the activity. Activities that have not been approved at least seven (7) days prior to the scheduled date shall be rescheduled.

Where traffic is permitted through the work area under stage construction, the Contractor may choose to construct, at no additional expense to the Department, temporary on-site bypasses or detours in order to expedite the work. Plans for such temporary bypasses or detours shall be submitted to the Engineer for review and approval 30 calendar days prior to the proposed construction. Such bypasses or detours shall be removed promptly when in the opinion of the Engineer; they are not longer necessary for the satisfactory progress of the Work. Bypasses and detours shall meet the minimum requirements of Section 150.02.B.4.

As an option to the Sequence of Operations in the Contract, the Contractor may submit an alternative Sequence of Operations for review and approval. Alternate Sequence of Operations for pedestrian facilities shall be in compliance with the MUTCD and ADA. Pedestrian needs identified in the preconstruction phase shall be included in the proposed alternate plan. The Department may consider the Contractor’s alternate Sequence of Operations as a Value Engineering Proposal as defined by Section 104.08. A twenty calendar days lead time for the Department’s review shall be given to this submission so that a decision on its acceptability can be made and presented at the Preconstruction Conference. Insufficient lead time or no submission by the Contractor shall be construed as acceptance of the Sequence of Operations outlined in the Contract and the willingness of the Contractor to execute this as-bid plan.

The Department will not pay, or in any way reimburse the Contractor for claims arising from the Contractor’s inability to perform the Work in accordance with the Sequence of Operations provided in the Contract or from an approved Contractor alternate.

The Contractor shall secure the Engineer’s approval of the Contractor’s proposed plan of operation, sequence of work and methods of providing for the safe passage of vehicular
and pedestrian traffic before it is placed in operation. The proposed plan of operation shall supplement the approved traffic control plan. Any major changes to the approved TTC plan, proposed by the Contractor, shall be submitted to the Department for approval.

Some additional traffic control details will be required prior to any major shifts or changes in traffic. The traffic control details shall include, but not be limited to, the following:

1. A detailed drawing showing traffic locations and laneage for each step of the change.
2. The location, size, and message of all signs required by the MUTCD, Plan, Special Provisions, and other signs as required to fit conditions. Any portable changeable message signs used shall be included in the details.
3. The method to be used in, and the limits of, the obliteration of conflicting lines and markings.
4. Type, location, and extent of new lines and markings.
5. Horizontal and vertical alignment and superelevation rates for detours, including cross-section and profile grades along each edge of existing pavement.
6. Drainage details for temporary and permanent alignments.
7. Location, length, and/or spacing of channelization and protective devices (temporary barrier, guardrail, barricades, etc.)
8. Starting time, duration and date of planned change.
9. For each traffic shift, a paving plan, erection plan, or work site plan, as appropriate, detailing workforce, materials, and equipment necessary to accomplish the proposed work. This will be the minimum resource allocation required in order to start the work.

A minimum of three copies of the above details shall be submitted to the Engineer for approval at least 14 days prior to the anticipated traffic shift. The Contractor shall have traffic control details for a traffic shift which has been approved by the Engineer prior to commencement of the physical shift. All preparatory work relative to the traffic shift, which does not interfere with traffic, shall be accomplished prior to the designated starting time. The Engineer and the Contractor’s representative will verify that all conditions have been met prior to the Contractor obtaining materials for the actual traffic shift.

150.02 TEMPORARY TRAFFIC CONTROL (TTC) ZONES:

A. DEVICES AND MATERIALS:

In addition to the other provisions contained herein, work zone traffic control shall be accomplished using the following means and materials:
1. **Portable Advance Warning Signs**

   Portable advance warning signs shall be utilized as per the requirements of the temporary traffic control plans. All signs shall meet the requirements of the MUTCD and shall be NCHRP 350 crashworthy compliant.

2. **Arrow Panels**

   Portable sequential or flashing arrow panels as shown in the Plans or Specifications for use on Interstate or multi-lane highway lane closure only, shall be a minimum size of 48” high by 96” wide with not less than 15 lamps used for the arrow. The arrow shall occupy virtually the entire size of the arrow panel and shall have a minimum legibility distance of one mile. The minimum legibility distance is that distance at which the arrow panel can be comprehended by an observer on a sunny day, or clear night. Arrow panels shall be equipped with automatic dimming features for use during hours of darkness. The arrow panels shall also meet the requirements for a Type C panel as shown in the MUTCD. The sequential or flashing arrow panels shall not be used for lane closure on two-lane, two-way highways when traffic is restricted to one-lane operations in which case, appropriate signing, flaggers and when required, pilot vehicles will be deemed sufficient.

   The sequential or flashing arrow panels shall be placed on the shoulder at or near the point where the lane closing transition begins. The panels shall be mounted on a vehicle, trailer, or other suitable support. Vehicle mounted panels shall be provided with remote controls. Minimum mounting height shall be seven feet above the roadway to the bottom of the panel, except on vehicle mounted panels which should be as high as practical.

   For emergency situations, arrow display panels that meet the MUTCD requirements for Type A or Type B panels may be used until Type C panels can be located and placed at the site. The use of Type A and Type B panels shall be held to the minimum length of time possible before having the Type C panel(s) in operation. The Engineer shall determine when conditions and circumstances are considered to be emergencies. The Contractor shall notify the Engineer, in writing, when any non-specification arrow display panel(s) is being used in the work.

3. **Portable Changeable Message Signs**

   Portable changeable message signs meeting the requirements of Section 632 and the MUTCD. Any PCMS in use that is not protected by positive barrier protection shall be delineated by a minimum of three drums that meet the requirement of Section 150.05.A.1. The drum spacing shall not exceed a maximum of ten (10’) feet as shown in Detail 150-PCMS. When the PCMS is within twenty (20’) feet of the opposing traffic flow, the trailing end of the PCMS shall be delineated with a minimum of three drums spaced in the same manner as the approach side of the PCMS.
When not in use the PCMS shall be removed from the roadway unless protected by positive barrier protection. If the PCMS is protected by positive barrier protection the sign panel shall be turned away from traffic when not in use.

4. **Channelization Devices**
   Channelization devices shall meet the standards of the MUTCD and Subsection 150.05.

5. **Temporary Barrier**
   Temporary barrier shall meet the requirements of Sections 620.

6. **Temporary Traffic Signals**
   Temporary traffic signals shall meet the requirements of Section 647 and the MUTCD.

7. **Pavement Marking**
   Pavement marking incorporated into the work shall comply with Subsections 150.04.A and 150.04.B.

8. **Portable Temporary Traffic Control Signals**
   The use of Portable Temporary Traffic Control Signals shall meet the following minimum requirements:

   Only two-lane two-way roadways will be allowed to utilize Portable Temporary Traffic Control Signals.

   All portable traffic control signals shall meet the physical display and operational requirements of conventional traffic signals described in the MUTCD.

   Each signal face shall have at least three lenses. The lenses shall be red, yellow, or green in color and shall give a circular type of indication. All lenses shall be twelve (12") inches nominal in diameter.
A minimum of two signal faces shall face each direction of traffic. A minimum of one signal head shall be suspended over the roadway travel lane in a manner that will allow the bottom of the signal head housing to be not less than seventeen (17’) feet above and not more than nineteen (19’) feet above the pavement grade at the center of the travel lane. The second signal head may be located over the travel lane with the same height requirements or the second signal head may be located on the shoulder. When the signal head is located on the shoulder the bottom of the signal head housing shall be at least eight (8’) feet but not more than (15’) feet above the pavement grade at the center of highway.

Advance warning signage and appropriate pavement markings shall be installed as part of the temporary signal operation.

The signals shall be operated in a manner consistent with traffic requirements. The signals may be operated in timed-mode or in a vehicle-actuated mode. The signals shall be interconnected in a manner to ensure that conflicting movements can not occur. To assure that the appropriate operating pattern including timing is displayed to the traveling public, regular inspections including the use of accurate timing devices shall be made by the Worksite Traffic Control Supervisor. If at any time any part of the system fails to operate within these requirements then the use of the signal shall be suspended and the appropriate flagging operation shall begin immediately.

The Worksite Traffic Control Supervisor (WTCS) shall continuously monitor the portable traffic control signal to insure compliance with the requirements for maintenance under the MUTCD. The signal shall be maintained in a manner consistent with the intention of the MUTCD, with emphasis on cleaning of the optical system. Timing changes shall be made only by the WTCS. The WTCS shall keep a written record of all timing changes.

The portable temporary signal shall have two power sources and shall be capable of running for seven calendar days continuously.

The Contractor shall have an alternate temporary traffic control plan in the event of failure of the signal.

9. RUMBLE STRIPS
Rumble strips incorporated into the work shall meet the requirements of Section 429 and the MUTCD. Existing rumble strips that are positioned in the traveled way to warn traffic of a stop condition shall be reinstalled based on the following requirements:

INTERMEDIATE SURFACES: Intermediate surfaces that will be in use for more than forty-five (45) calendar days shall have rumble strips reinstalled on the traveled way in the area of a stop condition. Non-refundable deductions in accordance with 150.08 will be assessed for any intermediate surface in place for greater than 45 days without rumble strips.

FINAL SURFACES: Rumble strips shall be installed on the final surface within fourteen (14) calendar days of the placement of the final surface in the area of the stop condition. Failure to install within fourteen (14) calendar days will result in assessment of non-refundable deductions in accordance with 150.08.
Prior to the removal of any rumble strips located in the travelway, stop ahead (W3-1a) warning signs shall be double indicated ahead of the stop condition. These warning signs shall be a minimum of 48 inches by 48 inches. The reflectorization of the warning signs shall be as required by Subsection 150.01.C. These warning signs shall remain in place until the rumble strips have been reinstalled on the traveled way. Any existing warning signs for the stop ahead condition shall be removed or covered while the 48” X 48” (W3-1a) signs are in place. When the rumble strips have been reinstalled these warning signs should be promptly removed and any existing signage placed back in service.

10. GUARDRAIL: When the removal and installation of guardrail is required as a part of the work the following time restrictions shall apply unless modified by the special conditions:

MULTI-LANE HIGHWAYS: From the time that the existing guardrail or temporary positive barrier protection is removed the Contractor has fourteen (14) calendar days to install the new guardrail and anchors. During the interim, the location without guardrail shall be protected with drums spaced at a maximum spacing of twenty (20') feet. The maximum length of rail that can be removed at any time without being replaced with positive barrier protection is a total of 2000 linear feet of existing rail or the total length of one run of existing rail, whichever is less.

ALL OTHER HIGHWAYS: From the time that the existing guardrail is removed or from the time that temporary positive barrier protection is removed the Contractor has thirty (30) calendar days to install the new guardrail and anchors. During the interim, the location without guardrail shall be protected with drums spaced at a maximum spacing of twenty (20') feet. The maximum length of rail that can be removed at any time without being replaced with positive barrier protection is a total of 1000 linear feet of existing rail or the total length of one run of existing rail, whichever is less.

Based on existing field conditions, the Engineer may review the work and require that the guardrail be installed earlier than the maximum time allowed above by giving written notification to the Contractor via the TC-1 traffic control report.

ALL HIGHWAYS: The contractor shall install new guardrail such that traffic exposure to fixed objects is minimized. Within the same work day, temporary attenuators, as defined in Subsection 150.05.B, should be installed on the approach to fixed objects that can’t be protected with guardrail. Truck mounted attenuators may be used to shield exposed fixed objects for periods not to exceed forty-eight (48) hours. No separate payment will be made for truck mounted attenuators used in lieu of temporary attenuators.

When the roadway is open to traffic, guardrail panels shall be lapped to comply with the directional flow of traffic. Should the staging of the work require that the lap of the guardrail be changed, this work shall be completed before the roadway is opened to traffic. The work to change the lap of any guardrail shall be included in Traffic Control-Lump Sum.

Failure to comply with the above time and quantity restrictions shall be considered as non-compliance under Section 150.08.
11. STOP SIGN REGULATED INTERSECTIONS: For intersections that utilize stop sign(s) to control the flow of traffic and to restrict the movement of vehicles, the stop sign(s) shall be maintained for the duration of the work or until such time that the stop condition is eliminated or until an interim or permanent traffic signal can be installed to provide proper traffic control. The traffic signal shall be installed and properly functioning before the removal of the existing stop sign(s) is permitted. If the existing intersection is enhanced traffic control features such as stop bars, double indicated stop signs, oversized signs, advanced warning stop ahead signs, rumble strips on the approaches or flashing beacons located overhead or on the shoulders then these features shall be maintained for the duration of the project or until the permanent traffic control plan has been implemented.

Whenever the staging of the work requires that the traveled-way be relocated or realigned the Contractor shall reinstall all enhanced traffic control features noted above on the newly constructed sections of the work. The cost of relocating the stop bars, stop signs, advanced warning signs, the rumble strips and the flashing beacons shall be included in the price bid for Lump-Sum-Traffic Control unless individual pay items are included in the contract for rumble strips and/or flashing beacons. When pay items are included in the contract for rumble strips or flashing beacons then these items will be paid per each.

When staging requires the relocation or realignment of an existing stop condition it may be necessary to consider the addition of enhanced traffic control features even though none existed at the original location. As a guide for enhanced traffic control features that may be considered, the Engineer or the WTCS may refer to the Department's guidelines for “Opening of New Roadways to Traffic” (Document #6635-2). Horizontal and vertical alignment changes at a new location may have decreased or restricted sight distance or the stop condition may occur sooner than in the previous alignment. If these conditions occur then the Engineer and/or the WTCS should consider additional measures to enhance the motorist's awareness of the changes even though the staging plans may not address enhanced features. Stop signs should be a minimum of 36 inches for interim situations. The use of 48 inch stop signs may be warranted under project specific conditions. Flags may be used on interim/permanent stop signs that are mounted at seven (7') feet in height for a short duration in order to direct additional attention to a new or relocated stop sign(s). Flags should not be used for durations exceeding two weeks unless unusual or site specify conditions warrant a longer period of time. The use of Type “A” flashing red light(s) attached to the stop sign(s) may be appropriate during the same period that the flags are in use to increase attention.

The use of rumble strips and/or portable changeable message signs may be considered. The use of new rumble strips, where none previously existed, shall have the prior approval of District Traffic Operations before being included as part of the temporary traffic control plan. The message(s) displayed on any PCMS shall have the prior approval of the Engineer and the message(s) shall be included as part of the TTC plan for the interim staging.

The placement of any additional interim ground-mounted signs and posts or stop bars shall be considered as incidental to the price bid for Lump Sum-Traffic Control. The installation of rumble strips, flashing beacons or the use of Portable Changeable Message Signs (PCMS) shall be considered as Extra Work unless pay items are included in the contract.
B. WORK ZONE RESTRICTIONS:

1. Interstate

The Contractor shall not simultaneously perform work on both the inside shoulder and outside shoulder on either direction of traffic flow when the Work is within 12 feet of the travel-way, unless such areas are separated by at least one-half mile of distance.

2. Non-Interstate Divided Highways

The Contractor shall not simultaneously perform work on both the inside shoulder and outside shoulder on either direction of traffic flow when the Work is within 12 feet of the travel-way, unless such areas are separated by at least one-half mile distance in rural areas or at least 500 feet of distance in urban areas.

3. Non-Divided Highways

a. The Contractor shall not simultaneously perform work on opposite sides of the roadway when the work is within 12 feet of the travel-way, unless such areas are separated by at least one-half mile of distance in rural areas or at least 500 feet of distance in urban areas.

b. On two-lane projects where full width sections of the existing subgrade, base or surfacing are to be removed, and new base, subgrade, or surfacing are to be constructed, the Contractor shall maintain one-lane traffic through the construction area by removing and replacing the undesirable material for half the width of the existing roadway at a time. Replacement shall be made such that paving is completed to the level of the existing pavement in the adjacent lane by the end of the workday or before opening all the roadway to traffic.

4. All Highways:

a. There shall be no reduction in the total number of available traffic lanes that existed prior to construction except as specifically allowed by the Contract and as approved by the Engineer.

b. Travelway Clearances: All portions of the work shall maintain the following minimum requirements:

   Horizontal: The combined dimensions of the paved shoulder and the roadway surface remaining outside the Work Zone shall be no less than sixteen (16) feet in width at any location.

   Vertical: The overhead clearance shall not be reduced to less than fifteen (15) feet at any location.

The restrictions above apply to all shifts, lane closures, on-site detours and off site detours whether shown in the contract or proposed by the Contractor. It shall be
the responsibility of the Contractor to verify that these minimum requirements have been met before proceeding with any phase of the Work.

Two-lane two-way roadways may have temporary horizontal restrictions of less than sixteen (16) feet provided a flagger operation for one-way traffic is utilized to restrict access to the work area by over-width loads. The minimum horizontal clearance shall be restored before the flagging operation is removed.

c. Highway Work Zone: All sections or segments of the roadway under construction or reconstruction shall be signed as a Highway Work Zone except non-state highway two-lane two-way resurfacing projects. Two conditions can be applied to a Highway Work Zone. Condition 1 is when no reduction in the existing speed limit is required. Condition 2 is when worksite conditions require a reduction of the speed limit through the designated Work Zone. Properly marking a Highway Work Zone shall include the following minimum requirements:

1. NO REDUCTION IN THE EXISTING POSTED SPEED LIMIT IN HIGHWAY WORK ZONE:

   a) Signage (Detail 150-HWZ-2) shall be posted at the beginning point of the Highway Work Zone warning the traveling public that increased penalties for speeding violations are in effect. The HWZ-2 sign shall be placed a minimum of six hundred (600') feet in advance of the Highway Work Zone and shall not be placed more than one thousand (1000') feet in advance of the Work Zone. If no speed reduction is required it is recommended that the HWZ-2 be placed at 750 feet from the work area between the ROAD WORK 500 FT. and the ROAD WORK 1000 FT. signs. HWZ-2 signs shall be placed at intervals not to exceed one mile for the length of the project. HWZ-2 signs should be placed on the mainline after all major intersections except State Routes. State Routes shall be signed as per the requirements for intersecting roadways below.

   b) The existing speed limit shall be posted at the beginning of the Work Zone. Existing Speed Limit signs (R2-1) shall be maintained.

   c) INTERSECTING ROADWAYS: Intersecting state routes shall be signed in advance of each intersection with the Work Zone with a HWZ-2 sign to warn motorists that increased fines are in effect. All other intersecting roadways that enter into a designated Highway Work Zone may be signed in advance of each intersection with the Work Zone. When construction equipment and personnel are present in the intersection on the mainline of a multi-lane roadway, the intersecting side roads shall be signed in advance with HWZ-2 signs. As soon as the work operation clears the intersection the signage may be removed.

   d) Signage (Detail 150-HWZ-3) shall be posted at the end of the Highway Work Zone indicating the end of the zone and indicating that increased penalties for speeding violations are no longer in effect.
e) When a designated Highway Work Zone is no longer necessary all signs shall be removed immediately.

2. REDUCING THE SPEED LIMIT IN A HIGHWAY WORK ZONE:

Highway Work Zone signs shall be posted as required in Condition 1 above.

For limited access (interstate) highways and controlled access multi-lane divided highways the posted speed limit shall be reduced as required below.

Speed Limit signage (R2-1) for the reduced speed limit shall be erected at the beginning of the work zone. Additional signs shall be placed to ensure that the maximum spacing of the reduced speed limit signs shall be no greater than one (1) mile apart. Existing speed limit signs shall be covered or removed. On multi-lane divided highways the speed limit signs shall be double indicated when the reduced speed is in use.

When any one or more of the following conditions exist and the existing speed limit is 65 mph or 70 mph, the speed limit shall be reduced by 10 mph. If the existing speed limit is 60 mph, the speed limit should be reduced by 5 mph. If the existing speed limit is 55 mph or less, the Contractor can only reduce the speed limit with the prior approval of the Engineer. The reduction in the speed limit shall be no greater than 10 mph:

a) Lane closure(s) of any type and any duration.
b) The difference in elevation exceeds two inches adjacent to a travel lane as shown in Subsection 150.06, Detail 150-B, 150-C.
c) Any areas where equipment or workers are within ten feet of a travel lane.
d) Temporary portable concrete barriers located less than two (2') feet from the traveled way.
e) As directed by the Engineer for conditions distinctive to this project.

When the above conditions are not present the speed limit shall be immediately returned to the existing posted speed limit. A speed reduction shall not be put in place for the entire length of the project unless conditions warranting the speed reduction are present for the entire project length. All existing speed limit signs within the temporary speed reduction zone shall be covered or removed while the temporary reduction in the speed limit is in effect. All signs shall be erected to comply with the minimum requirements of the MUTCD.

As a minimum the following records shall be kept by the WTCS:

a) Identify the need for the reduction.
b) Record the time of the installation and removal of the temporary reduction.
c) Fully describe the location and limits of the reduced speed zone.
d) Document any accident that occurs during the time of the reduction.

A copy of the weekly records for reduced speed zones shall be submitted to the Engineer.
Reduced speed zones shall, as a minimum, be signed as per Detail 150-HWZ-1. Interim signs shall meet the requirements of 150.03 D. Additional signs may be necessary to adjust for actual field conditions.

When a pilot vehicle is used on a two-lane two-way roadway the speed limit should not be reduced. For special conditions specific to the work, on two-lane two-way roadways or multi-lane highways, the contractor may reduce the posted speed limit with the prior approval of the Engineer.

5. MILLED SURFACE RESTRICTIONS:
Unless modified by the special conditions, a milled surface on any asphaltic concrete surface shall not be allowed to remain open to traffic for a period of time that exceeds thirty (30) calendar days.

6. INSTALLATION/REMOVAL OF WORK AREA SIGNAGE:
No payment will be made for Traffic Control-Lump Sum until the Work has actually started on the project. The installation of traffic control signage does not qualify as the start of work. Advanced warning signs shall not be installed until the actual beginning of work activities. Any permanent mount height signs installed as the work is preparing to start shall be covered until all signs are installed unless all signs are installed within seven (7) calendar days after beginning installation.

All temporary traffic control devices shall be removed as soon as practical when these devices are no longer needed. When work is suspended for short periods of time, temporary traffic control devices that are no longer appropriate shall be removed or covered.

All construction warning signs shall be removed within seven (7) calendar days after time charges are stopped or pay items are complete. If traffic control devices are left in place for more than ten (10) calendar days after completion of the Work, the Department shall have the right to remove such devices, claim possession thereof, and deduct the cost of such removal from any monies due, or which may become due, the Contractor.

PUNCHLIST WORK: Portable signs shall be utilized to accomplish the completion of all punchlist items. The portable signs shall be removed daily. All permanent mount height signs shall be removed prior to the beginning of the punchlist work except “Low/Soft Shoulder” signs and any signs that have the prior written approval of the Engineer to remain in place while the punchlist work is in progress.

Failure to promptly remove the construction warning signs within the seven (7) calendar days after the completion of the Work or failure to remove or cover signs when work is suspended for short periods of time shall be considered as non-performance under Section 150.08.
SPEED LIMIT REDUCTION FOR HIGHWAY WORK ZONE
INTERSTATE AND MULTI-LANE DIVIDED HIGHWAY SIGNING SHALL BE DOUBLE INDICATED (RIGHT SHOULDER AND MEDIAN SHOULDER)

600'  600'  600'  600'  600'  500' MAX.

WORK ZONE

HWZ-2 SIGNS

REDUCED SPEED AHEAD
R2-5a
48" X 60"

THIS SIGN SHALL BE INSTALLED ONLY WHEN THE SPEED REDUCTION IS GREATER THAN 10 M.P.H. FROM THE EXISTING POSTED SPEED LIMIT.

BEGIN SPEED ZONE

SPEED LIMIT
R2-1
48" X 60"

SPEED LIMIT
R2-1
48" X 60"

REDUCED SPEED LIMIT SHALL HAVE THE PRIOR APPROVAL OF THE ENGINEER.

R2-1
48" X 60"

POST EXISTING SPEED LIMIT PRIOR TO CONSTRUCTION SPEED ZONE REDUCTION

DOUBLED INDICATOR NOT REQUIRED FOR THIS SIGN

R2-1
48" X 60"

REDUCED CONSTRUCTION SPEED LIMIT SHALL BE SPACED A MAXIMUM OF ONE MILE APART.

HWZ-3 SIGNS

SPEED LIMIT
R2-1
48" X 60"

SPEED LIMIT
R2-1
48" X 60"

SIGN SIZE SHOWN ARE FOR INTERSTATE AND MULTI-LANE DIVIDED HIGHWAY.
FOR OTHER HIGHWAYS USE STANDARD SIZE SIGNS AS PER THE M.U.T.C.D. EXCEPT HWZ-2 AND HWZ-3 SIGNS.

DETAIL 150-HWZ-1

ALL INTERSECTING ROADWAYS SHALL BE SIGNED WITH A HWZ-2 SIGN TO WARN MOTORIST ENTERING THE HIGHWAY WORK ZONE.

INTERSTATE AND MULTI-LANE HIGHWAY SIGNING SHALL BE DOUBLE INDICATED (RIGHT SHOULDER AND MEDIAN SHOULDER).
COLORS

TOP PANEL
LEGEND & BORDER - BLACK (NON-REFL)
BACKGROUND - FLUORESCENT ORANGE
   (ASTM TYPE VII, VIII, IX or X)

MIDDLE & BOTTOM PANELS
LEGEND & BORDER - BLACK (NON-REFL)
BACKGROUND - WHITE (ASTM TYPE III OR IV REFLECTIVE SHEETING)

NOTES:
1. ALL HWZ-2 SIGN PANELS SHALL BE RIGID.
2. THE SIZE OF THE HWZ-2 SIGN SHALL NOT BE REDUCED FOR USE ON TWO-LANE ROADWAYS.
WORK ZONE
END
INCREASED
SPEEDING
FINES

COLORS
TOP PANEL
LEGEND & BORDER - BLACK (NON-REFL)
BACKGROUND - FLUORESCENT ORANGE
(ASTM TYPE VII, VIII, IX or X)

BOTTOM PANEL
LEGEND & BORDER - BLACK (NON-REFL)
BACKGROUND - WHITE (ASTM TYPE III OR IV REFL SHEETING)

NOTES:
1. ALL HWZ-3 SIGN PANELS SHALL BE RIGID.
2. THE SIZE OF THE HWZ-3 SIGN SHALL NOT BE REDUCED FOR USE ON TWO-LANE ROADWAYS.
C. LANE CLOSURES:

1. Approval/Restrictions
   All lane closures of any type or duration shall have the prior approval of the Engineer.
   a. The length of a lane closure shall not exceed two (2) miles in length excluding the length of the tapers unless the prior approval of the Engineer has been obtained. The Engineer may extend the length of a lane closure based upon field conditions however the length of a workzone should be held to the minimum length required to accomplish the Work. Lane closures shall not be spaced closer than one mile. The advanced warning signs for the project should not overlap with the advanced warning signs for lane shifts, lane closures, etc.
   b. Lane closures that require same direction traffic to be split around the Work Area will not be approved for roadways with posted speeds of 35 mph or greater, excluding turn lanes.
   c. For Interstate, Limited Access and Multi-lane Divided Highways, a Portable Changeable Message Sign (PCMS) shall be placed one (1) mile in advance of a lane closure with a message denoting the appropriate lane closure one mile ahead. The Portable Changeable Message Sign (PCMS) shall be placed on the outside shoulder in accordance with Detail 150-PCMS. This is in addition to the other traffic control devices required by Standard 9106.

2. Removal Of Lane Closures
   To provide the greatest possible convenience to the public in accordance with Sub-Subsection 107.07, the Contractor shall remove all signs, lane closure markings, and devices immediately when lane closure work is completed or temporarily suspended for any length of time or as directed by the Engineer. All portable signs and portable sign mounting devices shall be removed from the roadway to an area which will not allow the sign to be visible and will not allow the sign or sign mounting device to be impacted by traffic.

3. Exit And Entrance Ramps
   On multilane highways where traffic has been shifted to the inside lanes, the exit and entrance ramps shall have channelization devices placed on both sides of the ramp. This requirement will apply to any situation where traffic is shifted to contra flows or inside staging lanes to facilitate reconstruction work in the vicinity of exit and entrance ramps. The temporary ramp taper length shall be greater than, or equal to, the existing taper length. Interim EXIT gore signs shall be placed at the ramp divergence. The “EXIT OPEN” sign shown in Figure TA-42 of the MUTCD shall be utilized. For exit ramps, channelization device spacing shall be decreased to 10 feet for 200 feet in advance of the temporary gore, and be decreased to 10 feet for the first 100 feet of the temporary gore.

4. Lane Drop/Lane Closure
   The first seven (7) calendar days of any lane closure shall be signed and marked as per Standard 9106 or 9107. However, lane closures that exist for a duration longer than seven (7) calendar days may be signed and marked as per the details in Standard 9121, provided
the prior approval of the Engineer is obtained. The approved lane drop shall utilize only the signs and markings shown for the termination end of the lane drop in Standard 9121. All warning signs in the lane drop sequence shall be used. Drums may be substituted for the Type I Crystal Delineators at the same spacing.

5. Termination Area
The transition to normal or full width highway at the end of a lane closure shall be a maximum of 150 feet.

D. TRAFFIC PACING METHOD:

1. Pacing Of Traffic
With prior approval from the Engineer, traffic may be paced allowing the Contractor up to ten (10) minutes maximum to work in or above all lanes of traffic for the following purposes:

a. Placing bridge members or other bridge work.

b. Placing overhead sign structures.

c. Other work items requiring interruption of traffic.

The Contractor shall provide a uniformed police officer with patrol vehicle and blue flashing light for each direction of pacing. The police officer, Engineer, and flaggers at ramps shall be provided with a radio which will provide continuous contact with the Contractor.

When ready to start the work activity, the police vehicle will act as a pilot vehicle slowing the traffic thereby providing a gap in traffic allowing the Contractor to perform the Work. Any on-ramps between the pace and the work area shall be blocked during pacing of traffic, with a flagger properly dressed and equipped with a Stop/Slow paddle. Each ramp should be opened after the police vehicle has passed. Pilot vehicles shall travel at a safe pace speed, desirably not less than 20 mph interstate and 10 mph non-interstate. The Contractor shall provide a vehicle to proceed in front of the police vehicle and behind the other traffic in order to inform the Contractor’s work force when all vehicles have cleared the area.

Traffic will not be permitted to stop during pacing except in extreme cases as approved by the Engineer.
2. Methods Of Signing For Traffic Pacing

At a point not less than 1,000 feet in advance of the beginning point of the pace, the Contractor shall erect and cover a W-special sign (72 inch x 72 inch) with a Type “B” flashing light, with the legend “TRAFFIC SLOWED AHEAD SHORT DELAY” (See Detail 150-A). A portable changeable message sign may be used in lieu of the W-special sign. On divided highways this sign shall be double indicated. A worker with a two-way radio shall be posted at the sign, and upon notice that the traffic is to be paced shall turn on the flashing light and reveal the sign. When traffic is not being paced, the flashing light shall be turned off and the sign covered or removed. W-special signs are reflectorized black on orange, Series “C” letter and border of the size specified.
E. CONSTRUCTION VEHICLE TRAFFIC

The Contractor’s vehicles shall travel in the direction of normal roadway traffic and shall not reverse direction except at intersections, interchanges, or approved temporary crossings. The Contractor may submit a plan requesting that construction traffic be allowed to travel in the opposite direction of normal traffic when it would be desirable to modify traffic patterns to accommodate specific construction activities.

Prior approval of the Engineer shall be obtained before any construction traffic is allowed to travel in a reverse direction. If the Contractor’s submittal is approved the construction traffic shall be separated from normal traffic by appropriate traffic control devices.

F. ENVIRONMENTAL IMPACTS TO THE TEMPORARY TRAFFIC CONTROL (TTC) PLAN

The Contractor shall ensure that dust, mud, and other debris from construction activities do not interfere with normal traffic operations or adjacent properties. All outfall ditches, special ditches, critical storm drain structures, erosion control structures, retention basins, etc. shall be constructed, where possible, prior to the beginning of grading operations so that the best possible drainage and erosion control will be in effect during the grading operations, thereby keeping the roadway areas as dry as possible.

Areas within the limits of the project which are determined by the Engineer to be disturbed or damaged due either directly or indirectly from the progress or the lack of progress of the work shall be cleaned up, redressed, and regrassed. All surplus materials shall be removed and disposed of as required. Surplus materials shall be disposed of in accordance with Subsection 201.02.E.3 of the Specifications.

G. EXISTING STREET LIGHTS

Existing street lighting shall remain lighted as long as practical and until removal is approved by the Engineer.

H. NIGHTWORK

Adequate temporary lighting shall be provided at all nighttime work sites where workers will be immediately adjacent to traffic.

I. CONSTRUCTION VEHICLES IN THE WORKZONE

The parking of Contractor’s and/or workers personal vehicles within the work area or adjacent to traffic is prohibited. It shall be the responsibility of the Worksite Traffic Control Supervisor to ensure that any vehicle present at the worksite is necessary for the completion of the work.

J. ENCROACHMENTS ON THE TRAVELED-WAY

The Worksite Traffic Control Supervisor (WTCS) shall monitor the work to ensure that all the rocks, boulders, construction debris, stockpiled materials, equipment, tools and other potential hazards are kept clear of the travelway. These items shall be stored in a location, in so far as practical, where they will not be subject to a vehicle running off the road and striking them.
K. PEDESTRIAN CONSIDERATIONS

All existing pedestrian facilities, including access to transit stops, shall be maintained. Where pedestrian routes are closed, alternate routes shall be provided. Closures of existing, interim and final pedestrian facilities shall have the prior written approval of the Engineer. When existing pedestrian facilities are disrupted, closed or relocated in a TTC zone, the temporary facilities shall be detectable and shall include accessibility features consistent with the features present in the existing pedestrian facility. Pedestrian facilities are considered improvements and provisions made to accommodate or encourage walking. Whenever a sidewalk is to be closed, the Engineer shall notify the maintaining agency two (2) weeks prior to the closure. Prior to closure, detectable barriers (that are detectable by a person with a visual disability traveling with the aid of a long cane), as described by the MUTCD, shall be placed across the full width of the closed sidewalk. Barriers and channelizing devices used along a temporary pedestrian route shall be in compliance with the MUTCD.

Temporary Traffic Control devices used to delineate a Temporary Traffic Control zone pedestrian walkway shall be in compliance with Subsection 150.01.C. Temporary Traffic Control devices and construction material shall not intrude into the usable width of the pedestrian walkway. Signs and other devices shall be placed such that they do not narrow or restrict any pedestrian passage to less than 48 inches.

A pedestrian walkway shall not be severed or relocated for non-construction activities such as parking for construction vehicles and equipment. Movement by construction vehicles and equipment across designated pedestrian walkways should be minimized. When necessary, construction activities shall be controlled by flaggers. Pedestrian walkways shall be kept free of mud, loose gravel or other debris.

When temporary covered walkways are used, they shall be lighted during nighttime hours. When temporary traffic barrier is used to separate pedestrian and vehicular traffic, the temporary barrier shall meet NCHRP-350 Test Level Three. The barrier ends shall be protected in accordance with Georgia Standard 4960. Curbing shall not be used as a substitute for temporary traffic barriers when temporary traffic barriers are required. Tape, rope or plastic chain strung between temporary traffic control devices are not considered as detectable and shall not be used as a control for pedestrian movements.

The WTCS shall inspect the activity area daily to ensure that effective pedestrian TTC is being maintained. The inspection of TTC for pedestrian traffic shall be included as part of the TC-1 report.

1. Temporary Pedestrian Facilities

Temporary pedestrian facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. The geometry, alignment and construction of the facility should meet the applicable requirements of the “Americans with Disabilities Act Accessibility Guidelines for Buildings and Facilities (ADAAG)”.

a. Temporary Walkways with Detectable Edging

A smooth, continuous hard surface (firm, stable and slip resistant) shall be provided throughout the entire length of the temporary pedestrian facility.
Compacted soils, sand, crushed stone or asphaltic pavement millings shall not be used as a surface course for walkways.

Temporary walkways shall include detectable edging as defined in the MUTCD. When temporary traffic barrier is included as a pay item in the contract and where locations identified on the plans for positive protection will also allow them to serve as pedestrian detectable edging, payment will be made for the temporary traffic barrier in accordance with Section 620. No payment will be made for temporary walkways with Detectable Edging where existing pavements or existing edging (that meets the requirements of MUTCD) are utilized as temporary walkways. Payment for temporary detectable edging, including approved barriers and channelizing devices, installed on existing pavements shall be included in Traffic Control-Lump Sum.

Regardless of the materials used, temporary walkways shall be constructed of sufficient thickness and durability to withstand the intended use for the duration of the construction project. If concrete or asphalt is used as the surface course for the walkway, it shall be a minimum of one and one-half inches (1-1/2") thick. Temporary walkways constructed across unimproved streets and drives shall be a minimum thickness of four inches (4") for concrete and three inches (3") for asphalt. Joints formed in concrete sidewalks shall be in accordance with Section 441. Concrete surfaces shall have a broom finish.

If plywood is used as a walkway, it must be a minimum of three quarters of an inch (3/4") thick pressure treated and supported with pressure treated longitudinal joists spaced a maximum of sixteen inches (16") on center. The plywood shall be secured to the joist with galvanized nails or galvanized deck screws. Nails and screws shall be countersunk to prevent snagging or tripping the pedestrians. A slip resistant friction course shall be applied to any plywood surface that is used as a walkway. Any slip resistant material used shall have the prior written approval of the engineer.

The contractor may propose alternate types of Temporary Walkways provided the contractor can document that the proposed walkway meets the requirements of the “Americans with Disabilities Act Accessibility Guidelines for Buildings and Facilities (ADAAG)”. Alternate types of Temporary Walkways shall have the prior written approval of the engineer.

Temporary walkways shall be constructed and maintained so there are no abrupt changes in grade or terrain that could cause a tripping hazard or could be a barrier to wheelchair use. The contractor shall construct and maintain the walkway to ensure that joints in the walkway have a vertical difference in elevation of no more than one quarter (1/4") of an inch and that the horizontal joints have gaps no greater than one half (1/2") of an inch. The grade of the temporary walkway should parallel the grade of the existing walkway or roadway and the cross slope should be no greater than 2%.

A width of sixty (60") inches, if practical, should be provided throughout the entire length of any temporary walkway. The temporary walkway shall be a minimum width of forty eight inches (48"). When it is not possible to maintain a minimum width of sixty inches (60") throughout the entire length of temporary walkway, a
sixty inch (60") by sixty inch (60") passing space should be provided at least every two hundred feet (200 Ft.), to allow individuals in wheelchairs to pass.

Temporary walkways shall be constructed on firm subgrade. Compact the subgrade according to Section 209. Furnish and install any needed temporary pipes prior to constructing any walkway to ensure positive drainage away from or beneath the temporary walkway. Once the walkway is no longer required, remove any temporary materials and restore the area to the original conditions or as shown in the plans.

b. Temporary Curb Cut Wheelchair Ramps
Temporary curb cut wheelchair ramps shall be constructed in accordance with Section 441 and Detail A-3. Ramps shall also include a detectable warning surface in accordance with Detail A-4. Other types of material for the construction of the temporary curb cut wheelchair ramps, including the detectable warning surface, may be used provided the contractor can provide documentation that the material to be used meets the requirements of the “Americans with Disabilities Act Accessibility Guidelines for Buildings and Facilities (ADAAG)”. When a wheelchair ramp is no longer required, remove the temporary materials and restore the area to existing conditions or as shown in the plans. For the items required to restore the area to original conditions or as shown in the plans, measures for payment shall be covered by contract pay items. If pay items are not included in the contract, then payment for these items shall be included in Traffic Control-Lump Sum.

c. Temporary Audible Information Device
Temporary audible information devices, when shown in the plans, shall be installed in compliance with the “Americans with Disabilities Act Accessibility Guidelines for Buildings and Facilities (ADAAG)”. The devices shall be installed in accordance with the manufacturer’s recommendations. Prior to installation, the contractor shall provide the engineer with a set of manufacturer’s drawings detailing the proper installation procedures for each device. When no longer required, the devices shall remain the property of the contractor.

L. TRAFFIC SIGNALS
If the sequence of operations, staging, or the temporary traffic control plan requires the relocation or shifting of any components of an existing traffic signal system then any work on these traffic signals will be considered as part of Lump Sum- Traffic Control. The contractor becomes responsible for the maintenance of these traffic signals from the time that the system is modified until final acceptance. The maintenance of traffic signals that are not a part of the work and are not in conflict with any portion of the work shall not be the responsibility of the contractor.

When construction operations necessitate an existing traffic signal to be out of service, the Contractor shall furnish off-duty police officers to regulate and maintain traffic control at the site. Off-duty police officers may also be required to regulate and maintain traffic control at signal sites when lane closures or traffic shifts block or restrict movements.
causing interference with normal road user flows and will not allow the activated traffic signal to guide the traffic through the signal site.

M. REMOVAL/REINSTALLATION OF MISCELLANEOUS ITEMS

In the prosecution of the Work, if it becomes necessary to remove any existing signs, markers, guardrail, etc. not covered by specific pay item, they shall be removed, stored and reinstalled, when directed by the Engineer, to line and grade, and in the same condition as when removed.

150.03 SIGNS:

A. SIGNING REQUIREMENTS OF THE TEMPORARY TRAFFIC CONTROL (TTC) PLAN

When existing regulatory, warning or guide signs are required for proper traffic and pedestrian control the Contractor shall maintain these signs in accordance with the temporary traffic control (TTC) plan. The Contractor shall review the status of all existing signs, interim signs added to the work, and permanent sign installations that are part of the work to eliminate any conflicting or non-applicable signage in the TTC Plan. The Contractor’s review of all signs in the TTC Plan shall establish compliance with the requirements of the MUTCD and Section 150. Any conflicts shall be reported to the Engineer immediately and the WTCS shall take the necessary measures to eliminate the conflict.

The Contractor shall make every effort to eliminate the use of interim signs as soon as the Work allows for the installation of permanent signs.

All existing illuminated signs shall remain lighted and be maintained by the Contractor.

Existing street name signs shall be maintained at street intersections.

B. CONFLICTING OR NON-APPLICABLE SIGNS

Any sign(s) or portions of a sign(s) that are not applicable to the TTC plan shall be covered so as not to be visible to traffic or shall be removed from the roadway when not in use. The WTCS shall review all traffic shifts and changes in the traffic patterns to ensure that all conflicting signs have been removed. The review shall confirm that the highest priority signs have been installed and that signs of lesser significance are not interfering with the visibility of the high priority signs. High priority signs include signs for road closures, shifts, detours, lane closures and curves. Any signs, such as speed zones and speed limits, passing zones, littering fines and litter pick up, that reference activities that are not applicable due to the presence of the Work shall be removed, stored and reinstalled when the Work is completed.

Failure to promptly eliminate conflicting or non-applicable signs shall be considered as non-performance under Section 150.08.
C. REMOVAL OF EXISTING SIGNS AND SUPPORTS

The Contractor shall not remove any existing signs and supports without prior approval from the Engineer. All existing signs and supports which are to be removed shall be stored and protected if this material will be required later in the work as part of the TTC plan. If the signs are not to be utilized in the work then the signs will become the property of the Contractor unless otherwise specified in the contract documents.

D. INTERIM GUIDE, WARNING AND REGULATORY SIGNS

Interim guide, warning, or regulatory signs required to direct traffic and pedestrians shall be furnished, installed, reused, and maintained by the Contractor in accordance with the MUTCD, the Plans, Special Provisions, Special Conditions, or as directed by the Engineer. These signs shall remain the property of the Contractor. The bottom of all interim signs shall be mounted at least seven (7') feet above the level of the pavement edge when the signs are used for long-term stationary operations as defined by Section 6G.02 of the MUTCD. Special Conditions under Subsection 150.11 may modify this requirement.

Portable signs may be used when the duration of the work is less than three (3) days or as allowed by the special conditions in Subsection 150.11. Portable signs shall be used for all punchlist work. All portable signs and sign mounting devices utilized in work shall be NCHRP 350 compliant. Portable interim signs shall be mounted a minimum of one (1') foot above the level of the pavement edge for directional traffic of two (2) lanes or less and a minimum of seven (7') feet for directional traffic of three (3) or more lanes. Signs shall be mounted at the height recommended by the manufacturer’s crashworthy testing requirements. Portable interim signs which are mounted at less than seven (7') feet in height may have two 18 inch x 18 inch fluorescent red-orange or orange-red warning flags mounted on each sign.

All regulatory sign blanks shall be rigid whether the sign is mounted as a portable sign, on a Type III barricade or as a permanent mount height sign.

Any permanent mount height interim sign that is designed to fold in half to cover a non-applicable message on the sign shall have reflectorized material on the folded over portion of the sign. The reflectorized material shall be orange in color with a minimum of ASTM Type I engineering grade sheeting with a minimum area of six inches by six inches (6” x 6”) facing the direction of traffic at all times when the sign is folded.

Interim signs may be either English or metric dimensions.

E. EXISTING SPECIAL GUIDE SIGNS

Existing special guide signs on the Project shall be maintained until conditions require a change in location or legend content. When change is required, existing signs shall be modified and continued in use if the required modification can be made within existing sign borders using design requirements (legend, letter size, spacing, border, etc.) equal to that of the existing signs, or of Sub-Section 150.03.E.2. Differing legend designs shall not be mixed in the same sign.

1. Special Guide Signs

Special guide signs are those expressway or freeway guide signs that are designed with a message content (legend) that applies to a particular roadway location. When
an existing special guide sign is in conflict with work to be performed, the Contractor shall remove the conflicting sign and reset it in a new, non-conflicting location which has been approved by the Engineer.

2. Interim Special Guide Signs
When it is not possible to utilize existing signs, either in place or relocated, the Contractor shall furnish, erect, maintain, modify, relocate, and remove new interim special guide signs in accordance with the Plans or as directed by the Engineer. Interim special guide signs that may be required in addition to, or a replacement for, existing expressway and freeway (interstate) signs shall be designed and fabricated in compliance with the minimum requirements for guide signing contained in Part 2E “Guide Signs Expressway” and Part 2F “Guide Signs Freeways” of the MUTCD, except that the minimum size of all letters and numerals in the names and places, streets and highways on all signs shall be 16 inches Series “E” initial upper-case and 12 inches lower-case. All interstate shields on these signs shall be 48 inches and 60 inches for two-numeral and three-numeral routes, respectively.

The road name of the exit or route shield shall be placed on the exit gore sign.

3. Interim Overhead Guide Sign Structures
Interim overhead special guide sign structures are not required to be lighted unless specifically required by the Plans. If lighting is required the sign shall be lighted as soon as erected and shall remain lighted, during the hours of darkness, until the interim sign is no longer required. The Contractor shall notify the Power Company at least thirty (30) days prior to desired connection to the power source.

4. Permanent Special Guide Signs
The installation of new permanent special guide signs and the permanent modification or resetting of existing special guide signs, when included in the contract, shall be accomplished as soon as practical to minimize the use of interim special guide signs. If lighting is required by the Plans, all new permanent overhead special guide signs shall be lighted as soon as erected.

F. MATERIALS- INTERIM SIGNS:

1. Posts
Permanent mounting height of seven (7’) feet- Posts for all interim signs shall meet the requirements of Section 911 except that green or silver paint may be used in lieu of galvanization for steel posts or structural shape posts. Within the limits of a single project, all metal posts shall be the same color. Wood posts are not required to be pressure treated.

Interim posts may be either metric or English in dimensions.

Posts for all interim signs shall be constructed to yield upon impact unless the posts are protected by guardrail, portable barrier, impact attenuator or other type of positive barrier protection. Unprotected posts shall meet the breakaway requirements of the “1994 AASHTO Standard Specifications for Structural Support for Highway Signs, Luminaries and Traffic Signals”. Unprotected interim posts shall be spliced as shown in Detail 150-F unless full length unspliced posts are used.
Unprotected post splices will not be permitted any higher than four inches above the ground line to lessen the possibility of affecting the undercarriage of a vehicle. Installation of posts may require establishment of openings in existing pavements, islands, shoulders etc.
2. **Sign Blanks And Panels- Permanent mounting height of seven (7’) feet**
   All sign blanks and panels shall conform to Section 912 of the Specifications except that blanks and panels may be ferrous based or other metal alloys. Type 1 and Type 2 sign blanks shall have a minimum thickness of 0.08 inches regardless of the sign type used. Alternative sign blank materials (composites, poly carbonates, fiberglass reinforced plastics, recycled plastics, etc.) shall have a letter of approval from the Office of Materials and Research for use as interim construction signs before these materials are allowed to be incorporated into the work unless these rigid sign blanks are currently approved as a crashworthy sign blank material under QPL 34. The back side of sign panels shall be painted orange to prevent rust if other metals are used in lieu of aluminum. Plywood blanks or panels will not be permitted. The use of flexible signs will not be permitted for permanent mount height signs.

Interim blanks and panels may be either metric or English in dimensions.

3. **Portable Sign Mounting Devices, Portable Sign Blanks**
   All portable sign mounting devices and sign blanks utilized in the work shall be NCHRP 350 Test Level III compliant. All portable sign mounting devices and sign blanks shall be from the Qualified Products List. Any sign or sign mounting device shall have an identifying decal, logo, or manufacturer’s stamping that clearly identifies the device as NCHRP 350 compliant. The required decal, logo or manufacturer’s stamping shall not be displayed on the message face of the sign. The Contractor may be required to provide certification from the Manufacturer as proof of NCHRP 350 compliance. All portable signs shall be mounted according to height requirements of Subsection 150.03.D.

G. **SIGN VISIBILITY AND OFFSETS**

All existing, interim and new permanent signs shall be installed so as to be completely visible for an advance distance in compliance with the MUTCD. Any clearing required for maintaining the line of sight to existing, interim or permanent signs shall be done as part of the requirements of the TTC plan. The clearing shall include any advance warning signs, both interim and permanent, that are installed as a part of the work including advance warning signs that are installed outside the limits of the project. Any sign installed behind W-beam or T-beam guardrail with non-breakaway posts shall be installed with the leading edge of the sign a minimum of four feet and three inches (4’3”) behind the face of the guardrail with five feet (5’) of clearance being desirable. Limbs, brush, construction equipment and materials shall be kept clear of the driver’s line of sight to all signs that are part of the TTC plan.

H. **ADVANCE WARNING SIGNS:**

1. **All Type Of Highways**
   Advance warning signs shall be placed ahead of the work area in accordance with Part VI of the MUTCD and shall include a series of at least three advance road work (W20-1) signs placed at the termini of the project. The series shall have the legend ROAD WORK (1500 FEET, 1000 FEET, AND 500 FEET).

At grade intersecting roadways and on-ramps shall be signed with a minimum of one ROAD WORK AHEAD sign.
When work terminates at a "T" intersection, a minimum of one “ROAD WORK AHEAD” sign shall be placed in advance of the intersection and one “END ROAD WORK” sign shall be placed at the termination end of the intersection. Field conditions may require the use of additional warning signage.

Advanced Warning Signs on State Routes shall be a minimum dimension of 48 inches x 48 inches. When a State Route intersects a project which consists of adding travel lanes, reconstructing an existing roadway or new location work, the State Route approaches shall have a minimum of three (W20-1) advanced warning signs (1500 ft., 1000 ft., 500 ft.). The termination end of an intersecting State Route shall have END ROAD WORK signage.

The W20-1 signs shall be placed at the termini of the project or sufficiently in advance of the termini to allow for lane shifts, lane closures and other activities which may also require advanced warning signs. The advanced warning signs for the project should not overlap with the advanced warning signs for lane shifts, lane closures, etc.

The length of a workzone should be held to the minimum length required to accomplish the work. If a project has multiple individual worksites within the overall limits of the project, each site should be signed individually if the advance warning signs for each site can be installed without overlapping an adjacent worksite. As soon as the work is completed at any individual site the warning signs shall be removed from that site. Clean-up work and punchlist work shall be performed with portable signage.

Project mileage indicated on the G20-1 sign shall be the actual project mileage rounded up to the nearest whole mile. Projects less than two (2) miles in length or individual worksites that are part of a multiple worksite project may delete this sign. The G20-1 sign shall be 60” X 36” and the G20-2 sign shall be 48” X 24”.

2. Interstate, Limited Access And Multilane Divided Highways

In addition to the W20-1 signs required at 500 ft., 1000 ft. and 1500 ft., multi-lane divided highways shall also have additional advanced warning signs installed with the legend "ROAD WORK (2 MILES, 1 MILE and 1/2 MILE). All construction warning signs on divided highways shall be double indicated (i.e., on the left and right sides of the roadway.) If the use of the ½ mile, 1 mile and 2 mile advanced warning signs cause an overlap with other work or do not benefit field conditions then the Engineer may review the use of these signs and eliminate their installation. When the posted speed limit is 50 MPH or less, the ½ mile, 1 mile and 2 mile signs should be eliminated especially in urban areas.

The W20-1 advance warning signs for ROAD WORK 500 FEET; 1000 FEET; and 1500 FEET shall be temporarily covered when work involving the advanced warning signs for lane shifts and lane closures overlap these signs. The ROAD WORK ½ MILE, ROAD WORK 1 MILE, and ROAD WORK 2 MILES shall be in place when the 500, 1000 and 1500 feet signs are temporarily covered.

When the temporary traffic control zone already has advanced warning (W20-1) signs installed the W20-1 signs required for lane closures under Standard 9106 should be eliminated.
RAMP WORK ON LIMITED ACCESS HIGHWAYS: The workzone shall not be signed for the entire length of the mainline of a limited access highway when only short individual worksites, interchange or ramp work is being performed.

When work is restricted to ramp reconstruction or widening activities, the advance warning signs on the mainline section of the limited access highway shall be limited to the use of portable advance warning signs. These portable advance warning signs shall only be utilized when work activity is within the gore point of the ramp and the mainline traveled way or work is active in the accel/decel lane adjacent to the mainline traveled way. Portable advance warning signs (W20-1; 1500ft. /1000 ft. /500ft.) shall be installed on the traveled way of the limited access highway when the above conditions are present. The advance warning signs shall be installed only in one direction where work is active. All portable signs shall be double indicated. When work is not active, the ramp work shall be advanced warned by the use of a single 48 inch X 48 inch “RAMP WORK AHEAD” sign along the right shoulder of the mainline traveled way prior to the beginning of the taper for the decel lane. The “RAMP WORK AHEAD” sign shall be mounted at seven (7’) feet in height. Differences in elevation shall be in compliance with the requirements of Subsection 150.06 prior to the removal of the portable (W20-1) advanced warning signs from the mainline.

The G20-1 sign shall be eliminated on limited access highways when the work involves only ramp work, bridge reconstruction, bridge painting, bridge joint repairs, guardrail and anchor replacement or other site specific work which is confined to a short section of limited access highway.

I. PORTABLE CHANGEABLE MESSAGE SIGN

Unless specified as a paid item in the contract the use of a portable changeable message sign will not be required. When specified, a portable changeable message sign (PCMS) shall meet the minimum requirements of Section 632 and the MUTCD. The maximum amount of messages allowed to be flashed on one PCMS is two phases (flashes). The language and the timing of the messages shall comply with the MUTCD and Section 632. When used as an advanced device the PCMS should typically be placed ahead of the construction activities. If the PCMS is used as a substitute for another device then the requirements for the other device apply.

J. FLASHING BEACON

The flashing beacon assembly, when specified, shall be used in conjunction with construction warning signs, regulatory, or guide signs to inform traffic of special road conditions which require additional driver attention. The flashing beacon assembly shall be installed in accordance with the requirements of Section 647.

K. RUMBLE STRIP SIGNAGE

Signage for rumble strips located in the travelway shall be as required in Subsection 150.01.C and Subsection 150.02.A.9.

L. LOW/SOFT SHOULDER SIGNAGE

Low or soft shoulder signs shall be utilized in accordance with the following conditions:
CONSTRUCTION/RECONSTRUCTION PROJECTS:

“LOW/SOFT SHOULDER” signs shall be erected when a difference in elevation exceeds one (1”) inch but does not exceed three (3”) inches between the travelway and any type of shoulder unless the difference in elevation is four (4’) feet or greater from the edge of the traveled way.

The spacing of the signs shall not exceed one (1) mile and the signs shall be placed immediately past each crossroad intersection. The “Low/Soft” signs shall remain in place until the difference in elevation is eliminated and the shoulder has been dressed and permanently grassed for a minimum of thirty (30) calendar days. These signs shall be furnished, installed, maintained and removed by the Contractor as part of Traffic Control-Lump Sum. These signs shall be orange with black borders and meet the reflectorization requirements of Subsection 150.01.C.

“SHOULDER DROP-OFF” (W8-9a) signs shall be used when a difference in elevation, less than four (4’) feet from the traveled way, exceeds three (3”) inches and is not protected by positive barrier protection. These warning signs shall be placed in advance of the drop-off. For a continuous drop-off condition, the W8-9a) signs shall, as a minimum, be spaced in accordance with the above requirements for “Low/soft shoulder” signs.

PROJECTS CONSISTING PRIMARILY OF ASPHALTIC CONCRETE RESURFACING ITEMS:

“LOW/SOFT SHOULDER” signs shall be erected when a difference in elevation exceeds one (1”) inch but does not exceed three (3”) inches between the travelway and any type of shoulder unless the difference in elevation is four (4’) feet or greater from the edge of the traveled way.

SHOULDER BUILDING INCLUDED IN THE CONTRACT: “Low/Soft Shoulder” signs shall be erected as per the requirement of Standards 9102, 9106, and 9107. “Shoulder Drop-off” signs (W8-9a) shall be erected as per the requirements of the MUTCD. These signs shall be maintained until the conditions requiring their installation have been eliminated. The Contractor shall remove all interim warning signs before final acceptance.

SHOULDER BUILDING NOT INCLUDED IN THE CONTRACT: The Department will furnish the “Low/Soft Shoulder” signs, “Shoulder Drop-off” signs and the posts. The signs shall be erected to meet the minimum requirements of Subsection 150.03. The Contractor shall include the cost of furnishing installation hardware (bolts, nuts, and washers), erection and maintenance of the signs in the bid price for Traffic Control- Lump Sum. The Contractor shall maintain the signs until final acceptance. The Department will remove the signs.

LAU/LAR PROJECTS SHOULDER BUILDING NOT INCLUDED IN THE CONTRACT: The Contractor will furnish, install and maintain LOW/SOFT SHOULDERS (yellow with black borders, ASTM Type III or IV) at the appropriate spacing, until Final Acceptance of the project by the Department. After Final Acceptance by the Department the signs will become the property and responsibility of the local government.

M. BUMP SIGNAGE:

MULTI-LANE DIVIDED HIGHWAYS: A bump sign (W8-1) shall be utilized when a transverse joint in the pavement structure has a vertical difference in elevation of three quarters
(3/4") of an inch or greater in depth with no horizontal taper to ramp the traffic from one elevation to the other. This condition typically occurs at approach slabs during pavement milling operations and at transverse joints in asphaltic pavement lifts.

TWO-LANE TWO-WAY HIGHWAYS: A bump sign (W8-1) shall be utilized when a transverse joint in the pavement structure has a vertical difference in elevation that exceeds one and three quarters (1-3/4") inches in depth with no horizontal taper to ramp the traffic from one elevation to the other. This includes utility and storm drainage repairs that require concrete placement for patching and/or steel plating.

The (W8-1) sign shall be placed sufficiently in advance to warn the motorist of the condition.

N. PEDESTRIAN SIGNAGE:

Appropriate signs as described in the MUTCD shall be maintained to allow safe passage of pedestrian traffic or to advise pedestrians of walkway closures (Refer to MUTCD Figures TA-28 and TA-29 for guidance). Advance closure signing should be placed at intersections rather than midblock locations so that pedestrians are not confronted with midblock work sites that will induce them to attempt skirting the work site or making a midblock crossing. Signs and other devices mounted lower than seven (7) feet above the temporary pedestrian walkway shall not project more than four (4) inches into the accessible pedestrian facilities. Signs and other devices shall be placed such that they do not narrow any pedestrian passage to less than 48 inches.

150.04 PAVEMENT MARKINGS

A. GENERAL

Full pattern pavement markings in accordance with Section 652 and in conformance with Section 3A and 3B, except 3B.02, of the MUTCD are required on all courses before the roadway is opened to traffic. No passing zones shall be marked to conform to Subsection 150.04.E. During construction and maintenance activities on all highways open to traffic, both existing markings and markings applied under this Section shall be fully maintained until Final Acceptance. If the pavement markings are, or become, unsatisfactory in the judgement of the Engineer due to wear, weathering, or construction activities, they shall be restored immediately.

1. Resurfacing Projects

Pavement markings shall be provided on all surfaces that are placed over existing markings. Interim and final markings shall conform in type and location to the markings that existed prior to resurfacing unless changes or additions are noted in the Contract. The replacement of parking spaces will not be required unless a specific item or note has been included in the Contract. Any work to make additions to the markings that existed prior to resurfacing is to be considered as extra work.
2. **Widening And Reconstruction Projects**
   If the lane configuration is altered from the preconstruction layout then pavement markings will be as required by the plans or the Engineer.

3. **New Location Construction Projects**
   Pavement marking plans will be provided.

**B. MATERIALS**

All traffic striping applied under this Section shall be a minimum four inches in width or as shown in plans and shall conform to the requirements of Section 652, except as modified herein. Raised pavement markers (RPMs) shall meet the requirements of Section 654. Markings on the final surface course, which must be removed, shall be a removable type. The Contractor will be permitted to use paint, thermoplastic, or tape on pavement which is to be overlaid as part of the project, unless otherwise directed by the Engineer. Partial (skip) reflectorization (i.e. reflectorizing only a portion of a stripe) will not be allowed.

**C. INSTALLATION AND REMOVAL OF PAVEMENT MARKINGS:**

**INSTALLATION:** All pavement markings, both interim and permanent, shall be applied to a clean surface. The Contractor shall furnish the layout and preline the roadway surface for the placement of pavement markings applied as part of the temporary traffic control plan. All interim marking tape and RPM’s on the final surface shall be removed prior to the placement of the final markings.

The Contractor shall sequence the work in such a manner as to allow the installation of markings in the final lane configuration at the earliest possible stage of the work.

**REMOVAL:** Markings no longer applicable shall be removed in accordance with Subsection 656.2.

**THE ELIMINATION OF CONFLICTING PAVEMENT MARKINGS BY OVERPAINTING WITH UNAPPROVED PAINT OR ANY TYPE OF LIQUID ASPHALT IS NOT ACCEPTABLE.**

**INTERMEDIATE SURFACE:** Interim markings shall be removed by methods that will cause minimal damage to the pavement surface while also ensuring that traveling public will not be confused or misdirected by any residual markings remaining on the intermediate surface. The use of approved black-out tape and black-out paint (manufactured for the sole purpose of covering existing pavement markings) may be permitted on some interim surfaces, provided the results are satisfactory to the Engineer.

**FINAL SURFACE:** No interim paint or thermoplastic markings will be permitted on any final surface unless the interim markings are in alignment with the location of the permanent markings and the interim marking will not interfere or adversely affect placement of the permanent markings. The proposed method of removal for layout errors that require markings to be removed from the final surface shall have the prior approval of the Engineer. Any damage to the final pavement surface caused by the pavement marking removal process shall be repaired at the Contractor’s expense by methods acceptable and approved by the Engineer. Subsection 400.3.06.C shall apply when corrective measures are
required. The use of black-out tape or black-out paint will not be permitted under any circumstance to correct layout errors on any final surface.

Traffic shifts that are done on the final surface shall be accomplished using interim traffic marking tape that can be removed without any blemishing of the final surface. Interim traffic marking tape shall be used on any of the following final surfaces; asphaltic concrete, Portland cement concrete, and bridge deck surfaces. The contractor may propose alternate traffic markings and removal methods on the final surface. Submitted proposals shall include the type of material, method of removal and a cost comparison to the traffic marking tape method. Prior to any approval, the contractor shall field demonstrate to the satisfaction of the Engineer that the proposed traffic markings can be removed without any blemishing of the final surface. If the proposal is determined to be acceptable, a supplemental agreement will be executed prior to the installation of the proposed alternate traffic markings. The supplemental agreement shall denote the type of traffic marking materials, method of removal and any cost and/or time savings to the Department. The Department will not consider or participate in any cost increase that may result from implementing the proposed alternate method.

PAY FACTOR REDUCTION FOR ASPHALTIC CONCRETE FINAL SURFACES: When the correction of an error in the layout of the final pavement markings requires the final surface to be grounded, blemished, scarred, or polished the pay factor shall be reduced to 0.95 for the entire surface area of the final topping that has a blemish, polished or a scarred surface. The reduced pay factor shall not be confined to only the width and length of the stripe or the dimensions of the blemished areas, the whole roadway surface shall have the reduced pay factor applied. The area of the reduced pay factor shall be determined by the total length and the total width of the roadway affected. If the affected area is not corrected, the reduction in pay shall be deducted from the final payment for the topping layer of asphaltic concrete. The Engineer shall make the final determination whether correction or a reduced pay factor is acceptable.

The eradication of pavement markings on intermediate and final concrete surfaces shall be accomplished by a method that does not grind, polish, or blemish the surface of the concrete. The method used for the removal of the interim markings shall not spall chip the joints in the concrete and shall not damage the sealant in the joints. Any joint or sealant repairs shall be included in the bid price for Traffic Control-Lump Sum. The proposed method of removal shall have the prior approval of the Engineer.

Failure to promptly remove conflicting or non-applicable pavement markings shall be considered as non-performance under Subsection 150.08.

PREPARATION AND PLANNING FOR TRAFFIC SHIFTS: When shifting of traffic necessitates removal of centerline, lane lines, or edge lines, all such lines shall be removed prior to, during, or immediately after any change so as to present the least interference with traffic. Interim traffic marking tape shall be used as a temporary substitute for the traffic markings being removed.

Before any change in traffic lane(s) alignment, marking removal equipment shall be present on the project for immediate use. If marking removal equipment failures occur, the equipment shall be repaired or replaced (including leasing equipment if necessary), so that the removal can be accomplished without delay.
Except for the final surface, markings on asphaltic concrete may be obliterated by an overlay course, when approved by the Engineer. When an asphaltic concrete overlay is placed for the sole purpose of eliminating conflicting markings and the in place asphaltic concrete section will allow, said overlay will be eligible for payment only if designated in the Plans. Overlays to obliterate lines will be paid for only once and further traffic shifts in the same area shall be accomplished with removable markings. Only the minimum asphaltic concrete thickness required to cover lines will be allowed. Excessive build-up will not be permitted. When an overlay for the sole purpose of eliminating conflicting markings is not allowed, the markings no longer applicable shall be removed in accordance with Subsection 656.2.

D. RAISED PAVEMENT MARKERS

Raised pavement markers (RPMs) are required as listed below for all asphaltic concrete pavements before the roadway is open to traffic. On the final surface, RPM’s shall be placed according to the timeframes specified in 150.04 E. for full pattern pavement markings except Interstate Highways where RPM’s shall be placed and/or maintained when the roadway is open to traffic. When Portland Cement Concrete is an intermediate or final surface and is open to traffic, one calendar day is allowed for cleaning and drying before the installation of RPMs is required.

Raised pavement markers are not allowed on the right edge lines under any situation.

1. Interstate Highways

   Retro-reflective raised pavement markers (RPM’s) shall be placed and/or maintained on intermediate pavements surfaces on all interstate highways that are open to traffic. This includes all resurfacing projects along with widening and reconstruction projects. The spacing and placement shall be as required for MULTI-LANE DIVIDED HIGHWAYS.

2. Multi-Lane Divided Highways

   Retro-reflective raised pavement markers (RPMs) shall be placed and/or maintained on intermediate pavement surfaces on all multi-lane divided highways that are open to traffic when these roadways are being widened or reconstructed. Two lane-two way roadways that are being widened to a multi-lane facility, whether divided or undivided, are included in this provision. Projects consisting primarily of asphalt resurfacing items or shoulder widening items are excluded from this requirement. The RPMs shall be placed as follows:

   a. SUPPLEMENTING LANE LINES

      80 foot center on skip lines with curvature less than three degrees. (Includes tangents)

      40 foot centers on solid lines and all lines with curvature between three degrees and six degrees.

      20 foot centers on curves over six degrees.

      20 foot centers on lane transitions or shifts.
b. **SUPPLEMENTING RAMP GORE LINES**

20 foot centers, two each, placed side by side.

c. **OTHER LINES**

As shown on the plans or directed by the Engineer.

3. **Other Highways**

On other highways under construction RPMs shall be used and/or maintained on intermediate pavement surfaces as follows:

a. **SUPPLEMENTING LANE LINES AND SOLID LINES**

40 foot centers except on lane shifts. (When required in the Plans or Contract.)

20 foot centers on lane shifts. (Required in all cases.)

b. **SUPPLEMENTING DOUBLE SOLID LINES**

40 foot centers (one each beside each line) except on lane shifts. (When required in the Plans or Contract.)

20 foot centers on lane shifts. (Required in all cases.)

E. **EXCEPTIONS FOR INTERIM MARKINGS**

Some exceptions to the time of placement and pattern of markings are permitted as noted below; however, full pattern pavement markings are required for the completed project.

1. **Two-Lane, Two-Way Roadways**

   a. **SKIP LINES**

   All interim skip (broken) stripe shall conform to [Section 652](#) except that stripes shall be at least two feet long with a maximum gap of 38 feet. On curves greater than six degrees, a one-foot stripe with a maximum gap of 19 feet shall be used. In lane shift areas solid lines will be required. Interim skip lines shall be replaced with markings in full compliance with [Section 652](#) prior to expiration of the 14 calendar day period.

   Interim raised pavement markers may be substituted for the interim skip (broken) stripes. If raised pavement markers are substituted for the two foot interim skip stripe, three markers spaced at equal intervals over a two feet distance will be required. No separate payment will be made if the interim raised pavement markers are substituted for interim skip lines.

   Interim raised pavement markers shall be retro-reflective, shall be the same color as the pavement markers for which they are substituted, and shall be visible during daytime.
The type of interim marker and method of attachment to the pavement shall be approved by the Office of Materials and Research but in no case will the markers be attached by the use of nails. Flexible reflective markers, Type 14 or Type 15, may be used for a maximum of fourteen (14) calendar days as an interim marker. Any flexible reflective markers in use shall be from the qualified products list (QPL).

The interim raised pavement markers shall be maintained until the full pattern pavement markings are applied. At the time full pattern markings are applied the interim raised markers shall be removed in a manner that will not interfere with application of the full pattern pavement markings.

b. NO PASSING ZONES—TWO-LANE, TWO-WAY ROADWAYS
Passing zones shall be re-established in the locations existing prior to resurfacing. No changes to the location of passing zones shall be done without the written approval of the Engineer. For periods not to exceed three calendar days where interim skip centerlines are in place, no-passing zones shall be identified by using post or portable mounted DO NOT PASS regulatory signs (R4-1 24” x 30”) at the beginning and at intervals not to exceed ½ mile within each no-passing zone. A post or portable mounted PASS WITH CARE regulatory sign (R4-1 24” x 30”) shall be placed at the end of each no-passing zone. Post mounted signs shall be placed in accordance with the MUTCD. Portable signs shall conform to the requirements of the MUTCD and shall be NCHRP 350 compliant. Portable signs shall be secured in such a manner to prevent misalignment and minimize the possibility of being blown over by weather conditions or traffic.

On new location projects and on projects where either horizontal or vertical alignments has been modified, the location of No-Passing Zones will be identified by the Engineer.

c. EDGELINES

1) Bituminous Surface Treatment Paving
Edgelines will not be required on intermediate surfaces (including asphaltic concrete leveling for bituminous surface treatment paving) that are in use for a period of less than 60 calendar days except at bridge approaches, on lane transitions, lane shifts, and in such other areas as determined by the Engineer. On the final surface, edgelines shall be placed within 30 calendar days of the time that the final surface was placed.

2) All Other Types of Pavement
Edgelines will not be required on intermediate surfaces that are in use for a period of less than 30 calendar days except at bridge approaches, on lane transitions, lane shifts, and in such other areas as determined by the Engineer. On the final surface, edgelines shall be placed within 14 calendar days of the time that the surface was placed.

2. Multi-Lane Highways—With No Paved Shoulder(S) Or Paved Shoulder(S) Four Feet Or Less

a. UNDIVIDED HIGHWAYS (INCLUDES PAVED CENTER TURN LANE)
1) Centerlines and No-Passing Barrier-Full Pattern centerlines and no-passing barriers shall be restored before opening to traffic.

2) Lanelines- Interim skip (broken) stripe as described in Subsection 150.04E.1.a. may be used for periods not to exceed three calendar days. Skiplines are not permitted in lane shift areas. Solid lines shall be used.

3) Edgelines- Edgelines shall be placed on intermediate and final surfaces within three calendar days of obliteration.

b. DIVIDED HIGHWAYS (GRASS OR RAISED MEDIAN)

1) Lanelines- Full pattern skip stripe shall be restored before opening to traffic. Skip lines are not permitted in lane shift areas. Solid lines shall be required.

2) Centerline/Edgeline- Solid lines shall be placed on intermediate and final surfaces within three calendar days of obliteration.

3. Limited Access Roadways And Roadways With Paved Shoulders Greater Than Four Feet

a. Same as Subsection 150.04.E.2 except as noted in (b) below.

b. EDGELINES-

1) Asphaltic Concrete Pavement- Edgelines shall be placed on intermediate and final surfaces prior to opening to traffic.

2) Portland Cement Concrete Pavement- Edgelines shall be placed on any surface open to traffic no later than one calendar day after work is completed on a section of roadway. All water and residue shall be removed prior to daily striping.

4. Ramps For Multi-Lane Divided Highways

A minimum of one solid line edge stripe shall be placed on any intermediate surface of a ramp prior to opening the ramp to traffic. The other edge stripe may be omitted for a maximum period of three (3) calendar days on an intermediate surface. Appropriate channelization devices shall be spaced at a maximum of twenty-five (25') feet intervals until the other stripe has been installed.

The final surface shall have both stripes placed prior to opening the ramp to traffic.

5. MISCELLANEOUS PAVEMENT MARKINGS:

FINAL SURFACE: School zones, railroads, stop bars, symbols, words and other similar markings shall be placed on final surfaces conforming to Section 652 within fourteen (14) calendar days of completion of the final surface. Final markings shall conform to the type of pay item in the plans. When no pay item exists in the plans the final markings shall conform to Section 652 for painted markings.
INTERMEDIATE SURFACE: Intermediate surfaces that will be in use for more than forty-five (45) calendar days shall have the miscellaneous pavement markings installed to conform to the requirement of Section 652. Under Subsection 150.11, Special Conditions, or as directed by the Engineer these markings may be eliminated.

F. MOBILE OPERATIONS

When pavement markings (centerlines, lane lines, and edgelines) are applied in a continuous operation by moving vehicles and equipment, the following minimum equipment and warning devices shall be required. These devices and equipment are in addition to the minimum requirements of the MUTCD.

1. All Roadways
   All vehicles shall be equipped with the official slow moving vehicle symbol sign. All vehicles shall have a minimum of two flashing or rotating beacons visible in all directions. All protection vehicles shall have an arrow panel mounted on the rear. All vehicles requiring an arrow panel shall have, as a minimum, a Type B panel. All vehicle mounted signs shall be mounted with the bottom of the sign a minimum height of forty-eight inches (48”) above the pavement. All sign legends shall be covered or removed from view when work is not in progress.

2. Two-Lane Two-Way Roadways
   a. Lead Vehicles
      The lead vehicle may be a separate vehicle or the work vehicle applying the pavement markings may be used as the lead vehicle. The lead vehicle shall have an arrow panel mounted so that the panel is easily visible to oncoming (approaching) traffic. The arrow panel should typically operate in the caution mode.

   b. Work Vehicles
      The work vehicle(s) applying markings shall have an arrow panel mounted on the rear. The arrow panel should typically operate in the caution mode. The work vehicle placing cones shall follow directly behind the work vehicle applying the markings.

   c. Protection Vehicles
      A protection vehicle may follow the cone work vehicle when the cones are being placed and may follow when the cones are being removed.

3. MULTI-LANE ROADWAYS
   A lead vehicle may be used but is not required. The work vehicle placing cones shall follow directly behind the work vehicle applying the markings. A protection vehicle that does not function as a work vehicle should follow the cone work vehicle when traffic cones are being placed. A protection vehicle should follow the cone work vehicle when the cones are being removed from the roadway. Protection vehicles shall display a sign on the rear of the vehicle with the legend PASS ON LEFT (RIGHT).

INTERSTATES AND LIMITED ACCESS ROADWAYS: A protection vehicle shall follow the last work vehicle at all times and shall be equipped with a truck mounted attenuator
that is certified for impacts not less than 62 mph in accordance with NCHRP350 Test Level Three (3).

150.05 CHANNELIZATION

A. GENERAL

Channelization shall clearly delineate the travelway through the work zone and alert drivers and pedestrians to conditions created by work activities in or near the travelway. Channelization shall be done in accordance with the plans and specifications, the MUTCD, and the following requirements.

All Channelization Devices utilized on any project shall be NCHRP 350 compliant. Any device used on the Work shall be from the Qualified Products List. All devices utilized on the work shall have a decal, logo, or manufacturer’s stamping that clearly identifies the device as NCHRP 350 compliant. The Contractor may be required to furnish certification from the Manufacturer for any device to prove NCHRP 350 compliance.

1. Types of Devices Permitted for Channelization in Construction Work Zones:
   
a. DRUMS:

   1) DESIGN: Drums shall meet the minimum requirement of the MUTCD and shall be reflectorized as required in Subsection 150.01.C. The upper edge of the top reflectorized stripe on the drum shall be located a minimum of 33 inches above the surface of the roadway. A minimum drum diameter of 18 inches shall be maintained for a minimum of 34 inches above the roadway.

   2) APPLICATION: Drums shall be used as the required channelizing device to delineate the full length of a lane closure, shift, or encroachment, except as modified by this Subsection.

   3) TRANSITION TAPERS FOR LANE CLOSURES: Drums shall be used on all transition tapers. The minimum length for a merging taper for a lane closure on the travelway shall be as shown in Table 150-1:
TABLE 150-1

<table>
<thead>
<tr>
<th>Posted Speed Limit, MPH</th>
<th>Lane Width 9 Feet</th>
<th>Lane Width 10 Feet</th>
<th>Lane Width 11 Feet</th>
<th>Lane Width 12 Feet</th>
<th>Maximum Drum Spacing in Tapers, (Feet)</th>
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</thead>
<tbody>
<tr>
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<td>70</td>
<td>75</td>
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<td>75</td>
</tr>
</tbody>
</table>

If site conditions require a longer taper then the taper shall be lengthened to fit particular individual situations.

The length of shifting tapers should be at least ½ L.

The length of a closed lane or lanes, excluding the transition taper(s), shall be limited to a total of two (2) miles. Prior approval must be obtained from the Engineer before this length can be increased.

Night time conditions: When a merge taper exists into the night all drums located in the taper shall have, for the length of the taper only, a six (6”) inch fluorescent orange (ASTM Type VI, VII, VIII, IX or X) reflectorized top stripe on each drum. The top six-inch stripe may be temporarily attached to the drum while in use in a taper. The Engineer may allow the fluorescent orange reflectorized six (6”) inch top stripe on each drum in a merging taper to remain in place during daylight hours provided there is a lane closure(s) with a continuous operation that begins during one nighttime period and ends during another nighttime period. All drums that have the six-inch top stripe permanently attached shall not be used for any other conditions.

Multiple Lane Closures:

(a) A maximum of one lane at a time shall be closed with each merge taper.

(b) A minimum tangent length of 2 L shall be installed between each individual lane closure taper.

4) LONGITUDINAL CHANNELIZATION: Drums shall be spaced as listed below for various roadside work conditions except as modified by Subsection 150.06.
Spacing shall be used for situations meeting any of the conditions listed as follows:

(a) 40 FOOT SPACING MAXIMUM

(1) For difference in elevation exceeding two inches.

(2) For healed sections no steeper than 4:1 as shown in Subsection 150.06, Detail 150-E.

(b) 80 FOOT SPACING MAXIMUM

(1) For difference in elevation of two inches or less.

(2) Flush areas where equipment or workers are within ten feet of the travel lane.

(c) 200 FOOT SPACING MAXIMUM: Where equipment or workers are more than ten feet from travel lane. Lateral offset clearance to be four feet from the travel lane.

(1) For paved areas eight feet or greater in width that are paved flush with a standard width travel lane.

(2) For disturbed shoulder areas not completed to typical section that are flush to the travel lane and considered a usable shoulder.

REMOVAL OF DRUMS: Drums may be removed after shoulders are completed to typical section and grassed. Guardrail and other safety devices shall be installed and appropriate signs advising of conditions such as soft or low shoulder shall be posted before the drums are removed.

b. VERTICAL PANELS

1) DESIGN: All vertical panels shall meet the minimum requirements of the MUTCD. All vertical panels shall have a minimum of 270 square inches of retro-reflective area facing the traffic and shall be mounted with the top of the reflective panel a minimum of 36” above the roadway.

2) APPLICATION: Lane encroachment by the drum on the travelway should permit a remaining lane width of ten feet. When encroachment reduces the travelway to less than ten feet, vertical panels shall be used to restore the travelway to ten feet or greater. No other application of vertical panels will be permitted.

c. CONES

1) DESIGN: All cones shall be a minimum of 28 inches in height regardless of application and shall meet the requirement of the MUTCD. Reflectorization may be deleted from all cones.

2) APPLICATION: For longitudinal channelizing only, cones will be permitted for daylight closures or minor shifts. (Drums are required for all tapers.) The use
of cones for nighttime work will not be permitted. Cones shall not be stored or allowed to be visible on the worksite during nighttime hours.

d. BARRICADES

DESIGN: Type III barricades shall meet the minimum requirements of the MUTCD and shall be reflectorized as required in Subsection 150.01.C. The Contractor has the option of choosing Type III barricades from the Qualified Products List or the Contractor may utilize generic barricades that are approved by the Federal Highway Administration (FHWA). When barricades have been specifically crash tested with signs attached, the contractor has the responsibility to attach the signs as per the manufacturer's recommendations to ensure crashworthiness. If signs are attached to generic barricades or to barricades from the Qualified Products List (QPL) that have not been crash tested with signs attached then the responsibility for crashworthiness and the liability for mounting these signs to the barricades are assumed by the Contractor and the Contractor shall certify that the barricades are crashworthy under FHWA workzone guidelines for NCHRP 350 crashworthy compliance. Any generic barricades used in the work shall be stamped or stenciled to show compliance with NCHRP 350. The use of Type I and Type II barricades will not be permitted.

1) APPLICATION: Type III barricades shall be placed as required by the plans, the Standards, and as directed by the Engineer. All signs mounted on barricades shall be mounted to comply with the requirements of the MUTCD and NCHRP 350 Test Level III. NCHRP 350 crashworthy compliance may require that rigid signs be mounted separate from the Type III barricade.

When a barricade is placed so that it is subject to side impact from a vehicle, a drum shall be placed at the side of the barricade to add target value to the barricade.

e. WARNING LIGHTS:

1) DESIGN: All warning lights shall meet the requirements of the MUTCD.

2) APPLICATION

(a) Type A low-intensity flashing lights shall be used as shown in the Plans, the Standards, and as directed by the Engineer. Flashing lights are not required for advance warning signs in Subsection 150.03.H.

(b) Type C Steady-Burn lights shall be used as shown in the Plans, the Standards, and as directed by the Engineer. Steady-burn lights are not required on drums for merging tapers that exist into the night.

f. TEMPORARY BARRIERS

1) DESIGN: Temporary barriers shall meet the requirements of Sections 620.

2) APPLICATION: Temporary barriers shall be placed as required by the plans, standards, and as directed by the Engineer. When Temporary barrier is located
20 feet or less from a travel lane, yellow reflectors shall be fixed to the top of the barrier at intervals not greater than 40 feet in the longitudinal section and 20 feet in the taper section and shall be mounted approximately two inches above the barrier. If both lanes of a two-lane two-way roadway are within 20 feet or less of the barrier then the reflectors shall be installed for both directions of traffic.

The reflectors shall be 100 square inches (ASTM Type VII or VIII) reflective sheeting mounted on flat-sheet blanks. The reflectors shall be mounted approximately two inches above the top of the barrier. The reflectors shall be attached to the barrier with adhesive or by a drilled-in anchor type device. The reflectors shall not be attached to a post or board that is placed between the gap in the barrier sections.

Approach end of Temporary barrier shall be flared or protected by an impact attenuator (crash cushion) or other approved treatment in accordance with Construction Details/Standards and Standard Specifications.

On interstate or other controlled access highways where lane shifts or crossovers cause opposing traffic to be separated by less than 40 ft., portable barrier shall be used as a separator.

B. PORTABLE IMPACT ATTENUATORS:

1. DESCRIPTION
   This work consists of the furnishing (including spare parts), installation, maintenance, relocation, reuse as required, and removal of Portable Impact Attenuator Units/Arrays.

2. MATERIALS
   Materials used in the Attenuator shall meet the requirements of Section 648 for Portable Impact Attenuators.

3. CONSTRUCTION
   Portable Impact Attenuator Unit/Arrays installation shall conform to the requirements of Section 648, Manufacturer’s recommendations and Georgia Standard 4960 and shall be installed at locations designated by the Engineer, and/or as shown on the plans.

C. TEMPORARY GUARDRAIL ANCHORAGE- Type 12:

1. DESCRIPTION
   This work consists of the furnishing, installation, maintenance and removal or Temporary Guardrail Anchorage- Type 12 used for Portable Barrier or temporary guardrail end treatment.

2. MATERIALS
   Materials used in the Temporary Guardrail Anchorage- Type 12 shall meet the requirements of Subsection 641.2 of the Specifications and current Georgia Standards and may be new or used. Materials salvaged from the Project which meet the requirements of Standards may be utilized if available. The use of any salvaged materials will require prior approval of the Engineer.
3. CONSTRUCTION
Installation of the Temporary Guardrail Anchorage- Type 12 shall conform to the requirements of the Plans, current Georgia Standards and Subsection 641.3 of the Specifications. Installation shall also include sufficient additional guardrail and appurtenances to effect the transition and connection to Temporary Concrete Barrier as required by the details in Georgia Standard 4960.

150.06 DIFFERENCES IN ELEVATION BETWEEN TRAVEL LANES AND SHOULDERS (SEE SUBSECTION 150.06.G FOR PROJECTS CONSISTING PRIMARILY OF ASPHALTIC CONCRETE RESURFACING ITEMS)

Any type of work such as paving, grinding, trenching, or excavation that creates a difference in elevation between travel lanes or between the travelway and the shoulder shall not begin until the Contractor is prepared and able to continuously place the required typical section to within two inches (2”) of the existing pavement elevation. For any areas that the two inches minimum difference in elevation cannot be accomplished the section shall be healed as shown in Detail 150-E. If crushed stone materials are used to provide a healed section no separate payment will be made for the material used to heal any section. The Contractor may submit a plan to utilize existing pay items for crushed stone provided the plan clearly demonstrates that the materials used to heal an area will be incorporated into the work with minimal waste. Handling and hauling of any crushed stone used to heal shall be kept to a minimum. The Engineer shall determine if the crushed stone used to heal meets the specifications for gradation and quality when the material is placed in the final location.

A maximum of sixty (60) calendar days shall be allowed for conditions to exist that require any section or segment of the roadway or ramp to continue to require a healed section as described by Detail 150-E. Failure to meet this requirement shall be considered as non-performance of Work under Subsection 150.08.

When trenching or excavation for minor roadway or shoulder widening is required, all operations at one site shall be completed to the level of the existing pavement in the same work day.

Any channelization devices utilized in the work shall conform to the requirements of Subsection 150.05 and to the placement and spacing requirements in Details 150-B, 150-C, 150-D, and 150-E shown in this section.

Any construction activity that reduces the width of a travel lane shall require the use of a W-20 sign with the legend “LEFT/RIGHT LANE NARROWS”. Two 24” x 24” red or red/orange flags may be mounted above the W-20 sign. The W-20 sign shall be located on the side of the travelway that has been reduced in width just off the travelway edge of pavement. The W-20 sign shall be a minimum of 500 feet in advance of any channelization devices that encroach on the surface of travelway. A portable changeable message sign may be used in lieu of the W-20 sign.

GENERAL/TIME RESTRICTIONS:

A. STONE BASES, SOIL AGGREGATE BASE AND SOIL BASES
   1. All Highways
      Differences in elevation of more than two inches between surfaces carrying or adjacent to traffic will not be allowed for more than a 24-hour period. A single length of
excavated area that does not exceed 1000 feet in total length may be left open as a start up area for periods not to exceed 48 hours provided the Contractor can demonstrate the ability to continuously excavate and backfill in a proficient manner. Prior approval of the Engineer shall be obtained before any startup area may be allowed.

2. **LIMITED ACCESS HIGHWAY RAMPS (INTERSTATES):**
   On projects that include ramp rehabilitation work, one ramp at a time may be excavated for the entire length of the ramp from the gore point of the ramp with the interstate mainline to the intersection with the crossing highway. This single ramp may remain excavated with a vertical difference in elevation greater than two (2”) inches for a maximum of fourteen (14) calendar days with drums spaced at twenty (20’) feet intervals as shown in Detail 150-B and a buffer space accepted under Section 150.06.F. After fourteen (14) calendar days the section shall be healed as required for all other highways. This area will be allowed in addition to the 1000 feet allowed for all other highways.

B. **ASPHALT BASES, BINDERS AND TOPPINGS**

1. **DIFFERENCES IN ELEVATION BETWEEN THE SURFACES OF ADJACENT TRAVELWAYS**
   Travel lanes shall be paved with a plan that minimizes any difference in elevation between adjacent travel lanes. The following limitations will be required on all work:

   a. Differences of two inches (2”) or less may remain for a maximum period of fourteen (14) calendar days.

   b. Differences of greater than two inches (2”) shall be permitted for continuous operations only.

   **EMERGENCY SITUATIONS:** Inclement weather, traffic accidents, and other events beyond the control of the Contractor may prevent the work from being completed as required above. The Contractor shall notify the Engineer in writing stating the conditions and reasons that have prevented the Contractor from complying with the time limitations. The Contractor shall also outline a plan detailing immediate steps to complete the work. Failure to correct these conditions on the first calendar day that conditions will allow corrective work shall be considered as non-performance of Work under **Subsection 150.08**.

2. **Differences in Elevation Between Asphalt Travelway and Paved Shoulders**
   Differences in elevation between the asphalt travelway and asphalt paved shoulders shall not be allowed to exist beyond the maximum durations outlined below for the conditions shown in Details 150-B, 150-C, 150-D, and 150-E:

   Detail 150-B conditions shall not be allowed for more than 24 hours. A single length that does not exceed 1000 feet in total length may be left open for periods not to exceed 48 hours provided the Contractor can demonstrate the ability to continuously pave in a proficient manner. Prior approval of the Engineer shall be obtained before any section is allowed to exceed 24 hours. Any other disturbed shoulder areas shall be healed as in **Detail 150-E**.

   **Detail 150-C** conditions will not be allowed for more than 48 hours.
Detail 150-D conditions will not be allowed for more than 30 calendar days.

Detail 150-E conditions will not be allowed for more than 60 calendar days.

Failure to meet these requirements shall be considered as non-performance of Work under Subsection 150.08.

C. PORTLAND CEMENT CONCRETE

Work adjacent to a Portland Cement Concrete traveled way which involves the following types of base and shoulders shall be accomplished according to the time restrictions outlined for each type of base or shoulder. Traffic control devices shall be in accordance with Subsection 150.05.

1. Cement Stabilized Base
   Work adjacent to the traveled way shall be healed as per Detail 150-E within forty-eight (48) hours after the seven (7) calendar day curing period is complete for each section placed. During the placement and curing period, traffic control shall be in accordance Detail 150-B.

2. Asphaltic Concrete Base
   When an asphaltic concrete base is utilized in lieu of a cement stabilized base the asphaltic concrete base shall be healed as per Detail 150-E within forty-eight (48) hours after the placement of each section of asphaltic concrete base. For the first forty eight hours traffic control shall be in compliance with Detail 150-B.

3. Concrete Paved Shoulders
   Concrete paved shoulders shall be placed within sixty (60) calendar days after the removal of each section of existing shoulder regardless of the type of base materials being placed on the shoulders. During the placement period, traffic control devices shall be in accordance with the appropriate detail based on the depth of the change in elevation. Differences in elevation of more than two inches between the travel way and the shoulder will not be allowed for more than a 24-hour period. A single length of excavated area that does not exceed 1000 feet in total length may be left open as a start up area for periods not to exceed 48 hours provided the Contractor can demonstrate the ability to continuously excavate and backfill in a proficient manner. Prior approval of the Engineer shall be obtained before any startup area may be allowed. Any other disturbed shoulder areas shall be healed as in Detail 150-E.

4. Asphaltic Concrete Shoulders
   A difference in elevation that meets the requirements of Detail 150-B shall not be allowed to exist for a period greater than forty-eight (48) hours. After the removal of the existing shoulder the section or segment of travelway may be healed with stone as per Detail 150-E for a maximum of fourteen (14) calendar days. Asphaltic concrete shoulders shall be placed within two (2") inches or less of the traveled way surface within fourteen (14) calendar days after the removal of the stone healed section or the removal of each section of the existing shoulder. The two (2") inches or less difference in elevation shall not remain in existence for a period that exceeds thirty (30) calendar days unless the paved shoulder is utilized as a detour for the traveled way. During the placement period, traffic control shall be in accordance with the appropriate detail based on the depth of the change in elevation.
The Contractor may propose an alternate plan based on Subsection 150.06.F. Failure to meet the above requirements and time restrictions shall be considered as non-performance of Work under Subsection 150.08.

D. MISCELLANEOUS ELEVATION DIFFERENTIALS FOR EXCAVATIONS ADJACENT TO THE TRAVELWAY

Drainage structures, utility facilities, or any other work which results in a difference in elevation adjacent to the travelway shall be planned and coordinated to be performed in such a manner to minimize the time traffic is exposed to this condition. The excavation should be back filled to the minimum requirements of Detail 150-E as soon as practical. Stage construction such as plating or backfilling the incomplete work may be required. The difference in elevation shall not be allowed to exist for more than five (5) calendar days under any circumstances. Failure to correct this condition shall be considered as non-performance of Work under Subsection 150.08.

E. CONDUIT INSTALLATION IN PAVED AND DIRT SHOULDERS

The installation of conduit and conduit systems along the shoulders of a traveled way shall be planned and installed in a manner to minimize the length of time that traffic is exposed to a difference in elevation condition. The following restrictions and limitations shall apply:

1. Differences in Elevation of Two (2”) Inches or Less
   The shoulder may remain open when workers are not present. When workers are present the shoulder shall be closed and the channelization devices shall meet the requirements of Subsection 150.05. The difference in elevation on the shoulder shall remain for a maximum period of fourteen (14) calendar days.

2. Differences in Elevation Greater Than Two (2”) Inches
   The shoulder shall be closed. The shoulder closure shall not exceed twenty-four (24) hours in duration unless the Special Conditions in Subsection 150.11 modifies this restriction or the Engineer allows the work to be considered as a continuous operation.

Failure to meet these requirements shall be considered as non-performance of Work under Subsection 150.08.

F. MODIFICATIONS TO TIME RESTRICTIONS

The Contractor may propose any alternate temporary traffic control plan that utilizes a portion of the travel lane as a “buffer space”. This buffer space may allow for an enhanced work area that will allow for the placement of materials to proceed at a pace that could not be achieved with the time restriction requirements outlined in Section 150.06.A, 150.06.B, and 150.06.C. The Contractor may propose modified time restrictions based on the use of the buffer space. Any proposed modifications in the time duration allowed for the differences in elevations to exist shall be reviewed by the Engineer as a component of the overall TTC plan. No modifications shall be made until the proposed plan is accepted by
the Engineer. The Engineer shall have no obligation to consider any proposal which results in an increase in cost to the Department.

For the travel lane described in each of the details 150-B, 150-C, 150-D and 150-E it is presumed that the pavement marking edgeline (yellow or white solid stripe) is located at the very edge of the travel lane surface. A buffer space (temporary paved shoulder) that utilizes a portion of the travel lane should be six (6’) feet in width desirable but shall not be less than four (4’) feet in width. Any remaining travel lane(s) shall not be less than ten (10’) feet in width. Modifications to drum spacing shown in the details above will not be allowed.

If the proposed shifting of the traffic to obtain a buffer space and maintain a minimum travel lane(s) of ten (10’) feet requires the use of any existing paved shoulders then the cost of maintenance and repair of the existing paved shoulder(s) shall be the responsibility of the Contractor. The Contractor is responsible for the costs of maintenance and repairs even if the existing paved shoulder(s) is to be removed in a later stage of the work. Existing shoulders that have rumble strips shall have the rumble strips removed before the shoulder can be utilized as part of the travel lane. The cost of the removal of the rumble strips shall be done at no cost to the Department even if the shoulder is to be removed in a later stage of the work.

Any modifications to the staging and time restrictions that are approved as part of the TTC plan shall be agreed to in writing. Failure to meet these modifications shall be considered as non-performance of the Work under Subsection 150.08.

G. ASPHALTIC CONCRETE RESURFACING PROJECTS

SHOULDER CONSTRUCTION INCLUDED AS A PART OF THE CONTRACT: When the placement of asphaltic concrete materials creates a difference in elevation greater than two (2”) inches between the earth shoulder (grassed or un-grassed) and the edge of travelway or between the earth shoulder and a paved shoulder that is less than four (4’) feet in width, the Contractor shall place and maintain drums in accordance with the requirements of Subsection 150.05A.1.a.4). When the edge of the paved surface is tapered with a 30-45 degree wedge, drums may be spaced at 2.0 times the speed limit in MPH. Drums shall remain in place and be maintained until the difference in elevation has been eliminated by the placement of the appropriate shoulder materials.

SHOULDER CONSTRUCTION NOT INCLUDED AS A PART OF THE CONTRACT: When the placement of asphaltic concrete materials creates a difference in elevation greater than two (2”) inches between the earth shoulder (grassed or un-grassed) and the edge of travelway or between the earth shoulder and a paved shoulder that is less than four (4’) feet in width, the Contractor shall notify the Engineer, in writing, when the resurfacing work including all punchlist items has been completed.

See Subsection 150.03.L for the requirements for “LOW/SOFT SHOULDERS” and “SHOULDER DROP-OFF” signage.
Location of drums when Elevation Difference exceeds 4 inches. Drums spaced at 20 foot intervals. **Note:** If the travel way width is reduced to less than 10 feet by the use of drums, vertical panels shall be used in lieu of drums.

Drums spaced at 40 foot intervals. Location of drums when Elevation Difference is 2+ inches to 4 inches.

**ELEVATION DIFFERENCE GREATER THAN 4 INCHES**

DETAIL 150-B

<table>
<thead>
<tr>
<th>Drums spaced at 40 foot intervals.</th>
<th>Location of drums when Elevation Difference is 2+ inches to 4 inches.</th>
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</thead>
</table>

**ELEVATION DIFFERENCE 2+ to 4 inches**

DETAIL 150-C
Drums spaced at 80 foot intervals.

Location of drums when Elevation Difference is 2 inches or less.

**New Construction**

**Travel Lane**

**ELEVATION DIFFERENCE OF 2 INCHES OR LESS**

**DETAIL 150-D**

Compacted graded aggregate, subbase material or dirt.

**NO STEEPER THAN 4:1**

**HEALED SECTION**

**DETAIL 150-E**
150.07 FLAGGING AND PILOT CARS:

A. FLAGGERS

Flaggers shall be provided as required to handle traffic, as specified in the Plans or Special Provisions, and as required by the Engineer.

B. FLAGGER CERTIFICATION

All flaggers shall meet the requirements of the MUTCD and shall have received training and a certificate upon completion of the training from one of the following organizations:

- National Safety Council
- Southern Safety Services
- Construction Safety Consultants
- Ivey Consultants
- American Traffic Safety Services Association (ATSSA)

Certifications from other agencies will be accepted only if their training program has been approved by any one of the organizations listed above.

Failure to provide certified flaggers as required above shall be reason for the Engineer suspending work involving the flagger(s) until the Contractor provides the certified flagger(s). Flaggers shall have proof of certification and valid identification (photo I.D.) available any time they are performing flagger duties.

C. FLAGGER APPEARANCE AND EQUIPMENT

Flaggers shall wear high-visibility clothing in compliance with Subsection 150.01.A and shall use a Stop/Slow paddle meeting the requirements of the MUTCD for controlling traffic. The Stop/Slow paddles shall have a shaft length of seven (7) feet minimum. The Stop/Slow paddle shall be retro-reflectorized for both day and night usage. In addition to the Stop/Slow paddle, a flagger may use a flag as an additional device to attract attention. This flag shall meet the minimum requirements of the MUTCD. The flag shall, as a minimum, be 24” inches square and red or red/orange in color. For night work, the vest shall have reflectorized stripes which meet the requirements of the MUTCD.

D. FLAGGER WARNING SIGNS

Signs for flagger traffic control shall be placed in advance of the flagging operation in accordance with the MUTCD. In addition to the signs required by the MUTCD, signs at regular intervals, warning of the presence of the flagger shall be placed beyond the point where traffic can reasonably be expected to stop under the most severe conditions for that day’s work.

E. PILOT VEHICLE REQUIREMENTS

Pilot vehicles will be required during placement of bituminous surface treatment or asphaltic concrete on two-lane roadways unless otherwise specified. Pilot vehicles shall meet the requirements of the MUTCD.
F. PORTABLE TEMPORARY TRAFFIC CONTROL SIGNALS

The Contractor may request, in writing, the substitution of portable temporary traffic control signals for flaggers on two-lane two-way roadways provided the temporary signals meet the requirements of the MUTCD, Section 647, and Subsection 150.02.A.8. As a part of this request, the Contractor shall also submit an alternate temporary traffic control plan in the event of a failure of the signals. Any alternate plan that requires the use of flaggers shall include the use of certified flaggers. The Contractor shall obtain the approval of the Engineer before the use of any portable temporary traffic control signals will be permitted.

150.08 ENFORCEMENT

The safe passage of pedestrians and traffic through and around the temporary traffic control zone, while minimizing confusion and disruption to traffic flow, shall have priority over all other Contractor activities. Continued failure of the Contractor to comply with the requirements of Section 150 (TRAFFIC CONTROL) will result in non-refundable deductions of monies from the Contract as shown in this Subsection for non-performance of Work.

Failure of the Contractor to comply with this Specification shall be reason for the Engineer suspending all other work on the Project, except erosion control and traffic control, taking corrective action as specified in Subsection 105.15, and/or withholding payment of monies due to the Contractor for any work on the Project until traffic control deficiencies are corrected. These other actions shall be in addition to the deductions for non-performance of traffic control.

<table>
<thead>
<tr>
<th>SCHEDULE OF DEDUCTIONS FOR EACH CALENDAR DAY OF DEFICIENCIES OF TRAFFIC CONTROL INSTALLATION AND/OR MAINTENANCE</th>
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<tr>
<td><strong>ORIGINAL TOTAL CONTRACT AMOUNT</strong></td>
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150.09 MEASUREMENT

A. TRAFFIC CONTROL

When listed as a pay item in the Proposal, payment will be made at the Lump Sum price bid, which will include all traffic control not paid for separately, and will be paid as follows:

When the first Construction Report is submitted, a payment of 25 (twenty-five) percent of the Lump Sum price will be made. For each progress payment thereafter, the total of the
Project percent complete shown on the last pay statement plus 25 (twenty-five) percent will be paid (less previous payments), not to exceed one hundred (100) percent.

When no payment item for Traffic Control-Lump Sum is shown in the Proposal, all of the requirements of Section 150 and the Temporary Traffic Control Plan shall be in full force and effect. The cost of complying with these requirements will not be paid for separately, but shall be included in the overall bid submittal.

B. SIGNS

When shown as a pay item in the contract, interim special guide signs will be paid for as listed below. All other regulatory, warning, and guide signs, as required by the Contract, will be paid for under Traffic Control Lump Sum or included in the overall bid submitted.

1. Interim ground mounted or interim overhead special guide signs will be measured for payment by the square foot. This payment shall be full compensation for furnishing the signs, including supports as required, erecting, illuminating overhead signs, maintaining, removing, re-erecting, and final removal from the Project. Payment will be made only one time regardless of the number of moves required.

2. Remove and reset existing special guide signs, ground mount or overhead, complete, in place, will be measured for payment per each. Payment will be made only one time regardless of the number of moves required.

3. Modify special guide signs, ground mount or overhead, will be measured for payment by the square foot. The area measured shall include only that portion of the sign modified. Payment shall include materials, removal from posts or supports when necessary, and remounting as required.

C. TEMPORARY BARRIER

Temporary Barrier shall be measured as specified in Sections 620.

D. CHANGEABLE MESSAGE SIGN, PORTABLE

Changeable Message Sign, Portable will be measured as specified in Section 632.

E. TEMPORARY GUARDRAIL ANCHORAGE, Type 12

Temporary Guardrail Anchorage- Type 12 will be measured by each assembly, complete in place and accepted according to the details shown in the plans, which shall also include the additional guardrail and appurtenances necessary for transition and connection to Temporary Concrete Barrier. Payment shall include all necessary materials, equipment, labor, site preparation, maintenance and removal.

F. TRAFFIC SIGNAL INSTALLATION- TEMPORARY

Traffic Signal Installation- Temporary will be measured as specified in Section 647.

G. FLASHING BEACON ASSEMBLY

Flashing Beacon Assemblies will be measured as specified in Section 647.
H. PORTABLE IMPACT ATTENUATORS

Each Portable Impact Attenuator will be measured by the unit/array which shall include all material components, hardware, incidentals, labor, site preparation, and maintenance, including spare parts recommended by the manufacturer for repairing accident damage. Each unit will be measured only once regardless of the number of locations installed, moves required, or number of repairs necessary because of traffic damage. Upon completion of the project, the units shall be removed and retained by the Contractor.

I. PAVEMENT MARKINGS

Pavement markings will be measured as specified in Section 150.

J. TEMPORARY WALKWAYS WITH DETECTABLE EDGING

Temporary walkways with detectable edging will be measured in linear feet (meters), complete in place and accepted, which shall include all necessary materials, equipment, labor, site preparation, temporary pipes, passing spaces, maintenance and removal. Excavation and backfill are not measured separately for payment. No payment will be made for temporary walkways where existing pavements or existing edging (that meets the requirements of MUTCD) are utilized for the temporary walkway. Payment for temporary detectable edging, including approved barriers and channelizing devices, installed on existing pavement shall be included in Traffic Control-Lump Sum.

K. TEMPORARY CURB CUT WHEELCHAIR RAMPS

Temporary curb cut wheelchair ramps are measured as the actual number formed and poured, complete and accepted, which shall include all necessary materials, equipment, labor, site preparation, maintenance and removal. No additional payment will be made for sawing existing sidewalk and removal and disposal of removed material for temporary wheelchair ramp construction. No additional payment will be made for constructing the detectable warning surface.

L. TEMPORARY AUDIBLE INFORMATION DEVICE

Temporary audible information devices are measured as the actual number furnished and installed in accordance with the manufacturer’s recommendations, which shall include all necessary materials, equipment, labor, site preparation, maintenance and removal. Each temporary audible information device will be paid for only one time regardless of the number of times it’s reused during the duration of The Work. These devices shall remain the property of the Contractor.

150.10 PAYMENT:

All items described in the specification shall be paid for under CONSTRUCTION COMPLETE with the exception of Work Zone Law Enforcement. This will be paid for under:

Item No. 150. Traffic Control, Workzone Law Enforcement......... per Hour
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 153 – Field Engineer’s Office

Delete Subsection 153.3.05 B.7 and substitute the following:

7. Worktable: Provide a minimum of three (3) standard dimension desks. They shall be provided with a minimum of 1 1/8” (28mm) wood grain laminated tops with 23” (575mm) deep files and heavy-duty steel ball bearing drawers and locking center drawer. Provide one (1) 5’ X 3’ (1500mmX900mm) adjustable from 0 to 45 degree and 38” (950mm) high drafting table.

Delete Subsection 153.3.05 B.8 and substitute the following:

8. Stools: Provide one (1) posture stool with supportive backrest, waterfall edge seat and instant height lever (26” to 30’’)(650mm to 750mm). Provide a minimum of five (5) fully braced stackable full 2” (50mm) thick 16”X15” (400mmX375mm) seats with armrests and chrome frames. Provide a minimum of four (4) swivel chairs with arms and a 19”X19” (475mmX475mm) foam cushion and upholstered seat adjustable from 16 ½” to 20” (415mm to 500mm) high.

Retain Subsection 153.3.05 B.11 and add the following:

Process and pay the monthly bills for all utility services.

Delete Subsection 153.3.05 B.12 and substitute the following:

12. Electric Service: Provide 120/240 volt electric service that meets code.

Delete Subsection 153.3.05 B.15 and substitute the following:

15. Telephone: Provide in each Type 3 building three telephone lines. Provide two voice lines, with rollover capabilities, connected to two handsets (located on either end of the office). Provide separate telephone line for the computer, as directed by the Engineer. Install and maintain these lines for the life of the Project. Provide
telephone access to the Local Area Telephone Service (LATS) only for outgoing, credit card, collect and toll free calls. Ensure that the telephones can receive incoming non-collect long distance calls.

Provide with the telephone, an automatic answering system that can give a greeting message, record incoming messages, and activate remotely.

Delete Subsection 153.3.05 C.6 and substitute the following:

6. Outside Electrical Receptacle – Provide a weather-proof, exterior 220-volt electrical receptacle attached to a power source.

Delete Subsection 153.3.05 C.7 and substitute the following:

7. Chain Link Fence – Provide a minimum of 500 feet (150m) of 6 ft. (1.8 m) high chain link fence with an extension arm and barbed wire as specified in Section 643. Equip the fence with matching gates and meeting the requirements of Section 643 and consisting of a double 7 ft. (2.1m) by 6 ft. (1.8 m) and a single 4 ft. (1.2 m) by 6 ft. (1.8 m) gate. Include a positive-type locking devices, padlock and a minimum of two keys for each gate. Ensure the fence encompasses the entire compound.

Delete Subsection 153.3.05 C.8 and substitute the following:

8. Security Light – Provide two 150-watt high-pressure sodium security lights with photoelectric controls. Place as directed by the Engineer.

Delete Subsection 153.3.05 C.9 and substitute the following:

9. Copying Machine – The Contractor shall furnish the Field Office with one copying machine installed and maintained for the life of the Project. The copying machine shall have the capability of making letter-size copies (8 ½” x 11”), legal-size copies (8 ½” x 14”), two-sided copies, at least thirty copies per minute, and possess an auto-feed feature. Furnish all consumable and non-consumable supplies for the life of the Project.

Add the following to Subsection 153.3.05 C:

10. Place and spread 200 tons (181 Mg) of aggregate surface course on the Office grounds where indicated by the Engineer to facilitate parking. Remove aggregate and grass the area upon completion of the Project.

11. Ensure that the Office is supported with concrete blocks with mortar joints and anchored with ten storm-tie-down anchors. Enclose the area between the ground and the bottom of the Office with a vinyl skirting that matches the Office’s siding.

12. Install an alarm system that includes the following items and maintain in good operating condition:

- SRN-2000 Enforced Basonic with NAPCO Magnum Alert 850 – control box or Honeywell Vista-10P Master Control Panel with Honeywell 6150RF keypad or equivalent.
- All doors and windows with wired contacts.
- Outside sirens with wired contacts.
- Tamper-proof box with wired contacts.
- Inside sirens with wired contacts.
- Two smoke and heat detectors.

Tie all of the above equipment to a 24 hour control monitoring system (BRK –2812TH or equivalent). Use a wired keyboard system. Do not use a remote system.

Office of Construction

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Process and pay the monthly bills for the alarm system and monitoring.

13. Provide two (2) additional 4 drawer locking fireproof file cabinets.

14. Provide one Desktop Computer and Accessories meeting the following minimum requirements:
   A. Hardware:
      • 1.7 GHz Processor or better
      • 1 GB RAM or larger
      • 80 GB Hard Drive or larger
      • 40X Max, CD-RW
      • 64 MB Video memory
      • V.90 PCI DataFax Modem w/Voice
      • 19” (431 mm) Color Monitor
      • Human Input Device (Mouse)
      • Standard Windows Keyboard
   B. Software:
      • MS Windows XP Professional
      • MS Office XP Professional
      • MS Outlook (Most Recent Version)
      • WinZip
      • WS-FTP
      • A restore CD
   C. Printers:
      HP Officejet 6310 All-in-One Printer, Fax, Scanner, Copier or Dell 968w All-in-One Wireless Printer or approved equal connected so that all functions including fax capability are active. Furnish all consumable and non-consumable supplies for the life of the Project.
   D. Uninterruptible Power Supply:
      • American Power Conversion Corporation Back-UPS ES 650 or Newpoint 750 VA Battery Backup or Equal (minimum 5 Receptacles)
   E. DSL or Cable Broadband Internet Service
      • Provide DSL Internet Service with static IP address or provide Cable Broadband Internet Service as directed by the Engineer. If Cable Broadband Internet Service is used, the third telephone line shall be used for the fax mode of the printer.

16. Concrete Cylinder Curing Box – The Contractor shall furnish a Concrete Curing Box for any project that requires the placement of concrete. The curing box and its components shall be constructed of non-corroding materials and shall be capable of storing a minimum of 22 test cylinders, 6 inch x 12 inch (150 mm x 300 mm) stored vertically with the lid closed. Additional capacity may be required on large projects at the direction of the Engineer. The curing box shall be equipped with heating/cooling capabilities, automatic temperature control, and a maximum/minimum (high/low) temperature readout. The curing box shall be capable of meeting the moisture and temperature requirements of AASHTO T 23.
Add the following to Subsection 153.3.07:
Retain possession of all items that are required as part of the Field Office when the Engineer determines that these items are no longer needed.
Add the following:

**161.1 General Description**

This Work includes using control measures shown on the Plans, ordered by the Engineer, or as required during the life of the Contract to control soil erosion and sedimentation through the use of any of the devices or methods referred to in this Section.

**161.1.01 Definitions**

Certified Personnel— certified personnel are defined as persons who have successfully completed the Level IA certification course approved by the Georgia Soil and Water Conservation Commission. For Department projects the certified person must also have successfully completed the Department’s WECS certification course.

Design Professional as defined in the current GAR100002 NPDES permit.

**161.1.02 Related References**

A. **Standard Specifications**

Section 105—Control of Work
Section 106—Control of Materials
Section 107—Legal Regulations and Responsibility to the Public
Section 109—Measurement and Payment
Section 160—Reclamation of Material Pits and Waste Areas
Section 162—Erosion Control Check Dams
Section 163—Miscellaneous Erosion Control Items
Section 166—Restoration or Alteration of Lakes and Ponds
Section 170—Silt Retention Barrier
Section 171—Temporary Silt Fence
Section 205—Roadway Excavation
Erosion control measures contained in the Specifications include:

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<td>Bituminous Treated Mulch</td>
<td>700.3.05.G</td>
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<tr>
<td>Concrete Paved Ditches</td>
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<td>Silt Filter Bag</td>
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<tr>
<td>Organic &amp; Synthetic Material Fiber Blanket</td>
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**B. Referenced Documents**

Erosion and Sedimentation Pollution Control Plans (ESPCP)
161.1.03 Submittals

A. Status of Erosion Control Devices

The Worksite Erosion Control Supervisor (WECS) or certified personnel will inspect the installation and maintenance of the Erosion Control Devices according to Subsection 167.3.05.B and the ESPCP.

1. Submit all reports to the Engineer within 24 hours of the inspection. Refer to Subsection 167.3.05.C for report requirements.

2. The Engineer will review the reports and inspect the Project for compliance and concurrence with the submitted reports.

3. The Engineer will notify the WECS or certified personnel of any additional items that should be added to the reports.

4. Items listed in the report requiring maintenance or correction shall be completed within 72 hours.

B. Erosion and Sedimentation Pollution Control Plan

1. Project Plans

The Contractor is responsible for preparing an erosion and sedimentation pollution control plan (ESPCP) for the construction of the project. The ESPCP shall be prepared for the various stages of construction necessary to complete the project and shall include the appropriate items and quantities. Submit these plans to the Engineer for review prior to land disturbing activities. These plans are to be prepared by a Design Professional.

If the Contractor alters the stage construction from the approved plans, it will be the responsibility of the Contractor to have the plans revised and prepared in accordance with the current GAR100002 NPDES permit by a Design Professional to reflect all changes in Staging. This will also include any revisions to erosion and sedimentation control item quantities. If the changes affect the Comprehensive Monitoring Program (CMP), the Contractor will be responsible for any revisions to the CMP as well. Submit revised plans and quantities to the Engineer for review prior to land disturbing activities.

2. Haul Roads, Borrow Pits, Excess Material Pits, etc.

The Contractor is responsible for preparing erosion and sedimentation control plans for construction access roads and or haul roads borrow pits, excess material pits, etc (inside the Right of Way). Prepare these plans for all stages of construction and include the appropriate items and quantities. Submit these plans to the Engineer for review prior to land disturbing activities. These plans are to be prepared by a Design Professional.

If construction of access roads, haul roads, borrow pits, excess material pits, etc., (inside the Right of Way) encroach within the 25 foot (7.6 m) buffer along the banks of all state waters or within the 50 ft. (15 m) buffer along the banks of any state waters classified as a “trout stream”, a state water buffer variance must be obtained by the Contractor prior to beginning any land disturbing activity in the stream buffer.

3. Erosion Control for Borrow and Excess Material Pits Outside the Right-of-Way

Erosion control for borrow pits and excess material pits outside the right of way is the responsibility of the Contractor. If borrow or excess material pits require coverage under the National Pollutant Discharge Elimination System permit (NPDES) or other permits or variances are required, submit a copy of all documentation required by the permitting agency to the Engineer. All costs associated with complying with local, state, and federal laws and regulations are the responsibility of the Contractor.

4. Culverts and Pipes

The ESPCP does not contain approved methods to construct a stream diversion or stream diversion channel. The Contractor shall prepare a diversion plan utilizing a Design Professional as defined in the current NPDES permit. See 161.3.05.G for additional information.

5. Temporary Asphalt or Concrete Batch Plants
In addition to the requirements of any applicable specifications, if the Department authorizes the temporary installation and use of any asphalt, concrete or similar batch plants within its right of way, the contractor shall submit an NOI to the Georgia Environmental Protection Division for coverage under the following NPDES permits; The Infrastructure permit for the construction of the plant, and the Industrial permit for the operation of, such a plant. The contractor shall submit the NOIs as both the Owner and the Operator.

161.2 Materials
General Provisions 101 through 150.

161.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

161.3 Construction Requirements

161.3.01 Personnel
A. Duties of the Worksite Erosion Control Supervisor

Before beginning Work, designate a Worksite Erosion Control Supervisor (WECS) to initiate, install, maintain, inspect, and report the condition of all erosion control devices as described in Sections 160 through 171 or in the Contract and ESPCP documents. The designee shall submit their qualifications on the Department provided resume form for consideration and approval. The contractor may utilize additional persons having WECS qualifications to facilitate compliance however, only one WECS shall be designated at a time.

The WECS and alternates shall:

- Be an employee of the Prime Contractor.
- Have at least one year of experience in erosion and sediment control, including the installation, inspection, maintenance and reporting of BMPs.
- Successfully completed the Georgia Soil and Water Conservation Commission Certification Course Level IA and the Department’s WECS Certification Course.
- Provide phone numbers where the WECS can be located 24 hours a day.

The WECS’ duties include the following:

1. Be available or have an approved representative available 24 hours a day and have access to the equipment, personnel, and materials needed to maintain erosion control and flooding control.
2. Inform the Engineer in writing whenever the alternate WECS assumes project responsibilities.
3. Ensure that erosion control deficiencies are corrected within seventy two (72) hours or immediately during emergencies. Deficiencies that interfere with traffic flow, safety or downstream turbidity are to be corrected immediately.
4. During heavy rain, have the construction area patrolled day or night, any day of the week to quickly detect and correct erosion or flooding problems before they interfere with traffic flow, safety, or downstream turbidity.
5. Be on the site within three (3) hours after receiving notification of an emergency prepared to positively respond to the conditions encountered. The Department may handle emergencies without notifying the Contractor. The Department will recover costs for emergency
Failure of the WECS or alternate to perform the duties specified in the Contract, or whose performance, has resulted in a citation being received from a State or Federal Regulatory Agency, e.g. the Georgia Environmental Protection Division, shall result in one or more of the following:

- Suspension of the WECS’ certification for a period of not less than 30 days
- Removal of the Contractor’s project superintendent in accordance with Sections 105.05 and 108.05 for a period not less than 14 days
- Department wide revocation of the WECS certification for a period of 12 months
- Removal of the Contractor’s project superintendent in accordance with Sections 105.05 and 108.05

161.3.02 Equipment
General Provisions 101 through 150.

161.3.03 Preparation
General Provisions 101 through 150.

161.3.04 Fabrication
General Provisions 101 through 150.

161.3.05 Construction
Coordinate the temporary and permanent erosion control provisions in this Specification with the permanent erosion control provisions in the Contract to ensure economical, effective, and continuous erosion control throughout the construction and post-construction periods.

At all times that land disturbing activity is underway, a person meeting the requirements of, “certified person” by the GSWCC (Level IA) must be on the project.

A. Control Dust Pollution
   The contractor shall keep dust pollution to a minimum during any of the activities performed on the project. It may be necessary to apply water or other BMPs to roadways or other areas reduce pollution.

B. Perform Permanent or Temporary Grassing
   Perform permanent grassing, temporary grassing, or mulching on cut and fill slopes weekly (unless a shorter period is required by Subsection 107.23) during grading operations. When conditions warrant, the Engineer may require more frequent intervals.

   Under no circumstances shall the grading (height of cut) exceed the height operating range of the grassing equipment. It is extremely important to obtain a cover, whether it is mulch, temporary grass or permanent grass. Adequate mulch is a must.
When grading operations or other soil disturbing activities have stopped, perform grassing or erosion control as shown in the Plans, as shown in an approved Plan submitted by the Contractor, or as directed by the Engineer.

C. Seed and Mulch

Refer to Subsection 161.3.05.B, “Perform Permanent or Temporary Grassing”.

D. Implement Permanent or Temporary Erosion Control

1. Silt fence shown along the perimeter, e.g. right of way, and sediment containment devices, e.g. sediment basins, shall be installed prior to or concurrently with clearing and grubbing operations.

2. Incorporate permanent erosion control features into the Project at the earliest practicable time, e.g. velocity dissipation, permanent ditch protection.

3. Use temporary erosion control measures to address conditions that develop during construction but were unforeseen during the design stage.

4. Use temporary erosion control measures when installation of permanent erosion control features cannot be accomplished.

The Engineer has the authority to:

- Limit the surface area of erodible earth material exposed by clearing and grubbing.
- Limit the surface area of erodible earth material exposed by excavation and borrow and fill operations.
- Limit the area of excavation, and embankment operations in progress to correspond with the Contractor’s ability to keep the finish grading, mulching, seeding, and other permanent erosion control measures current.
- Direct the Contractor to provide immediate permanent or temporary erosion control to prevent contamination of adjacent streams or water courses, lakes, ponds, or other areas of water impoundment.

Such Work may include constructing items listed in the table in Subsection 161.1.02.A, “Related References” or other control devices or methods to control erosion.

E. Erodible Area

**NOTE: Never allow the surface area of erodible earth material exposed at one time to exceed 17 acres (7 ha) except as approved by the State Construction Engineer.**

The maximum of 17 acres (7 ha) of exposed erodible earth applies to the entire Project and to all of its combined operations as a whole, not to the exposed erodible earth of each individual operation.

Upon receipt of a written request from the contractor, the State Construction Engineer, or his designee, will review; the request, any justifications and the Project conditions for waiver of the 17 acres (7 ha) limitation.

If the 17 acre limitation is increased by the State Construction Engineer, the WECS shall not be assigned to another project in that capacity and should remain on site each work day that the exposed acreage exceeds 17 acres.

After installing temporary erosion control devices, e.g., grassing, mulching, stabilizing an area, and having it approved by the Engineer, that area will be released from the 17 acres (7 ha) limit.
F. Perform Grading Operations

Perform the following grading operations:

1. Complete each roadway cut and embankment continuously, unless otherwise specified in the Contract or ordered by the Engineer.
2. Maintain the top of the earthwork in roadway sections throughout the construction stages to allow water to run off to the outer edges.
3. Provide temporary slope drain facilities with inlets and velocity dissipaters (straw bales, silt fence, aprons, etc.) to carry the runoff water to the bottom of the slopes. Place drains at intervals to handle the accumulated water.
4. Continue temporary erosion control measures until permanent drainage facilities have been constructed, pavement placed, and the grass on planted slopes stabilized to deter erosion.

G. Perform Construction in Rivers and Streams

Perform construction in river and stream beds as follows:

1. Unless otherwise agreed to in writing by the Engineer, restrict construction operations in rivers, streams, and impoundments to:
   - Areas where channel changes or access for construction are shown on the Plans to construct temporary or permanent structures.
2. If channel changes or diversions are not shown on the Plans, the Contractor shall develop diversion plans prepared in accordance with the current GAR100002 NPDES Infrastructure Construction permit utilizing a design professional as defined within the permit. The Engineer will review prepared diversion plans for content only and accepts no responsibility for design errors or omissions. Amendments will be made part of the project plans by attachment. Include any associated costs in the price bid for the overall contract. Any contract time associated with the submittal or its review and subsequent response will not be considered for an extension of Contract time. All time associated with this subsection shall be considered incidental.
3. If additional access for construction or removal of work bridges, temporary roads/access or work platforms is necessary, and will require additional encroachment upon river or stream banks and bottoms, the contractor shall prepare a plan in accordance with the current GAR100002 NPDES Infrastructure Construction permit utilizing a design professional as defined within the permit. Plans should be submitted at least 12 weeks prior to the date the associated work is expected to begin. If necessary, the plan will be provided to the appropriate regulating authority, e.g. United States Army Corps of Engineers by the Department for consideration and approval. No work that impacts areas beyond what has been shown in the approved plans will be allowed to begin until written approval of the submitted plan has been provided by the Department. Approved plan amendments will be made part of the project plans by attachment. Include any associated costs in the price bid for the overall contract. Any contract time associated with the submittal or its review and subsequent response will not be considered for an extension of Contract time. All time associated with this subsection shall be considered incidental.
4. Clear rivers, streams, and impoundments of the following as soon as conditions permit:
• Falsework
• Piling that is to be removed
• Debris
• Other obstructions placed or caused by construction operations

5. Do not ford live streams with construction equipment.
6. Use temporary bridges or other structures that are adequate for a 25-year storm for stream crossings. Include costs in the price bid for the overall contract.
7. Do not operate mechanized equipment in live streams except to construct channel changes or temporary or permanent structures, and to remove temporary structures, unless otherwise approved in writing by the Engineer.

H. State Water Buffers and Environmental Restrictions

1. The WECS shall review the plans and contract documents for environmental restrictions, Environmentally Sensitive Areas (ESA), e.g. buffers, etc prior to performing land disturbing activities.
2. The WECS shall ensure all parties performing land disturbing activities within the project limits are aware of all environmental restrictions.
3. Buffer delineation shall be performed prior to clearing, or any other land disturbing activities. Site conditions may require temporary delineation measures are implemented prior to the installation of orange barrier/safety fencing. The means of temporary delineation shall have the Engineer’s prior approval.
4. The WECS shall allow the Engineer to review the buffer delineation prior to performing any land disturbing activities, including but not limited to clearing, grubbing and thinning of vegetation. Any removal and relocation of buffer delineation based upon the Engineer’s review will not be measured for separate payment.
5. The WECS shall advise the Engineer of any surface water(s) encountered that are not shown in the plans. The WECS shall prevent land disturbing activities from occurring within surface water buffers until the Engineer provides approval to proceed.

I. General Requirements

Projects that consist of asphalt resurfacing, shoulder reconstruction and/or shoulder widening; schedule and perform the construction of the project to comply with the following:

After temporary and permanent erosion control devices are installed and the area permanently stabilized (temporary or permanent) and approved by the Engineer, the area may be released from the 1 acre (0.4 ha) limit.

The maximum of 1 acre (0.4 ha) of erodible earth applies to the entire project and to all combined operations, including borrow and excess material operations that are within the right of way, not 1 acre (0.4 ha) of exposed erodible earth for each operation.

NOTE: Never allow the surface area of erodible earth material exposed at one time to exceed 1 acre (0.4 ha).

1. Do not allow the disturbed exposed erodible area to exceed 1 acres (0.4 ha). This 1 acre (0.4 ha) limit includes all disturbed areas relating to the construction of the project including but not limited to slope and shoulder construction.
2. At the end of each working day, permanently stabilize all of the area disturbed by slope and shoulder reconstruction to prevent any contamination of adjacent streams or other watercourses, lakes, ponds or other areas of water impoundment. For purposes of this Specification, the end of the working day is defined as when the construction operations cease. For example, 6:00 a.m. is the end of the working day on a project that allows work only between 9:00 p.m. and 6:00 a.m.)

3. Stabilize the cut and fill slopes and shoulder with permanent or temporary grassing and a Wood Fiber Blanket (Section 713, Type II). Mulching is not allowed. Borrow pits, soil disposal sites and haul roads will not require daily applications of wood fiber blanket. The application rate for the Wood Fiber Blanket on shoulder reconstruction is the rate specified for Shoulders. For shoulder reconstruction, the ground preparation requirements of Subsection 700.3.05.A.1 are waived. Preparation consists of scarifying the existing shoulders 4 to 6 in (100 to 150 mm) deep and leaving the area in a smooth uniform condition free from stones, lumps, roots or other material.

4. If a sudden rain event occurs that would not allow the Contractor to apply the Type II Wood Fiber Blanket per Section 713, install Wood Fiber Blanket Type I per Section 713 if directed by the Engineer. Wood Fiber Blanket Type I application is for emergency use only.

5. Place asphalt in the trench the same day as the excavation occurs. Place asphalt or concrete in driveways and side roads being re-graded the same day as the excavation occurs. Stabilize any disturbed or exposed soil that is not covered with asphalt with a Wood Fiber Blanket (and grass seed). Payment will be made for the Wood Fiber Blanket and grass seed only if the shoulder has been constructed to final dimensions and grade and no further grading will be required.

6. Do not allow the grading (height of cut or fill) to exceed the operating range of the grassing equipment.

7. When grading operations or other soil disturbing activities are suspended, regardless of the reason, promptly perform all necessary permanent stabilization and/or erosion control work.

8. Use temporary erosion control measures to:
   - To correct conditions that develop during construction but were unforeseen during the design stage.
   - To use as needed before installing permanent erosion control features.
   - To temporarily control erosion that develops during normal construction practices but are not associated with permanent control features on the Project.

9. When conditions warrant, such as unfavorable weather (rain event), the Engineer may require more frequent intervals for this work.

161.3.06 Quality Acceptance

Before Final Acceptance of the Work, clean drainage structures within the project limits, both existing and newly constructed, and ensure that they are functioning properly. Costs to accomplish this work are incidental and shall be included in the overall bid for the Contract.

161.3.07 Contractor Warranty and Maintenance

Maintain the erosion control features installed to:

- Contain erosion within the limits of the right-of-way
• Control storm water discharges from disturbed areas

Effectively install and maintain the erosion control features. Ensure these features contain the erosion and sediment within the limits of the rights of way and control the discharges of storm-water from disturbed areas to meet all local, state, and federal requirements on water quality.

If a construction Project has separate contractors, the Prime Contractor shall maintain the erosion control features at grading sites as acceptable to the Engineer until the Contract is accepted. If any erosion control devices are damaged by any contractor either by neglect, by construction methods, or any other reasons, including acts of nature, they shall be repaired within 24 hours by the Prime Contractor at no cost to the Department.

161.4 Measurement
Control of soil erosion and sedimentation is not measured separately for payment.

161.4.01 Limits
General Provisions 101 through 150.

161.5 Payment
The requirements of this Specification and the Erosion Control Plan shall be in full effect. The cost of complying with these requirements will not be paid for separately, but shall be included in the overall bid submitted for CONSTRUCTION COMPLETE.

No payment will be made for erosion control outside the Right-of-Way or construction easements except as provided for by the Plans.

161.5.01 Enforcement and Adjustments
A. Failure to Provide a WECS
If a designated WECS is not maintained or if the Contractor does not comply with this Specification, cease activities except traffic control and erosion control work. Monies that are due or that may become due also may be withheld according to the Specifications

B. Failure to submit reports
A non-refundable deduction will be taken from the schedule below whenever the WECS fails to submit completed reports required by Subsection 167.3.05.C in accordance with the provisions of this specification.

C. Failure to Comply with Specifications
If the Contractor fails to comply with any of the requirements of this Specification, all activities shall cease immediately except traffic control and erosion control related work.

Monies that are currently due or that may become due shall be withheld according to the specifications. In addition, nonrefundable monies shall be deducted from the contract as shown in the Schedule of Deductions table below. These deductions are in addition to any actions taken in the above subsections. Deductions assessed for uncorrected deficiencies shall continue until all corrections are completed to the satisfaction of the Engineer.

D. Receipt of a Consent Order or Notice of Violation, etc

Regulatory enforcement actions will be resolved including at a minimum the following steps;

• The Department will perform an internal review of the alleged violations
• The Department will then meet with the Contractor to review and further determine responsibilities for the alleged violations
• The Department will then arrange to collectively meet with the regulatory agencies to negotiate resolutions and/or settlements.

The Department does not waive any rights of the Contractor to resolve such matters however, in the event that regulatory agency communication is addressed jointly to the Department and to the contractor, the Department reserves the right to coordinate all communications, e.g., written correspondence, and to schedule jointly attended meetings with Regulatory agencies such that timely and accurate responses are known to the Department.

Such Orders or Notices may result in the assessment of Deductions from the table below for each day the condition remains non-compliant following an agreed remedy.

Monetary penalties for which the contractor is obligated for as a result of regulatory enforcement may be withheld from future monies due the contractor.

<table>
<thead>
<tr>
<th>Schedule of Deductions for Each Calendar Day of Erosion Control Deficiencies</th>
</tr>
</thead>
<tbody>
<tr>
<td>Initial Occurrence*</td>
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<tr>
<td>From More Than</td>
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<tr>
<td>0</td>
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<tr>
<td>$100,000</td>
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<tr>
<td>$1,000,000</td>
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<tr>
<td>$5,000,000</td>
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<tr>
<td>$15,000,000</td>
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</tbody>
</table>

*Continued non-compliance with the requirements of this specification may result in the doubling of the above tabulated Daily Charge.

Upon written request from the Contractor, the Engineer may allow, limited activities to concurrently proceed once significant portions of the corrective work have been completed. This authorization may be similarly rescinded if in the opinion of the Engineer corrective work is not being diligently pursued.
163.1 General Description
This work includes constructing and removing:

- Silt control gates
- Temporary erosion control slope drains shown on the Plans or as directed
- Sediment basins
- Baled straw sediment barrier and check dams
- Other temporary erosion control structures shown on the Plans or directed by the Engineer

This work also includes applying mulch (straw or hay, erosion control compost), and temporary grass.

163.1.01 Related References
A. Standard Specifications

  - Section 109—Measurement and Payment
  - Section 161—Control of Soil Erosion and Sedimentation
  - Section 171—Temporary Silt Fence
  - Section 500—Concrete Structures
  - Section 603—Rip Rap
  - Section 700—Grassing
  - Section 715—Bituminous Treated Roving
  - Section 720—Triangular Silt Barrier
  - Section 822—Emulsified Asphalt
  - Section 860—Lumber and Timber
  - Section 863—Preservative Treatment of Timber Products
  - Section 890—Seed and Sod
163.1.02 Submittals
Provide written documentation to the Engineer as to the average weight of the bales of mulch.

163.2 Materials
Provide materials shown on the Plans, such as pipe, spillways, wood baffles, and other accessories including an anti-seep collar, when necessary. The materials shall remain the Contractor’s property after removal, unless otherwise shown on the Plans.

Materials may be new or used; however, the Engineer shall approve previously used materials before use.

Materials shall meet the requirements of the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Mulch</td>
<td>893.2.02</td>
</tr>
<tr>
<td>Temporary Silt Fence</td>
<td>171</td>
</tr>
<tr>
<td>Concrete Aprons and Footings shall be Class A</td>
<td>500</td>
</tr>
<tr>
<td>Rip Rap</td>
<td>603</td>
</tr>
<tr>
<td>Temporary Grass</td>
<td>700</td>
</tr>
<tr>
<td>Bituminous Treated Roving</td>
<td>715</td>
</tr>
<tr>
<td>Triangular Silt Barrier</td>
<td>720</td>
</tr>
<tr>
<td>Lumber and Timber</td>
<td>860.2.01</td>
</tr>
<tr>
<td>Preservative Treatment of Timber Products</td>
<td>863.1</td>
</tr>
<tr>
<td>Corrugated Polyethylene Temporary Slope Drain Pipe</td>
<td>AASHTO M252 or M294</td>
</tr>
</tbody>
</table>

163.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

163.3 Construction Requirements

163.3.01 Personnel
General Provisions 101 through 150.

163.3.02 Equipment
General Provisions 101 through 150.

163.3.03 Preparation
General Provisions 101 through 150.

163.3.04 Fabrication
General Provisions 101 through 150.
163.3.05 Construction

A. Silt Control Gates

If silt control gates are required or are directed by the Engineer, follow these guidelines to construct them:

1. Clear and grade only that portion of the roadway within the affected drainage area where the drainage structure will be constructed.
2. Construct or install the drainage structure and backfill as required for stability.
3. Install the silt control gate at the inlet of the structure. Use the type indicated on the Plans.
4. Vary the height of the gate as required or as shown on the Plans.
5. Finish grading the roadway in the affected drainage area. Grass and mulch slopes and ditches that will not be paved. Construct the ditch paving required in the affected area.
6. Keep the gate in place until the work in the affected drainage area is complete and the erodible area is stabilized.
7. Remove the Type 1 silt gate assembly by sawing off the wood posts flush with the concrete apron. Leave the concrete apron between the gate and the structure inlet in place. The gate shall remain the property of the Contractor.

B. Temporary Slope Drains

If temporary slope drains are required, conduct the roadway grading operation according to Section 161 and follow these guidelines:

1. Place temporary pipe slope drains with inlets and velocity dissipaters (straw bales, silt fence, or aprons) according to the Plans.
2. Securely anchor the inlet into the slope to provide a watertight connection to the earth berm. Ensure that all connections in the pipe are leak proof.
3. Place temporary slope drains at a spacing of 350 ft (105 m) maximum on a 0% to 2% grade and at a spacing of 200 ft (60m) maximum on steeper grades, or more frequently as directed by the Engineer. Keep the slope drains in place until the permanent grass has grown enough to control erosion.
4. Remove the slope drains and grass the disturbed area with permanent grass. However, the temporary slope drains may remain in place to help establish permanent grass if approved by the Engineer.

C. Sediment Basins

Construct sediment basins according to the Plans at the required location, or as modified by the Engineer.

1. Construct the unit complete as shown, including:
   - Grading
   - Drainage
   - Rip rap
   - Spillways
   - Anti-seep collar
   - Temporary mulching and grassing on internal and external slopes
   - Accessories to complete the basin
2. When the sediment basin is no longer needed, remove and dispose of the remaining sediment.
3. Remove the sediment basin. Grade to drain and restore the area to blend with the adjacent landscape.
4. Mulch and permanently grass the disturbed areas according to Section 700.
D. Sediment Barrier (baled straw)

Construct sediment barrier (baled straw) according to the Plan details. Use rectangular, standard size baled straw in mechanically produced bales.

The following items may be substituted for sediment barrier (baled straw)

1. Type B Silt Fence.
2. Triangular Silt Barrier.

3. Synthetic Fiber: Use synthetic fiber bales of circular cross section at least 18 in (450 mm) in diameter. Use synthetic bales of 3 ft or 6 ft (0.9 m or 1.8 m) in length that are capable of being linked together to form a continuous roll of the desired total length. Use bales that are enclosed in a geotextile fabric and that contain a pre-made stake hole for anchoring.

4. Coir: Use coir fiber bales of circular cross section at least 16” (400 mm) in diameter. Use coir bales of 10 ft, 15 ft, or 20 ft (3 m, 4.5 m, or 6 m) in length. Use coir baled with coir twine netting with 2 in X 2 in (50 mm X 50 mm) openings. Use coir bales with a dry density of at least 7 lb/ft³ (112 kg/m³). Anchor in place with 2 in X 4 in (50 mm X 100 mm) wooden wedges with a 6 in (150 mm) nail at the top. Place wedges no more than 36 in (900 mm) apart.

5. Excelsior: Use curled aspen excelsior fiber with barbed edges in circular bales of at least 18 in (450 mm) in diameter and nominally 10 ft (3 m) in length. Use excelsior baled with polyester netting with 1 in X 1 in (25 mm by 25 mm) triangular openings. Use excelsior bales with a dry density of at least 1.4 lb/ft³ (22 kg/m³). Anchor in place with 1 in (25 mm) diameter wooden stakes driven through the netting at intervals of no more than 2 ft (600 mm).

6. Compost Filter Sock: Use general use compost (see Subsection 893.2.02.A.5.b) in circular bales at least 18 in in diameter. Use compost baled with photo-degradable plastic mesh 3 mils thick with a maximum 0.25 in X 0.25 in (6 mm X 6 mm) openings. Anchor in place with 1 in (25 mm) diameter wooden stakes driven through the netting at intervals of no more than 2 ft (600 mm). The sock shall be dispersed on site when no longer required, as determined by the Engineer. Do not use Compost Filter Socks in areas where the use of fertilizer is restricted.

7. Compost Filter Berm: Use erosion control compost (see Subsection 893.2.02) to construct an uncompacted 1.5 ft to 2 ft (450 mm to 600 mm) high trapezoidal berm which is approximately 2 ft to 3 ft (600 mm to 1 m) wide at the top and minimum 4 ft (1.2 m) wide at the base. Do not use Compost Filter Berms in areas where the use of fertilizer is restricted.

The construction of the compost filter berm includes the following:

a. Keeping the berm in a functional condition.

b. Installing additional berm material when necessary.

c. Removing the berm when no longer required, as determined by the Engineer. At the Engineer’s discretion, berm material may be left to decompose naturally, or distributed over the adjacent area.

E. Other Temporary Structures

When special conditions occur during the design stage, the Plans may show other temporary structures for erosion control with required materials and construction methods.

F. Temporary Grass

Use a quick growing species of temporary grass such as rye grass, millet, or a cereal grass suitable to the area and season.

Use temporary grass in the following situations:
• When required by the Specifications or directed by the Engineer to control erosion where permanent grassing cannot be planted.
• To protect an area for longer than mulch is expected to last (60 calendar days).

Plant temporary grass as follows:

1. Use seeds that conform to Subsection 890.2.01, “Seed.” Perform seeding according to Section 700, except use the minimum ground preparation necessary to provide a seed bed if further grading is required.
2. Prepare areas that require no further grading according to Subsection 700.3.05.A, “Ground Preparation.” Omit the lime unless the area will be planted with permanent grass without further grading. In this case, apply the lime according to Section 700.
3. Apply mixed grade fertilizer at 400 lbs/acre (450 kg/ha). Omit the nitrogen. Mulch (with straw or hay) temporary grass according to Section 700. (Erosion control compost Mulch will not be allowed with grassing.)
4. Before planting permanent grass, thoroughly plow and prepare areas where temporary grass has been planted according to Subsection 700.3.05.A, “Ground Preparation”.
5. Apply Polyacrylamide (PAM) to all areas that receive temporary grassing.
6. Apply Pam (powder) before grassing or PAM (emulsion) to the hydroseeding operation.
7. Apply PAM according to manufacturer specifications.
8. Use only anionic PAM.

For projects that consist of shoulder reconstruction and/or shoulder widening refer to Section 161.3.05H for Wood Fiber Blanket requirements.

G. Mulch

When stage construction or other conditions prevent completing a roadway section continuously, apply mulch (straw or hay or erosion control compost) to control erosion. Mulch may be used without temporary grassing for 60 calendar days or less. Areas stabilized with only mulch (straw/hay/compost) shall be planted with temporary grass after 60 calendar days.

Apply mulch as follows:

1. Mulch (Hay or Straw)
   a. Uniformly spread the mulch over the designated areas from 2 in to 4 in (50 mm to 100 mm) thick.
   b. After spreading the mulch, walk in the mulch by using a tracked vehicle (preferred method), empty sheep foot roller, light discing, or other means that preserves the finished cross section of the prepared areas. The Engineer will approve of the method.
   c. Place temporary mulch on slopes as steep as 2:1 by using a tracked vehicle to imbed the mulch into the slope. Where specified, use bituminous treated mulch (straw or hay) according to Subsection 700.3.05.G.1, “Mulch with Binder”.
   d. When grassing operations begin, leave the mulch inplace and plow the mulch into the soil during seed bed preparation. The mulch will become beneficial plant food for the newly planted grass.
2. Apply mulch (erosion control compost) as follows:
   a. Uniformly spread the mulch (erosion control compost) over the designated areas 2 in (50 mm) thick.
   b. When rolling is necessary, or directed by the Engineer, use a light corrugated drum roller.
   c. When grassing operations begin, leave the mulch in place and plow the mulch into the soil during seed bed preparation. The mulch will become beneficial plant food for the newly planted grass.
d. Plant temporary grass on area stabilized with mulch (erosion control compost) after 60 calendar days.
e. Do not use Erosion Control Compost in areas where the use of fertilizer is restricted.

H. Miscellaneous Erosion Control Not Shown on the Plans

When conditions develop during construction that were unforeseen in the design stage, the Engineer may direct the Contractor to construct temporary devices such as but not limited to:

- Bulkheads
- Sump holes
- Half round pipe for use as ditch liners
- U-V resistant plastic sheets to cover critical cut slopes

The Engineer and the Contractor will determine the placement to ensure erosion control in the affected area.

I. Diversion Channels

When constructing a culvert or other drainage structure in a live stream that requires diverting a stream, construct a diversion channel.

J. Temporary Check Dams

Temporary check dams are constructed of the following materials:

- Stone plain rip rap according to Section 603 or of sand bags as in Section 603 without Portland cement. (Place plastic filter fabric on ditch section before placing rip rap.)
- Fabric (Type C silt fence)
- Hay Bales

Temporary check dams shall be constructed according to plan details and shall remain in place until the permanent ditch protection is in place or being installed and the removal is approved by the Engineer.

K. Construction Exits

Locate construction exits at any point where vehicles will be leaving the project onto a public roadway. Install construction exits at the locations shown in the plans and in accordance with plan details.

L. Retrofit

Add the retrofit device to the permanent outlet structure as shown on the Plan details.

When all land disturbing activities that would contribute sediment-laden runoff to the basin are complete, clean the basin of sediment and stabilize the basin area with vegetation.

When the basin is stabilized, remove the retrofit device from the permanent outlet structure of the detention pond.

M. Inlet Sediment Trap

Inlet sediment traps consist of a temporary device placed around a storm drain inlet to trap sediment. An excavated area adjacent to the sediment trap will provide additional sediment storage.

Inlet sediment traps may be constructed of Type C silt fence, plastic frame and filter, hay bales, baffle box, or other filtering materials approved by the Engineer.

Construct inlet sediment traps according to the appropriate specification for the material selected for the trap.
Place inlet sediment traps as shown on the Plans or as directed by the Engineer.

**163.3.06 Quality Acceptance**  
General Provisions 101 through 150.

**163.3.07 Contractor Warranty and Maintenance**  
General Provisions 101 through 150.

### 163.4 Measurement

A. **Silt Control Gates**  
No separate measurement will be made.

B. **Temporary Slope Drains**  
No separate measurement will be made. When required, the inlet spillway and outlet apron and/or other dissipation devices are incidental and not measured separately.

C. **Sediment Basins**  
No separate measurement will be made for sediment basins which includes but is not limited to, construction, maintenance, and removal, as well as earthwork, drainage, spillways, baffles, rip rap, final cleaning to remove the basin. Permanent and temporary grassing for sediment basins is not measured separately for payment.

D. **Diversion Channels**  
Diversion channels are not measured for payment. Costs for the entire structure complete, including materials, construction (including earthwork), and removal is included in the price bid for the CONSTRUCTION COMPLETE.

E. **Temporary Grass**  
No separate measurement will be made. Temporary grass includes mulch and fertilizer.

F. **Mulch**  
No separate measurement will be made.

G. **Baled Straw Sediment Barrier, Baled Straw Check Dam and Fabric Check Dams**  
No separate measurement will be made for baled straw sediment barrier, baled straw check dams, and fabric check dams. When the Contractor substitutes a product allowed in Subsection 163.3.05.D for baled straw sediment barrier or when the Engineer directs this substitution, no separate measure will be made for the product.

H. **Rip Rap Check Dams**  
No separate measurement will be made.

I. **Construction Exits**  
No separate measurement will be made.

J. **Retrofit**  
No separate measurement will be made.

K. **Inlet Sediment Trap**  
No separate measurement will be made.
163.4.01 Limits
General Provisions 101 through 150.

163.5 Payment
A. Silt Control Gates
   To be paid for under CONSTRUCTION COMPLETE. Includes:
   • Furnishing the material and labor
   • Constructing the concrete apron as shown on the Plans
   • Excavating and backfilling to place the apron
   • Removing the gate

B. Temporary Slope Drains
   To be paid for under CONSTRUCTION COMPLETE. Includes materials, construction, removal (if required), inlet spillways, velocity dissipaters, and outlet aprons.
   When temporary drain inlets and pipe slope drains are removed, they remain the Contractor’s property and may be reused or removed from the Project as the Contractor desires. Reused pipe or inlets are paid for the same as new pipe or inlets.

C. Sediment Basin
   To be paid for under CONSTRUCTION COMPLETE. Includes work and supervision to construct, and remove the sediment basin, including final clean-up.

D. Diversion Channel
   Diversion channels are not paid for separately; they are included in the price bid CONSTRUCTION COMPLETE.

E. Temporary Grass
   To be paid for under CONSTRUCTION COMPLETE. Includes all equipment, labor, ground preparation, materials, wood fiber mulch, polyacrylamide, and other incidentals. Lime (when required) is paid for by the ton (megagram). Mulch and fertilizer are paid for separately.

F. Mulch
   To be paid for under CONSTRUCTION COMPLETE. Includes all materials, labor, maintenance, equipment and other incidentals.

G. Baled Straw Sediment barrier, Baled Straw Check Dams and Fabric Check Dams (Type C Silt Fence)
   To be paid for under CONSTRUCTION COMPLETE. Includes constructing, and removing (when directed) the baled straw sediment barrier or either check dam.
   When the Contractor substitutes any product allowed in Subsection 163.3.05.D for baled straw sediment barrier or when the Engineer directs this substitution, payment is also made at the bid price CONSTRUCTION COMPLETE.

H. Rip Rap Check Dams
   To be paid for under CONSTRUCTION COMPLETE. Includes all materials, construction, and removal. Reused stone plain rip rap or sandbags are paid for on the same basis as new items. Filter fabric required under rip rap check dams is included in the price bid for each check dam.
I. **Construction Exits**
To be paid for under CONSTRUCTION COMPLETE. Includes all materials including the required geotextile, construction, and removal.

J. **Retrofit**
To be paid for under CONSTRUCTION COMPLETE. Includes all work, supervision, materials (including the stone filter), labor and equipment necessary to construct and remove the retrofit device from an existing or proposed detention pond outlet structure.

K. **Inlet Sediment Trap**
To be paid for under CONSTRUCTION COMPLETE. Includes all materials, construction, and removal

**163.5.01 Adjustments**
General Provisions 101 through 150.
Section 165—Maintenance of Temporary Erosion and Sedimentation Control Devices

165.1 General Description
This work consists of providing maintenance on temporary erosion and sediment control devices, including but not limited to the following:

- Silt fence
- Sediment basins
- Silt control gates
- Check dams
- Silt retention barriers

It also consists of removing sediment that has accumulated at the temporary erosion and sediment control devices.

165.1.01 Definitions
General Provisions 101 through 150.

165.1.02 Related References
A. Standard Specifications
   General Provisions 101 through 150.

B. Referenced Documents
   General Provisions 101 through 150.

165.1.03 Submittals
General Provisions 101 through 150.

165.2 Materials
General Provisions 101 through 150.

165.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.
165.3 Construction Requirements

165.3.01 Personnel
General Provisions 101 through 150.

165.3.02 Equipment
General Provisions 101 through 150.

165.3.03 Preparation
General Provisions 101 through 150.

165.3.04 Fabrication
General Provisions 101 through 150.

165.3.05 Construction

A. General

As a minimum, clean the sediment from all temporary erosion control devices (except sediment basins) installed on the project when one half the capacity, by height, depth or volume has been reached. Clean the sediment from all temporary sediment basins installed on a project when one third the capacity of the storage volume has been filled.

Handle sediment excavated from any erosion or sediment control device in one of the following ways:

• Remove sediment from the immediate area and immediately stabilize it to prevent the material from refilling any erosion or sediment control device.
• Place and mix it in the roadway embankment, or waste it in an area approved by the Engineer.
• Repair or replace at no cost to the Department, any erosion or sediment control devices that are not functioning properly or are damaged due to negligence or abuse.

B. Temporary Silt Fence

Maintenance of Temporary Silt Fence consists of furnishing all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original ground line (0% filled). Also included is the removal of sediment accumulations (“filtercake”) on the fabric by tapping the fabric on the downstream side.

C. Silt Control Gates

Maintenance of Temporary Silt Control Gates consists of all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original ground line (0% filled). When applicable, this item will include the removal of sediment accumulations on the fabric by tapping the fabric on the downstream side.

D. Check Dams (all types)

Maintenance of Temporary Erosion Control Check Dams shall consist of all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original ground line (0% filled). This item also includes the removal of any material deposited in sump holes. When applicable, this item will include the removal of sediment accumulations on the fabric by tapping the fabric on the downstream side, or from the baled straw by similar means.

E. Silt Retention Barrier

Maintenance of Temporary Silt Retention Barrier consists of all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original ground line (0% filled).
F. Temporary Sediment Basins

Maintenance of Temporary Sediment Basins consists of all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original bottom of the basin. This also includes removing accumulated sediment from the rock filter and restoring the rock filter to its original specified condition and any work necessary to restore all other components to the pre-maintenance conditions.

G. Sediment Barrier (baled straw)

Maintenance of sediment barrier (baled straw) consists of furnishing all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original ground line (0% filled). Also included is the removal of sediment accumulations on the bales by tapping.

H. Triangular Silt Barrier

Maintenance of Triangular Silt Barrier consists of all labor, tools, materials, equipment and necessary incidentals to remove and dispose of accumulated sediment down to the original ground line (0% filled).

I. Retrofit:

Maintenance of the retrofit device consists of all labor, tools, materials, equipment and necessary incidentals to remove and properly dispose of accumulated sediment in the permanent detention pond being utilized as a temporary sediment basin. This item also includes any maintenance that is required to ensure the retrofit device is maintained per Plan details and any maintenance of the stone filter to maintain its filtering ability, including cleaning and replacement.

J. Construction Exit:

Maintenance of the construction exit consists of all labor, tools, materials, equipment and incidentals, including additional stone and geotextile fabric as required to prevent the tracking or flow of soil onto public roadways. This includes, scarifying existing stone, cleaning existing stone, or placement of additional stone.

Cleaning of the construction exit by scraping and/or brooming only will not be measured for payment.

K. Inlet Sediment Trap

Maintenance of inlet sediment traps consists of all labor, tools, materials, equipment and necessary incidentals to remove and properly dispose of accumulated sediment in the trap and/or the excavated area adjacent to the trap. It also includes any maintenance that is required to remove sediment accumulations (“filtercake”) from the material selected to construct the inlet sediment trap.

165.3.06 Quality Acceptance

General Provisions 101 through 150.

165.3.07 Contractor Warranty and Maintenance

General Provisions 101 through 150.

165.4 Measurement

A. Temporary Silt Fence:

No separate measurement will be made.

B. Silt Control Gates:

No separate measurement will be made.
C. Check Dams (All Types):
   No separate measurement will be made.

D. Silt Retention Barrier:
   No separate measurement will be made.

E. Temporary Sediment Basins:
   No separate measurement will be made.

F. Sediment Barrier (baled straw)
   No separate measurement will be made.

F. Triangular Silt Barrier:
   No separate measurement will be made.

G. Retrofit:
   No separate measurement will be made.

H. Construction Exit:
   No separate measurement will be made.

I. Inlet Sediment Trap
   No separate measurement will be made.

165.4.01 Limits
General Provisions 101 through 150.

165.5 Payment
A. Temporary Silt Fence:
   To be paid for under CONSTRUCTION COMPLETE.

B. Silt Control Gates:
   To be paid for under CONSTRUCTION COMPLETE.

C. Check Dams (All Types):
   To be paid for under CONSTRUCTION COMPLETE.

D. Silt Retention Barrier:
   To be paid for under CONSTRUCTION COMPLETE.

E. Temporary Sediment Basins:
   To be paid for under CONSTRUCTION COMPLETE.

F. Sediment Barrier (baled straw):
   To be paid for under CONSTRUCTION COMPLETE.

G. Triangular Silt Barrier:
   To be paid for under CONSTRUCTION COMPLETE.

H. Retrofit:
   To be paid for under CONSTRUCTION COMPLETE.
I. Construction Exit:
   To be paid for under CONSTRUCTION COMPLETE.

J. Inlet Sediment Trap
   To be paid for under CONSTRUCTION COMPLETE.

165.5.01 Adjustments
General Provisions 101 through 150.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 167—Water Quality Monitoring

Add the following:

167.1 General Description
This Specification establishes the Contractor’s responsibility to meet the requirements of the National Pollutant Discharge Elimination System (NPDES) Infrastructure Permit No. GAR 100002 as it pertains to Part IV. Erosion, Sedimentation and Pollution Control Plan.

167.1.01 Definitions
Certified Personnel— certified personnel are defined as persons who have successfully completed the appropriate certification course approved by the Georgia Soil and Water Conservation Commission. For Department projects the certified person must also have successfully completed the Department’s WECS certification course.

167.1.02 Related References
A. Standard Specifications

Section 161—Control of Soil Erosion and Sedimentation

B. Referenced Documents

NPDES Infrastructure Permit No. GAR 100002, Part IV
GDOT WECS seminar.
Environmental Protection Divisions Rules and Regulations (Chapter 391-3-26)
Georgia Soil and Water Conservation Commission Certification Level IA course.

167.1.03 Submittals
General Provisions 101 through 150

167.2 Materials
General Provisions 101 through 150.

167.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.
167.3 Construction Requirements

167.3.01 Personnel
Use certified personnel to perform all monitoring, sampling, inspections, and rainfall data collection. Use the Contractor designated WECS or select a prequalified consultant from the Qualified Consultant List (QCL) to perform water quality monitoring. Ensure that monitoring consultants’ employees who perform monitoring, sampling, inspections, and rainfall data collection are GASWCC Certified.

167.3.02 Equipment
Provide equipment necessary to complete the Work or as directed.

167.3.03 Preparation
General Provisions 101 through 150.

167.3.04 Fabrication
General Provisions 101 through 150.

167.3.05 Construction

A. General
Perform inspections, rainfall data collection, testing of samples, and reporting the test results on the project according to the requirements in Part IV of the NPDES Infrastructure permit and this Specification.
Take samples manually or with the use of automatic samplers, according to the permit. Analyze all according to the permit, regardless of the method used to collect the samples.
If samples are analyzed in the field using portable turbidimeters, the monitoring results shall state that they are being used and a digital readout of NTUs is what is provided.
Submit bench sheets, work sheets, etc., when using portable turbidimeters. There are no exceptions to this requirement.
Perform required inspections and submit all reports required by this Specification within the time frames specified. Failure to perform the inspections within the time specified will result in the cessation of all construction activities with the exception of traffic control and erosion control. Failure to submit the required reports within the times specified will result in non-refundable deductions as specified in Subsection 161.5.01.B.

B. Inspections
The Department will provide one copy of required inspection forms for use and duplication. Inspection forms may change during the contract to reflect regulatory agency needs or the need of the Department. Any costs associated with the change of inspection forms shall be considered incidental. Alternate formats of the provided forms maybe created, used and submitted by the Contractor provided the required content and/or data fields and verbatim certification statements from the Department’s current forms are included.
The Engineer shall inspect the installation and condition of each erosion control device required by the erosion control plan within seven days after initial installation. This inspection is performed for each stage of construction when new devices are installed. The WECS shall ensure all installation deficiencies reported by the Engineer are corrected within two business days.
Ensure that the inspections of the areas listed below are conducted by certified personnel and at the frequencies listed. Document all inspections on the appropriate form provided by the Department.

1. Daily:
   a. Petroleum product storage, usage and handling areas
   b. All locations where vehicles enter/exit the site
      Continue these inspections until all entry and exit sites are stabilized and fuel is not stored or transferred on the site. Utilize the Daily inspection form.

2. Weekly and after Rainfall Events:
Conduct inspections on these areas every seven calendar days and within twenty-four hours after the end of a rainfall event that is 0.5 in (13 mm) or greater:

a. Disturbed areas not permanently stabilized  
b. Material storage areas  
c. Structural control measures, Best Management Practices (BMPs)  
d. Water quality monitoring locations and equipment  

Continue these inspections until all BMPs have been removed. Utilize the EC-1 Form.

3. Monthly:  
Once per month, inspect all areas where final stabilization has been completed. Look for evidence of sediments or pollutants entering the drainage system and or receiving waters. Inspect all permanent erosion control devices that remain in place to verify the maintenance status and that the devices are functioning properly.  
Continue these inspections until the Notice of Termination is submitted. Utilize the Monthly inspection form.

C. Reports:  
1. Inspection Reports:  
Summarize the results of inspections noted above in writing on the appropriate Daily, Weekly, Monthly or EC-1 form provided by the Department. Include the following information:

- Date(s) of inspection  
- Name of personnel performing inspection  
- Status of devices  
- Observations  
- Action taken  
- Signature of personnel performing the inspection  
- Any incidents of non-compliance  

The inspection form certification sheet shall be signed by the project WECS and the inspector performing inspections on behalf of the WECS (if not the same person).

Submit all inspection reports to the Engineer within twenty-four hours of the inspection. The Engineer will review the submitted reports and inspect the project to determine their accuracy. The Engineer will notify the certified personnel of any additional items that should be added to the inspection report.

Correct any items listed in the inspection report requiring routine maintenance within 72 (seventy-two) hours of notification.

Assume responsibility for all costs associated with additional sampling as specified in Part IV.D.6.d.3.(c) of the NPDES GAR 100002 permit if either of these conditions arise:

- BMPs shown in the Plans are not properly installed and maintained, or  
- BMPs designed by the Contractor are not properly designed, installed and maintained.

2. Monitoring Reports  
a. Report Requirements  
Include in all reports, the following certification statement, signed by the WECS or consultant providing monitoring on the project:

“I certify under penalty of law that this document and all attachments were prepared under my direct supervision in accordance with a system designed to assure that certified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information is, to the best of my knowledge and belief,
true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.”

When a rainfall event requires a sample to be taken, submit a report of the monitoring results to the Engineer within seven working days of the date the sample was obtained. Include the following information:

1) Date of sampling
2) Rainfall amount on sample date (sample date only)
3) NTU of sample & analysis method
4) Location where sample was taken (station number, etc.)
5) Receiving water or outfall sample
6) Project number and county
7) Whether the sample was taken by automatic sampler or manually (grab sample)

b. Report Requirements with No Qualifying Rainfall Events

In the event that a qualifying rainfall event does not occur prior to the submittal of the NOT (Notice of Termination), submit a report that states “No qualifying rainfall event occurred and no samples were taken.”

c. Test Results

Provide monitoring test results to the Engineer within 48 hours of the samples being analyzed. This notification may be verbal or written. This notification does not replace the requirement to submit the formal monitoring summary to the Engineer within 7 working days of the samples being collected.

3. Rainfall Data Reports

Record the measurement of rainfall once each twenty-four hour period. Measure rainfall data at the active phase of construction on the site.

Project rain gauges and those used to trigger the automatic samplers are to be emptied after every rainfall event. This will prevent a cumulative effect and prevent automatic samplers from taking samples even though the rainfall event was not a qualifying event.

The daily rainfall data supplied by the WECS to the Engineer will be the official rainfall data for the project.

167.3.06 Quality Acceptance

General Provisions 101 through 150.

167.3.07 Contractor Warranty and Maintenance

General Provisions 101 through 150.

167.4 Measurement

Water Quality Inspections in accordance with the inspection and reports sub-sections will not be measured separately for payment up to the time the Contract Time expires. Required inspections and reports after Contract Time has expired will not be measured for payment.

Water Quality Monitoring and Sampling are not measured separately for payment.

167.4.01 Limits

General Provisions 101 through 150. Submit the monitoring summary report to the Engineer within 7 working days.
167.5 Payment
Payment for Water Quality Monitoring and Sampling will be paid for under CONSTRUCTION COMPLETE.
Water Quality Monitoring and Sampling shall include meeting the requirements of the monitoring sections of the NPDES permit and this Specification, obtaining samples, analyzing samples, any and all necessary incidentals, and providing results of turbidity tests to the Engineer, within the time frame required by the NPDES Infrastructure permit, and this Specification.
This item is based on the rainfall events that require sampling as described in Part IV.D.5 of the permit. The Department will not pay for samples taken and analyzed for rainfall events that are not qualifying events as compared to the daily rainfall data supplied by the WECS.
Water Quality Inspections will be paid for under CONSTRUCTION COMPLETE. This shall include performing the requirements of the inspection section of the NPDES permit and this Specification, any and all necessary incidentals, and providing results of inspections to the Engineer, within the time frame required by the NPDES Infrastructure permit, and this Specification.

167.5.01 Adjustments
General Provisions 101 through 150.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA
SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 170—Silt Retention Barrier

Delete Subsection 170.3.05 and substitute the following:

170.3.05 Construction

Install a silt retention barrier as follows: Barriers shall be either staked or floating depending upon current, tides, water depth, and other variables, or as shown in the plans and contract.

A. Floating Silt Retention Barrier

1. Confine dredged materials to ponding areas or settlement basins using standpipes or weirs.
2. Place the barrier approximately 25 ft (7.5 m) outside the affected construction area, and at a depth within 5 ft (1.5 m) of the bottom.
3. If the body of water has a significant current, place the barrier parallel to the water flow.
4. Vary the dimensions and methods to suit the conditions and to meet the requirements of other local and State water control agencies to ensure that silt dispersion is effectively controlled.
5. Provide a fabric that is weighted to prevent the bottom from floating.

B. Staked Silt Retention Barrier

1. Where a staked barrier is used to protect a stream or inundated area, ensure the fabric:
   a. Extends to the bottom of the stream or inundated area and is weighted to prevent it from floating
   b. Is not trenched in at the bottom
   c. Extends 1 foot (300 mm) above normal water

2. Posts:
   a. Options: 2 inch (50 mm) x 4 inch (100 mm) wood; or 2 ½ inch (62.5 mm min. diameter) wood; or steel at a minimum of 1.33 pounds per foot (1.980 kg/m)
   b. space posts at a maximum spacing of 4 feet (1.2 m)
   c. posts are minimum of 5 feet (1.5 m) in length
   d. posts extend a minimum of 18 inches (450 mm) into the soil
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  
Project Number: CSNHS-0006-00(043)  
P.I. Number: 0006043  
Cherokee County  

Section 171—Silt Fence

Delete Section 171 and substitute the following:

171.1 General Description
This work includes furnishing, installing, and removing a water permeable filter fabric fence to remove suspended particles from drainage water.

171.1.01 Definitions
General Provisions 101 through 150.

171.1.02 Related References
A. Standard Specifications
   Section 163—Miscellaneous Erosion Control Items
   Section 700—Grassing
   Section 862—Wood Posts and Bracing
   Section 881—Fabrics
   Section 894—Fencing
B. Referenced Documents
   ASTM D 3786
   ASTM D 4355
   ASTM D 4632
   ASTM D 4751
   GDT 87
   QPL 36

171.1.03 Submittals
General Provisions 101 through 150.
171.2 Materials
Materials shall meet the requirements of the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Filter Fabrics</td>
<td>881</td>
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<tr>
<td>Fencing</td>
<td>894</td>
</tr>
<tr>
<td>Wood Posts and Bracing</td>
<td>862</td>
</tr>
</tbody>
</table>

Conditions during Project construction will affect the quantity of the silt fence to be installed.

The Engineer may increase, decrease, or eliminate the quantity at his or her direction. Variations in quantity are not changes in details of construction or in the character of the work.

For Type A, B, and C fences, use fabric as specified in Subsection 881.2.07, “Silt Fence Filter Fabric.”

171.2.01 Delivery, Storage, and Handling
During shipment and storage, wrap the fabric in a heavy-duty covering that will protect the cloth from sunlight, mud, dust, dirt, and debris. Do not expose the fabric to temperatures greater than 140 °F (60 °C).

When installed, the Engineer will reject the fabric if it has defects, rips, holes, flaws, deterioration, or damage incurred during manufacture, transportation, or storage.

171.3 Construction Requirements

171.3.01 Personnel
General Provisions 101 through 150.

171.3.02 Equipment
General Provisions 101 through 150.

171.3.03 Preparation
General Provisions 101 through 150.

171.3.04 Fabrication
General Provisions 101 through 150.

171.3.05 Construction
Install the silt fence according to this Specification, as shown on the Plans, or as directed by the Engineer as; perimeter, ditch check or similar protection.

A. Install Silt Fence

Install silt fence by either of the following methods:

1. Excavated Trench Method
   a. Excavate a trench 4 to 6 in (100 to 150 mm) deep using equipment such as a trenching machine or motor grader. If equipment cannot be operated on the site, excavate the trench by hand.

2. Soil Slicing Method
   a. Create a mechanical slice in the soil 8 to 12 in (200 to 300 mm) deep to receive the silt fence. Ensure that the width of the slice is not more than 3 in (75 mm). Mechanically insert the silt fence fabric into the slice in a simultaneous operation with the slicing that ensures consistent depth and placement.

Install the first post at the center of the low point (if applicable). Space the remaining posts a maximum of 6 ft (1.8 m) apart for Types A and B fence and 4 ft (1.2 m) apart for Type C fence.

Bury the posts at least 18 in (450 mm) into the ground. If this depth cannot be attained, secure the posts enough to prevent the fence from overturning from sediment loading.

Attach the filter fabric to the post using wire, cord, staples, nails, pockets, or other acceptable means.
a. Staples and Nails (Wood Posts): Evenly space staples or nails with at least five per post for Type A fence and four per post for Type B fence.

b. Pockets: If using pockets and they are not closed at the top, attach the fabric to a wood post using at least one additional staple or nail, or to a steel post using wire. Ensure that the additional attachment is within the top 6 in (150 mm) of the fabric.

Install the filter fabric so that 6 to 8 in (150 to 200 mm) of fabric is left at the bottom to be buried. Provide a minimum overlap of 18 in (450 mm) at all splice joints.

For Type C fence:

1. Woven Wire Supported
   a. Steel Post: Use wire to attach the fabric to the top of the woven wire support fence at the midpoint between posts. Also, use wire to attach the fabric to the post.

2. Polypropylene Mesh Supported
   a. Wood Post: Use at least six staples per post. Use two staples in a crisscross or parallel pattern to secure the top portion of the fence. Evenly space the remaining staples down the post.
   b. Steel Post: Use wire to attach the fabric and polypropylene mesh to the post.

Install the fabric in the trench so that 4 to 6 in (100 to 150 mm) of fabric is against the side of the trench with 2 to 4 in (50 to 100 mm) of fabric across the bottom in the upstream direction.

Backfill and compact the trench to ensure that flow cannot pass under the barrier. When the slice method is used, compact the soil disturbed by the slice on the upstream side of the silt fence first, and then compact the downstream side.

When installing a silt fence across a waterway that produces significant runoff, place a settling basin in front of the fence to handle the sediment load, if required. Construct a suitable sump hole or storage area according to Section 163.

B. Install silt fence ditch checks

   Temporary Silt Fence Ditch Checks

   Temporary silt fence ditch checks shall be constructed of the material type selected and shown on the approved erosion and sediment control plan. Item installation shall be constructed and placed according to approved Plan details. Temporary ditch checks shall remain in place until the permanent ditch protection is in place or being installed and the removal is approved by the Engineer.

C. Remove the Silt Fence

Keep all silt fence in place unless or until the Engineer directs it to be removed. A removed silt fence may be used at other locations if the Engineer approves of its condition.

After removing the silt fence, dress-the area to natural ground, grass-and mulch the area according to Section 700.

The silt fence shall remain until the Project is accepted or until the fence is removed. Also, remove and dispose of the silt accumulations at the silt fence.

Remove and replace any deteriorated filter fabric that reduces the effectiveness of the silt fence.

Repair or replace any undermined silt fence at no additional cost to the Department.

171.3.06 Quality Acceptance

Approved silt fence is listed in QPL 36. Approved fabrics must consistently exceed the minimum requirements of this Specification as verified by the Office of Materials and Research. The Office of Materials and Research will remove fabric that fails to meet the minimum requirements of this specification from the QPL until the products' acceptability has been reestablished to the Department’s satisfaction.

At the time of installation, the Engineer will reject the fabric if it has defects, rips, holes, flaws, deterioration, or damage incurred during manufacture, transportation, or storage.

171.3.07 Contractor Warranty
The silt fence shall remain until the Project is accepted or until the fence is removed. Also, remove and dispose of the silt accumulations at the silt fence.

Remove and replace any deteriorated filter fabric that reduces the effectiveness of the silt fence.

Repair or replace any undermined silt fence at no additional cost to the Department.

*Delete Subsection 171.4 and 171.5.*

*Add Subsection 170.4*

**Measurement and Payment:**

No separate measurement or payment will be made for this item. This item will be included in price bid for CONSTRUCTION COMPLETE. Includes furnishing materials, erecting the fence, dressing and grassing, when required, and removing the fence, when required.

If the silt fence must be repaired or removed, as the result of neglect or damage, perform the work at no additional cost to the Department.

**171.5.01 Adjustments**

General Provisions 101 through 150.

Office of Materials and Research
Delete Section 400 and substitute the following:

400.1 General Description
This work includes constructing one or more courses of bituminous plant mixture on the prepared foundation or existing roadway surface. The mixture shall conform with lines, grades, thicknesses, and typical cross sections shown on the Plans or established by the Engineer.

This section includes the requirements for all bituminous plant mixtures regardless of the gradation of the aggregates, type and amount of bituminous material, or pavement use.

Acceptance of work is on a lot-to-lot basis according to the requirements of this Section and Section 106.

400.1.01 Definitions
Segregated Mixture: Mixture lacking homogeneity in HMA constituents of such a magnitude that there is a reasonable expectation of accelerated pavement distress or performance problems. May be quantified by measurable changes in temperature, gradation, asphalt content, air voids, or surface texture.

New Construction: A roadway section more than 0.5 mile (800 m) long that is not longitudinally adjacent to the existing roadway. If more than one lane is added, and if any of the lanes are longitudinally adjacent to the existing lane, the lanes shall be tested under the criteria for a resurfacing project.

Trench Widening: Widening no more than 4 ft. (1.2 m) in width.

Comparison sample: Opposite quarter of material sampled by the Contractor.

Quality assurance sample: Independent sample taken by the Department.

Referee sample: A sample of the material remaining after quartering which is used for evaluation if a comparison of Contractor and Departmental test results is outside allowable tolerances.

400.1.02 Related References
A. Standard Specifications
   Section 106—Control of Materials
   Section 109—Measurement and Payment
   Section 152—Field Laboratory Building
   Section 413—Bituminous Tack Coat
   Section 424—Bituminous Surface Treatment
400.1.03 Submittals

A. Invoices

When the Department requests, furnish formal written invoices from a supplier for all materials used in production of HMA. Show the following on the Bill of Lading:

- Date shipped
- Quantity in tons (megagrams)
- Included with or without additives (for asphalt cement)

Purchase asphaltic cement from a supplier who will provide copies of Bill of Lading upon the Department’s request.
B. Paving Plan

Before starting asphaltic concrete construction, submit a written paving plan to the Engineer for approval. Include the following on the paving plan:

- Proposed starting date
- Location of plant(s)
- Rate of production
- Average haul distance(s)
- Number of haul trucks
- Paver speed feet (meter)/minute for each placement operation
- Mat width for each placement operation
- Number and type of rollers for each placement operation
- Sketch of the typical section showing the paving sequence for each placement operation
- Electronic controls used for each placement operation
- Temporary pavement marking plan

If staged construction is designated in the Plans or contract, provide a paving plan for each construction stage.

If segregation is detected, submit a written plan of measures and actions to prevent segregation. Work will not continue until the plan is submitted to and approved by the Department.

C. Job Mix Formula

After the Contract has been awarded, submit to the Engineer a written job mix formula proposed for each mixture type to be used based on an approved mix design. Furnish the following information for each mix:

- Specific project for which the mixture will be used
- Source and description of the materials to be used
- Mixture I.D. Number
- Proportions of the raw materials to be combined in the paving mixture
- Single percentage of the combined mineral aggregates passing each specified sieve
- Single percentage of asphalt by weight of the total mix to be incorporated in the completed mixture
- Single temperature at which to discharge the mixture from the plant
- Theoretical specific gravity of the mixture at the designated asphalt content
- Name of the person or agency responsible for quality control of the mixture during production

Do the following to have the formulas approved and to ensure their quality:

1. Submit proposed job mix formulas for review at least two weeks before beginning the mixing operations.
2. Do not start hot mix asphaltic concrete work until the Engineer has approved a job mix formula for the mixture to be used. No mixture will be accepted until the Engineer has given approval.
3. Provide mix designs for all Superpave and 4.75 mm mixes to be used. The Department will provide mix design results for other mixes to be used.
4. After a job mix formula has been approved, assume responsibility for the quality control of the mixtures supplied to the Department according to Subsection 106.01, “Source of Supply and Quantity of Materials.”

D. Quality Control Program

Submit a Quality Control Plan to the Office of Materials and Research for approval. The Quality Control Program will be included as part of the certification in the annual plant inspection report.

400.2 Materials

Ensure that materials comply with the specifications listed in Table 1.

Table 1—Materials Specifications
When required, provide Uintaite material, hereafter referred to by the common trade name Gilsonite, as a reinforcing agent for bituminous mixtures. Supply a manufacturer’s certification that the Gilsonite is a granular solid which meets the following requirements:

- Softening Point (AASHTO: T-53) 300-350 °F (150-175 °C)
- Specific Gravity, 77 °F (25 °C) (AASHTO: T-228) 1.04 ± 0.02
- Flash Point, COC (AASHTO: T-48) 550 °F (290 °C) Min.
- Ash Content (AASHTO: T-111) 1.0% Max.
- Penetration, 77 °F (25 °C), 100 gm., 5 sec. (AASHTO: T-49) 0

### 400.2.01 Delivery, Storage, and Handling

Storage of material is allowed in a properly sealed and insulated system for up to 24 hours except that Stone Matrix Asphalt (SMA), Open-Graded Friction Course (OGFC), or Porous European Mix (PEM) mixtures shall not be stored more than 12 hours. Mixtures other than SMA, OGFC, or PEM may be stored up to 72 hours in a sealed and insulated system, equipped with an auxiliary inert gas system, with the Engineer’s approval. Segregation, lumpiness, drain-down, or stiffness of stored mixture is cause for rejection of the mixture. The Engineer will not approve using a storage or surge bin if the mixture segregates, loses excessive heat, or oxidizes during storage.

The Engineer may obtain mixture samples or recover asphalt cement according to GDT 119. AASHTO T 202 and T 49 will be used to perform viscosity and penetration tests to determine how much asphalt hardening has occurred.

### A. Vehicles for Transporting and Delivering Mixtures

Ensure that trucks used for hauling bituminous mixtures have tight, clean, smooth beds.

Follow these guidelines when preparing vehicles to transport bituminous mixtures:

1. Use an approved releasing agent from QPL 39 in the transporting vehicle beds, if necessary, to prevent the mixture from sticking to the bed. Ensure that the releasing agent is not detrimental to the mixture. When applying the agent, drain the excess agent from the bed before loading. Remove from the project any transporting vehicles determined to contain unapproved releasing agents.
2. Protect the mixture with a waterproof cover large enough to extend over the sides and ends of the bed. Securely fasten the waterproof cover before the vehicle begins moving.
3. Insulate the front end and sides of each bed with an insulating material with the following specifications:
   - Consists of builders insulating board or equivalent
   - Has a minimum “R” value of 4.0
   - Can withstand approximately 400 °F (200 °C) temperatures

Install the insulating material so it is protected from loss and contamination. A “Heat Dump Body” may be used in lieu of insulation of the bed. “Heat Dump Body” refers to any approved transport vehicle that is capable of diverting
engine exhaust and transmitting heat evenly throughout the dump body to keep asphalt at required temperature. Mark the “Heat Dump Body” clearly with “OPEN” and “CLOSE” position at the exhaust diverter. Install a padlock and lock it in the “OPEN” position when the “Heat Dump Body” is used to transport bituminous mixtures.

4. Mark each transporting vehicle with a clearly visible identification number.

5. Create a hole in each side of the bed so that the temperature of the loaded mixture can be checked. The placement of these holes shall be located to assure that the thermometer is being placed in the hot mix asphaltic concrete. Ensure that the mixture is delivered to the roadway at a temperature within ± 20 °F (± 11 °C) of the temperature on the job mix formula.

If the Engineer determines that a truck may be hazardous to the Project or adversely affect the quality of the work, remove the truck from the project.

B. Containers for Transporting, Conveying, and Storing Bituminous Material

To transport, convey, and store bituminous material, use containers free of foreign material and equipped with sample valves. Bituminous material will not be accepted from conveying vehicles if material has leaked or spilled from the containers.

400.3 Construction Requirements

400.3.01 Personnel

General Provisions 101 through 150.

400.3.02 Equipment

Hot mix asphaltic concrete plants that produce mix for Department use are governed by Quality Assurance for Hot Mix Asphaltic Concrete Plants in Georgia, Laboratory Standard Operating Procedure No. 27.

The Engineer will approve the equipment used to transport and construct hot mix asphaltic concrete. Ensure that the equipment is in satisfactory mechanical condition and can function properly during production and placement operations. Place the following equipment at the plant or project site:

A. Field Laboratory

Provide a field laboratory according to Section 152.

B. Plant Equipment

1. Scales

Provide scales as follows:

a. Furnish (at the Contractor’s expense) scales to weigh bituminous plant mixtures, regardless of the measurement method for payment.

b. Ensure that the weight measuring devices that provide documentation comply with Subsection 109.01, “Measurement and Quantities.”

c. When not using platform scales, provide weight devices that record the mixture net weights delivered to the truck. A net weight system will include, but is not limited to:
   - Hopper or batcher-type weight systems that deliver asphaltic mixture directly to the truck
   - Fully automatic batching equipment with a digital recording device

d. Use a net weight printing system only with automatic batching and mixing systems approved by the Engineer.

e. Ensure that the net weight scale mechanism or device manufacturer, installation, performance, and operation meets the requirements in Subsection 109.01, “Measurement and Quantities.”

f. Provide information on the Project tickets according to Department of Transportation SOP-15.

2. Time-Locking Devices

Furnish batch type asphalt plants with automatic time-locking devices that control the mixing time automatically. Construct these devices so that the operator cannot shorten or eliminate any portion of the mixing cycle.

3. Surge- and Storage-Systems

Provide surge and storage bins as follows:
a. Ensure that bins for mixture storage are insulated and have a working seal, top and bottom, to prevent outside air infiltration and to maintain an inert atmosphere during storage. Bins not intended as storage bins may be used as surge bins to hold hot mixtures for part of the working day. However, empty these surge bins completely at the end of the working day.

b. Ensure that surge and storage bins can retain a predetermined minimum level of mixture in the bin when the trucks are loaded.

c. Ensure that surge and storage systems do not contribute to mix segregation, lumpiness, drain-down, or stiffness.

4. Controls for Dust Collector Fines

Control dust collection as follows:

a. When collecting airborne aggregate particles and returning them to the mixture, have the return system meter all or part of the collected dust uniformly into the aggregate mixture and waste the excess. The collected dust percentage returned to the mixture is subject to the Engineer’s approval.

b. When the collected dust is returned directly to the hot aggregate flow, interlock the dust feeder with the hot aggregate flow and meter the flow to maintain a flow that is constant, proportioned, and uniform.

5. Mineral Filler Supply System

When mineral filler is required as a mixture ingredient:

a. Use a separate bin and feed system to store and proportion the required quantity into the mixture with uniform distribution.

b. Control the feeder system with a proportioning device that meets these specifications:
   - Is accurate to within ± 10 percent of the filler required
   - Has a convenient and accurate means of calibration
   - Interlocks with the aggregate feed or weigh system to maintain the correct proportions for all rates of production and batch sizes

c. Provide flow indicators or sensing devices for the mineral filler system and interlock them with the plant controls to interrupt the mixture production if mineral filler introduction fails to meet the required target value after no longer than 60 seconds.

d. Add mineral filler to the mixture as follows, according to the plant type:
   - Batch Type Asphalt Plant. Add mineral filler to the mixture in the weigh hopper.
   - Continuous Plant Using Pugmill Mixers. Feed the mineral filler into the hot aggregate before it is introduced into the mixer so that dry mixing is accomplished before the bituminous material is added.
   - Continuous Plants Using the Drier-Drum Mixers. Add the mineral filler so that dry mixing is accomplished before the bituminous material is added and ensure that the filler does not become entrained into the air stream of the drier.

6. Hydrated Lime Treatment System

When hydrated lime is required as a mixture ingredient:

a. Use a separate bin and feed system to store and proportion the required quantity into the mixture.

b. Ensure that the aggregate is uniformly coated with hydrated lime aggregate before adding the bituminous material to the mixture. Add the hydrated lime so that it will not become entrained in the exhaust system of the drier or plant.

c. Control the feeder system with a proportioning device that meets these specifications:
   - Is accurate to within ± 10 percent of the amount required
   - Has a convenient and accurate means of calibration
   - Interlocks with the aggregate feed or weigh system to maintain the correct proportions for all rates of production and batch sizes and to ensure that mixture produced is properly treated with lime

d. Provide flow indicators or sensing devices for the hydrated lime system and interlock them with the plant controls to interrupt mixture production if hydrated lime introduction fails to meet the required target value after no longer than 60 seconds.

7. Net Weight Weighing Mechanisms
Certify the accuracy of the net weight weighing mechanisms by an approved registered scale serviceperson at least once every 6 months. Check the accuracy of net weight weighing mechanisms at the beginning of Project production and thereafter as directed by the Engineer. Check mechanism accuracy as follows:

a. Weigh a load on a set of certified commercial truck scales. Ensure that the difference between the printed total net weight and that obtained from the commercial scales is no greater than 4 lbs/1,000 lbs (4 kg/Mg) of load.

Check the accuracy of the bitumen scales as follows:

• Use standard test weights.
• If the checks indicate that printed weights are out of tolerance, have a registered scale serviceperson check the batch scales and certify the accuracy of the printer.
• While the printer system is out of tolerance and before its adjustment, continue production only if using a set of certified truck scales to determine the truck weights.

b. Have plants that use batch scales maintain ten 50 lb (25 kg) standard test weights at the plant site to check batching scale accuracy.

Ensure that plant scales that are used only to proportion mixture ingredients, not to determine pay quantities, are within two percent throughout the range.

8. Fiber Supply System
When stabilizing fiber is required as a mixture ingredient:

a. Use a separate feed system to store and proportion by weight the required quantity into the mixture with uniform distribution.

b. Control the feeder system with a proportioning device that meets these Specifications:

• Is accurate to within ± 10 percent of the amount required. Automatically adjusts the feed rate to maintain the material within this tolerance at all times
• Has a convenient and accurate means of calibration
• Provide in-process monitoring, consisting of either a digital display of output or a printout of feed rate, in pounds (kg) per minute, to verify feed rate
• Interlocks with the aggregate feed or weigh system to maintain the correct proportions for all rates of production and batch sizes

c. Provide flow indicators or sensing devices for the fiber system and interlock them with the plant controls to interrupt the mixture production if fiber introduction fails or if the output rate is not within the tolerances given above.

d. Introduce the fiber as follows:

• When a batch type plant is used, add the fiber to the aggregate in the weigh hopper. Increase the batch dry mixing time by 8 to 12 seconds from the time the aggregate is completely emptied into the mixer to ensure the fibers are uniformly distributed prior to the injection of asphalt cement into the mixer.
• When a continuous or drier-drum type plant is used, add the fiber to the aggregate and uniformly disperse prior to the injection of asphalt cement. Ensure the fibers will not become entrained in the exhaust system of the drier or plant.

C. Equipment at Project Site

1. Cleaning Equipment
Provide sufficient hand tools and power equipment to clean the roadway surface before placing the bituminous tack coat. Use power equipment that complies with Subsection 424.3.02.F, “Power Broom and Power Blower.”

2. Pressure Distributor
To apply the bituminous tack coat, use a pressure distributor that complies with Subsection 424.3.02.B, “Pressure Distributor.”

3. Bituminous Pavers
To place hot mix asphaltic concrete, use bituminous pavers that can spread and finish courses that are:

• As wide and deep as indicated on the Plans
• True to line, grade, and cross section
• Smooth
• Uniform in density and texture

a. Continuous Line and Grade Reference Control. Furnish, place, and maintain the supports, wires, devices, and materials required to provide continuous line and grade reference control to the automatic paver control system.
b. Automatic Screed Control System. Equip the bituminous pavers with an automatic screed control system actuated from sensor-directed mechanisms or devices that will maintain the paver screed at a pre-determined transverse slope and elevation to obtain the required surface.
c. Transverse Slope Controller. Use a transverse slope controller capable of maintaining the screed at the desired slope within ± 0.1 percent. Do not use continuous paving set-ups that result in unbalanced screed widths or off-center breaks in the main screed cross section unless approved by the Engineer.
d. Screed Control. Equip the paver to permit the following four modes of screed control. The method used shall be approved by the Engineer.

- Automatic grade sensing and slope control
- Automatic dual grade sensing
- Combination automatic and manual control
- Total manual control

Ensure that the controls are referenced with a taut string or wire set to grade, or with a ski-type device or mobile reference at least 30 ft (9 m) long when using a conventional ski. A non-contacting laser or sonar-type ski with at least four referencing mobile stations may be used with a reference at least 24 ft. (7.3 m) long. Under limited conditions, a short ski or shoe may be substituted for a long ski on the second paver operating in tandem, or when the reference plane is a newly placed adjacent lane.

Automatic screed control is required on all Projects; however, when the Engineer determines that Project conditions prohibit the use of such controls, the Engineer may waive the grade control, or slope control requirements, or both.

e. Paver Screed Extension. When the laydown width requires a paver screed extension, use bolt-on screed extensions to extend the screeds, or use an approved mechanical screed extension device. When the screed is extended, add auger extensions to assure a length of no more than 18 inches from the auger to the end gate of the paver. Auger extensions may be omitted when paving variable widths. Ensure the paver is equipped with tunnel extensions when the screed and augers are extended.

f. 30 - 45 Degree Wedge. When shown on/required by the plans, equip the paver to ensure a 30 degree minimum up to a 45 degree maximum wedge along the outside edge of the roadway (measured from the horizontal plane) is in place after final compaction on the final surface course. Use an approved mechanical device that will:

- Apply compactive effort to the asphalt mixture to eliminate objectionable voids as the mixture passes through the wedge device
- Produce a wedge with a uniform texture, shape, and density while automatically adjusting to varying heights encountered along the roadway shoulder.

NOTE: Do not use extendible strike-off devices instead of approved screed extensions. Only use a strike-off device in areas that would normally be luted in by hand labor.

4. Compaction Equipment

Ensure that the compaction equipment is in good mechanical condition and can compact the mixture to the required density. The compaction equipment number, type, size, operation, and condition is subject to the Engineer’s approval

5. Materials Transfer Vehicle (MTV)

a. Use a Materials Transfer Vehicle (MTV) when placing asphaltic concrete mixtures on Projects on the state route system with the following conditions. If a project fails to meet any one of the following conditions, the MTV’s use is not required.
1) When to use:
   • The ADT is equal to or greater than 6000,
   • The project length is equal to or greater than 3000 linear feet (915 linear meters),
   • The total tonnage (megagrams) of all asphaltic concrete mixtures is greater than 2000 tons (1815 Mg).

2) Where to use:
   • Mainline of the traveled way
   • Collector/distributor (C/D) lanes on Interstates and limited access roadways
   • Leveling courses at the Engineer’s discretion

3) Do not use the MTV for the following conditions:
   • A resurfacing project that only 9.5 mm mix is required.
   • A project with lane width that is equal or less than 11 feet.
   • A passing lane only project.
   • When noted on the plans.

b. Ensure the MTV and conventional paving equipment meet the following requirements:
   1) MTV
      • Has a truck unloading system which receives mixture from the hauling equipment and independently deliver mixtures from the hauling equipment to the paving equipment.
      • Has mixture remixing capability by either a storage bin in the MTV with a minimum capacity of 14 tons (13 megagrams) of mixture and a remixing system in the bottom of MTV storage bin, or a dual pugmill system located in the paver hopper insert with two full length transversely mounted paddle mixers to continuously blend the mixture as it discharges to a conveyor system.
      • Provides to the paver a homogeneous, non-segregated mixture of uniform temperature with no more than 20 °F(18 °C) difference between the highest and lowest temperatures when measured transversely across the width of the mat in a straight line at a distance of one foot to three feet from the screed while the paver is operating. Ensure that the MTV is capable of providing the paver a consistent material flow that is sufficient to prevent the paver from stopping between truck exchanges.

   2) Conventional Paving Equipment
      • Has a paver hopper insert with a minimum capacity of 14 tons (13 Mg) installed in the hopper of conventional paving equipment when an MTV is used.

   c. If the MTV malfunctions during spreading operations, discontinue placement of hot mix asphaltic concrete after there is sufficient hot mix placed to maintain traffic in a safe manner. However, placement of hot mix asphaltic concrete in a lift not exceeding 2 in. (50 mm) may continue until any additional hot mix in transit at the time of the malfunction has been placed. Cease spreading operations thereafter until the MTV is operational.

   d. Ensure the MTV is empty when crossing a bridge and is moved across without any other Contractor vehicles or equipment on the bridge. Move the MTV across a bridge in a travel lane and not on the shoulder. Ensure the speed of the MTV is no greater than 5 mph (8 kph) without any acceleration or deceleration while crossing a bridge.

400.3.03 Preparation

A. Prepare Existing Surface

Prepare the existing surface as follows:

1. Clean the Existing Surface. Before applying hot mix asphaltic concrete pavement, clean the existing surface to the Engineer’s satisfaction.

2. Patch and Repair Minor Defects
   Before placing leveling course:
   a. Correct potholes and broken areas that require patching in the existing surface and base as directed by the Engineer.
   b. Cut out, trim to vertical sides, and remove loose material from the areas to be patched.
Section 400—Hot Mix Asphaltic Concrete Construction

c. Prime or tack coat the area after it has been cleaned. Compact patches to the Engineer’s satisfaction. Material for patches does not require a job mix formula, but shall meet the gradation range shown in Section 828. The Engineer must approve the asphalt content to be used.

3. Apply Bituminous Tack Coat
   Apply the tack coat according to Section 413. The Engineer will determine the application rate, which must be within the limitations Table 2.

   **Table 2—Application Rates for Bituminous Tack, gal/yd² (L/m²)**

<table>
<thead>
<tr>
<th></th>
<th>Minimum</th>
<th>Maximum</th>
</tr>
</thead>
<tbody>
<tr>
<td>Under OGFC and PEM Mixes</td>
<td>0.06 (0.270)</td>
<td>0.08 (0.360)</td>
</tr>
<tr>
<td>All Other Mixes</td>
<td>0.04 (0.180)</td>
<td>0.06 (0.270)</td>
</tr>
</tbody>
</table>

   *On thin leveling courses and freshly placed asphaltic concrete mixes, reduce the application rate to 0.02 to 0.04 gal/yd² (0.09 to 0.18 L/m²).

B. Place Patching and Leveling Course

1. When the existing surface is irregular, bring it to the proper cross section and grade with a leveling course of hot mix asphaltic concrete materials.

2. Place leveling at the locations and in the amounts directed by the Engineer.

3. Use leveling course mixtures that meet the requirements of the job mix formulas defined in:
   - Subsection 400.3.05.A, “Observe Composition of Mixtures”
   - Section 828
   - Leveling acceptance schedules in Subsection 400.3.06.A, “Acceptance Plans for Gradation and Asphalt Cement Content”

4. If the leveling and patching mix type is undesignated, determine the mix type by the thickness or spread rate according to Table 3, but do not use 4.75 mm mix on interstate projects.

   **Table 3—Leveling and Patching Mix Types**

<table>
<thead>
<tr>
<th>Thickness</th>
<th>Rate of Spread</th>
<th>Type of Mix</th>
</tr>
</thead>
<tbody>
<tr>
<td>Up to 0.75 in  (19 mm)</td>
<td>Up to 85 lbs/yd² (45 kg/m²)</td>
<td>4.75 mm Mix or 9.5 mm Superpave Type 1</td>
</tr>
<tr>
<td>0.75 to 1.5 in (19 to 38 mm)</td>
<td>85 to 165 lbs/yd² (45 to 90 kg/m²)</td>
<td>9.5 mm Superpave Type 2</td>
</tr>
<tr>
<td>1.5 to 2 in (38 to 50 mm)</td>
<td>165 to 220 lbs/yd² (90 to 120 kg/m²)</td>
<td>12.5 mm Superpave *</td>
</tr>
<tr>
<td>2 to 2.5 in (50 to 64 mm)</td>
<td>220 to 275 lbs/yd² (120 to 150 kg/m²)</td>
<td>19 mm Superpave *</td>
</tr>
<tr>
<td>Over 2.5 in (64 mm)</td>
<td>Over 275 lbs/yd² (150 kg/m²)</td>
<td>25 mm Superpave</td>
</tr>
</tbody>
</table>

   * These mixtures may be used for isolated patches no more than 6 in. (150 mm) deep and no more than 4 ft. (1.2 m) in diameter or length.

400.3.04 Fabrication
General Provisions 101 through 150.

400.3.05 Construction
Provide the Engineer at least one day’s notice prior to beginning construction, or prior to resuming production if operations have been temporarily suspended.

A. Observe Composition of Mixtures

1. Calibration of plant equipment
If the material changes, or if a component affecting the ingredient proportions has been repaired, replaced, or adjusted, check and recalibrate the proportions.

Calibrate as follows:

a. Before producing mixture for the Project, calibrate by scale weight the electronic sensors or settings for proportioning mixture ingredients.

b. Calibrate ingredient proportioning for all rates of production.

2. Mixture control

Compose hot mix asphaltic concrete from a uniform mixture of aggregates, bituminous material, and if required, hydrated lime, mineral filler, or other approved additive.

Make the constituents proportional to produce mixtures that meet the requirements in Section 828. The general composition limits prescribed are extreme ranges within which the job mix formula must be established. Base mixtures on a design analysis that meets the requirements of Section 828. Ensure that the field performance of the in-place mixtures meet the requirements of Subsection 828.2B.

If control test results show that the characteristic tested does not conform to the job mix formula control tolerances given in Section 828, take immediate action to ensure that the quality control methods are effective.

Control the materials to ensure that extreme variations do not occur. Maintain the gradation within the composition limits in Section 828.

B. Prepare Bituminous Material

Uniformly heat the bituminous material to the temperature specified in the job mix formula with a tolerance of ± 20 °F (± 10 °C).

C. Prepare the Aggregate

Prepare the aggregate as follows:

1. Heat the aggregate for the mixture, and ensure a mix temperature within the limits of the job mix formula.

2. Do not contaminate the aggregate with fuel during heating.

3. Reduce the absorbed moisture in the aggregate until the asphalt does not separate from the aggregate in the prepared mixture. If this problem occurs, the Engineer will establish a maximum limit for moisture content in the aggregates. When this limit is established, maintain the moisture content below this limit.

D. Prepare the Mixture

Proportion the mixture ingredients as necessary to meet the required job mix formula. Mix until a homogenous mixture is produced.

1. Add Mineral Filler

When mineral filler is used, introduce it in the proper proportions and as specified in Subsection 400.3.02.B.5, “Mineral Filler Supply System.”

2. Add Hydrated Lime

When hydrated lime is included in the mixture, add it at a rate specified in Section 828 and the job mix formula. Use methods and equipment for adding hydrated lime according to Subsection 400.3.02.B.6, “Hydrated Lime Treatment System.”

Add hydrated lime to the aggregate by using Method A or B as follows:

Method A—Dry Form—Add hydrated lime in its dry form to the mixture as follows, according to the type of plant:

a. Batch Type Asphalt Plant: Add hydrated lime to the mixture in the weigh hopper or as approved and directed by the Engineer.

b. Continuous Plant Using Pugmill Mixer: Feed hydrated lime into the hot aggregate before it is introduced into the mixer so that dry mixing is complete before the bituminous material is added.

c. Continuous Plant Using Drier-Drum Mixer: Add hydrated lime so that the lime will not become entrained into the air stream of the drier and so that thorough dry mixing will be complete before the bituminous material is added.
Method B—Lime/Water Slurry—Add the required quantity of hydrated lime (based on dry weight) in lime/water slurry form to the aggregate. This solution consists of lime and water in concentrations as directed by the Engineer.

Equip the plant to blend and maintain the hydrated lime in suspension and to mix it with the aggregates uniformly in the proportions specified.

3. Add Stabilizing Fiber

When stabilizing fiber is included in the mixture, add it at a rate specified in Section 819 and the Job Mix Formula. Introduce it as specified in Subsection 400.3.02.B.8, “Fiber Supply System.”

4. Add Gilsonite Modifier

When required, add the Gilsonite modifier to the mixture at a rate such that eight percent by weight of the asphalt cement is replaced by Gilsonite. Use either PG 64-22 or PG 67-22 asphalt cement as specified in Subsection 820.2.01. Provide suitable means to calibrate and check the rate of Gilsonite being added. Introduce Gilsonite modifier by either of the following methods.

a. For batch type plants, incorporate Gilsonite into the pugmill at the beginning of the dry mixing cycle. Increase the dry mix cycle by a minimum of 10 seconds after the Gilsonite is added and prior to introduction of the asphalt cement. For this method, supply Gilsonite in plastic bags to protect the material during shipment and handling and store the modifier in a waterproof environment. The bags shall be capable of being completely melted and uniformly blended into the combined mixture.

Gilsonite may also be added through a mineral filler supply system as described in Subsection 400.3.02.B.5, “Mineral Filler Supply System.” The system shall be capable of injecting the modifier into the weigh hopper near the center of the aggregate batching cycle so the material can be accurately weighed.

b. For drum drier plants, add Gilsonite through the recycle ring or through an acceptable means which will introduce the Gilsonite prior to the asphalt cement injection point. The modifier shall be proportionately fed into the drum mixer at the required rate by a proportioning device which shall be accurate within ±10 percent of the amount required. The entry point shall be away from flames and ensure the Gilsonite will not be caught up in the air stream and exhaust system.

5. Materials from Different Sources

Do not use mixtures prepared from aggregates from different sources intermittently. This will cause the color of the finished pavement to vary.

E. Observe Weather Limitations

Do not mix and place asphaltic concrete if the existing surface is wet or frozen. Do not lay asphaltic concrete OGFC mix or PEM at air temperatures below 60 °F (16 °C). When using a MTV, OGFC mix or PEM may be placed at 55 °F (13 °C) when approved by the Engineer. For other courses, follow the temperature guidelines in the following table:

**Table 4—Lift Thickness Table**

<table>
<thead>
<tr>
<th>Lift Thickness</th>
<th>Minimum Temperature</th>
</tr>
</thead>
<tbody>
<tr>
<td>1 in (25 mm) or less</td>
<td>55 °F (13 °C)</td>
</tr>
<tr>
<td>1.1 to 2 in (26 mm to 50 mm)</td>
<td>45 °F (8 °C)</td>
</tr>
<tr>
<td>2.1 to 3 in (51 mm to 75 mm)</td>
<td>40 °F (4 °C)</td>
</tr>
<tr>
<td>3.1 to 4 in (76 mm to 100 mm)</td>
<td>35 °F (2 °C)</td>
</tr>
<tr>
<td>4.1 to 8 in (101 mm to 200 mm)</td>
<td>32 °F (0 °C) and rising. Base Material must not be frozen</td>
</tr>
</tbody>
</table>

F. Perform Spreading and Finishing

Spread and finish the course as follows:

1. Determine the course’s maximum compacted layer thickness by the type mix being used according to Table 5.

**Table 5—Maximum Layer Thickness**
<table>
<thead>
<tr>
<th>Mix Type</th>
<th>Minimum Layer Thickness</th>
<th>Maximum Layer Thickness</th>
<th>Maximum Total Thickness</th>
</tr>
</thead>
<tbody>
<tr>
<td>25 mm Superpave</td>
<td>2 1/2 in (64 mm)</td>
<td>4 in (100 mm) *</td>
<td>—</td>
</tr>
<tr>
<td>19 mm Superpave</td>
<td>1 3/4 in (44 mm)</td>
<td>3 in (75 mm) *</td>
<td>—</td>
</tr>
<tr>
<td>12.5 mm Superpave</td>
<td>1 3/8 in (35 mm)</td>
<td>2 1/2 in (64 mm) *</td>
<td>8 in (200 mm)</td>
</tr>
<tr>
<td>9.5 mm Superpave Type 2</td>
<td>1 1/8 in. (28 mm)</td>
<td>1 1/2 in (38 mm)</td>
<td>4 in (100 mm)</td>
</tr>
<tr>
<td>9.5 mm Superpave Type 1</td>
<td>7/8 in (22 mm)</td>
<td>1 1/4 in (32 mm)</td>
<td>4 in (100 mm)</td>
</tr>
<tr>
<td>4.75 mm Mix</td>
<td>3/4 in (19 mm)</td>
<td>1 1/8 in (28 mm)</td>
<td>2 in (50 mm)</td>
</tr>
<tr>
<td>9.5 mm OGFC</td>
<td>55 lbs/yd² (30 kg/m²)</td>
<td>65 lbs/yd² (36 kg/m²)</td>
<td>—</td>
</tr>
<tr>
<td>12.5 mm OGFC</td>
<td>85 lbs/yd² (47 kg/m²)</td>
<td>95 lbs/yd² (53 kg/m²)</td>
<td>—</td>
</tr>
<tr>
<td>12.5 mm PEM</td>
<td>110 lbs/yd² (80 kg/m²)</td>
<td>165 lbs/yd² (90 kg/m²)</td>
<td>—</td>
</tr>
<tr>
<td>9.5 mm SMA</td>
<td>1 1/8 in (28 mm)</td>
<td>1 1/2 in (38 mm)</td>
<td>4 in (100 mm)</td>
</tr>
<tr>
<td>12.5 mm SMA</td>
<td>1 1/4 in (32 mm)</td>
<td>3 in (75 mm)</td>
<td>6 in (150 mm)</td>
</tr>
<tr>
<td>19 mm SMA</td>
<td>1 3/4 in (44 mm)</td>
<td>3 in (75 mm)</td>
<td>—</td>
</tr>
</tbody>
</table>

* Allow up to 6 in (150 mm) per lift on trench widening. Place 9.5 mm Superpave and 12.5 mm Superpave up to 4 in (100 mm) thick for driveway and side road transition.

2. Unload the mixture into the paver hopper or into a device designed to receive the mixture from delivery vehicles.
3. Except for leveling courses, spread the mixture to the loose depth for the compacted thickness or the spread rate. Use a mechanical spreader true to the line, grade, and cross section specified.
4. For leveling courses, use a motor grader equipped with a spreader box and smooth tires to spread the material or use a mechanical spreader meeting the requirements in Subsection 400.3.02.C, “Equipment at Project Site.”
5. Obtain the Engineer’s approval for the sequence of paving operations, including paving the adjoining lanes. Minimize tracking tack onto surrounding surfaces.
6. Ensure that the outside edges of the pavement being laid are aligned and parallel to the roadway center line.
7. For New Construction or Resurfacing Contracts that contain multiple lifts or courses, arrange the width of the individual lifts so that the longitudinal joints of each successive lift are offset from the previous lift at least 1 ft (300 mm). This requirement does not apply to the lift immediately over thin lift leveling courses. Ensure that the longitudinal joint(s) in the surface course and the mix immediately underneath asphaltic concrete OGFC or PEM are at the lane line(s).

NOTE: Perform night work with artificial light provided by the Contractor and approved by the Engineer.

8. Where mechanical equipment cannot be used, spread and rake the mixture by hand. Obtain the Engineer’s approval of the operation sequence, including compactive methods, in these areas.
9. Keep small hand raking tools clean and free from asphalt build up. Do not use fuel oil or other harmful solvents to clean tools during the work.
10. Do not use mixture with any of these characteristics:
    - Segregated
    - Nonconforming temperature
    - Deficient or excessive asphalt cement content
    - Otherwise unsuitable to place on the roadway in the work
11. Remove and replace mixture placed on the roadway that the Engineer determines has unacceptable blemish levels from segregation, streaking, pulling and tearing, or other characteristics. Replace with acceptable mixture at the Contractor’s expense. Do not continually place mixtures with deficiencies.
Do not place subsequent course lifts over another lift or courses placed on the same day while the temperature of the previously placed mix is 140 °F (60 °C) or greater.

12. Obtain the Engineer’s approval of the material compaction equipment. Perform the rolling as follows:
   a. Begin the rolling as close behind the spreader as possible without causing excessive distortion of the asphaltic concrete surface.
   b. Continue rolling until roller marks are no longer visible.
   c. Use pneumatic-tired rollers with breakdown rollers on all courses except asphaltic concrete OGFC, PEM and SMA or other mixes designated by the Engineer.

13. If applicable, taper or “feather” asphaltic concrete from full depth to a depth no greater than 0.5 in (13 mm) along curbs, gutters, raised pavement edges, and areas where drainage characteristics of the road must be retained. The Engineer will determine the location and extent of tapering.

G. Maintain Continuity of Operations

Coordinate plant production, transportation, and paving operations to maintain a continuous operation. If the spreading operations are interrupted, construct a transverse joint if the mixture immediately behind the paver screed cools to less than 250 °F (120 ºC).

H. Construct the Joints

1. Construct Transverse Joints
   a. Construct transverse joints to facilitate full depth exposure of the course before resuming placement of the affected course.
   b. Properly clean and tack the vertical face of the transverse joint before placing additional material.

   **NOTE: Never burn or heat the joint by applying fuel oil or other volatile materials.**

   c. Straightedge transverse joints immediately after forming the joint.
   d. Immediately correct any irregularity that exceeds 3/16 in. in 10 ft (5 mm in 3 m).

2. Construct Longitudinal Joints

   Clean and tack the vertical face of the longitudinal joint before placing adjoining material. Construct longitudinal joints so that the joint is smooth, well sealed, and bonded.

3. Construction Joint Detail for OGFC and PEM Mixtures

   In addition to meeting joint requirements described above, construct joints and transition areas for 12.5 mm OGFC and 12.5 mm PEM mixtures as follows:

   a. For projects which do not have milling included as a pay item:
      1) Place OGFC mixture meeting gradation requirements of 9.5 mm OGFC as specified in Section 828 on entrance and exit ramp gore areas and end of project construction joints.
         - Taper mixture from 3/8 in (10 mm) at end of project to full plan depth within maximum distance of spread for one load of mixture
         - Taper mixture placed on gore areas from thickness of the edge of the mainline to 3/8 in (10 mm) at the point of the ramp transverse joint.
      2) Construct the ramp transverse joint at the point specified in the plans or as directed by the Engineer.
      3) Mixture placed in the transition and gore areas will be paid for at the contract unit price for 12.5 mm OGFC or 12.5 mm PEM as applicable.

   b. For projects which have milling included as a pay item:
      1) Taper milling for a distance of no less than 50 ft (15 m) to a depth of 2 1/4 in (59 mm) at the point of the transverse joint
      2) Taper thickness, if needed, of the dense-graded surface mix within the 50 ft (15 m) distance to 1 1/2 in (40 mm) at the point of the transverse joint
      3) Taper thickness of the 12.5 mm OGFC or 12.5 mm PEM to 3/4 in (19 mm) so that it ties in at grade level with the existing surface at the point of the transverse joint
I. Protect the Pavement

Protect sections of the newly finished pavement from traffic until the traffic will not mar the surface or alter the surface texture. If directed by the Engineer, use artificial methods to cool the newly finished pavement to open the pavement to traffic more quickly.

J. Modify the Job Mix Formula

If the Engineer determines that undesirable mixture or mat characteristics are being obtained, the job mix formula may require immediate adjustment.

400.3.06 Quality Acceptance

A. Acceptance Plans for Gradation and Asphalt Cement Content

The Contractor will randomly sample and test mixtures for acceptance on a lot basis. The Department will monitor the Contractor testing program and perform comparison and quality assurance testing.

1. Determine Lot Amount

A lot consists of the tons (megagrams) of asphaltic concrete produced and placed each production day. If this production is less than 500 tons (500 Mg), or its square yard (meter) equivalent, production may be incorporated into the next working day. The Engineer may terminate a lot when a pay adjustment is imminent if a plant or materials adjustment resulting in a probable correction has been made. Terminate all open lots at the end of the month, except for materials produced and placed during the adjustment period. The lot will be terminated as described in Subsection 400.5.01, "Adjustments."

If the final day’s production does not constitute a lot, the production may be included in the lot for the previous day’s run; or, the Engineer may treat the production as a separate lot with a corresponding lower number of tests.

2. Determine Lot Acceptance

Determine lot acceptance as found in Subsection 400.5.01, "Adjustments."

The Department will perform the following task:

Determine the pay factor by using the mean of the deviations from the job mix formula of the tests in each lot and apply it to Table 9—Mixture Acceptance Schedule for Surface Mixes or Table 10—Mixture Acceptance Schedule for Subsurface Mixes, whichever is appropriate. This mean will be determined by averaging the actual numeric value of the individual deviations from the job mix formula, disregarding whether the deviations are positive or negative amounts. Do not calculate lot acceptance using test results for materials not used in the Work. Determine the pay factor for each lot by multiplying the contract unit price by the appropriate pay factor from the Mixture Acceptance Schedule - Table 9 or Table 10. When two or more pay factors for a specific lot are less than 1.0, determine the adjusted payment by multiplying the contract unit price by the lowest pay factor.

If the mean of the deviations from the job mix formula of the lot acceptance tests for a control sieve or for asphalt cement content exceeds the tolerances established in the appropriate Mixture Acceptance Schedule, and if the Engineer determines that the material need not be removed and replaced, the lot may be accepted at an adjusted unit price as determined by the Engineer. If the Engineer determines that the material is not acceptable to leave in place, the materials shall be removed and replaced at the Contractor’s expense.

3. Provide Quality Control Program

Provide a Quality Control Program as established in SOP 27 which includes:

- Assignment of quality control responsibilities to specifically named individuals who have been certified by the Office of Materials and Research
- Provisions for prompt implementation of control and corrective measures
- Provisions for communication with Project Manager, Bituminous Technical Services Engineer, and Testing Management Operations Supervisor at all times
- Provisions for reporting all test results daily through the Office of Materials and Research computerized Field Data Collection System; other checks, calibrations and records will be reported on a form developed by the Contractor and will be included as part of the project records
- Notification in writing of any change in quality control personnel

a. Certification Requirements:
• Use laboratory and testing equipment certified by the Department. (Laboratories which participate in and maintain AASHTO accreditation for testing asphaltic concrete mixtures will be acceptable in lieu of Departmental certification.)

• Provide certified quality control personnel to perform the sampling and testing. A Quality Control Technician (QCT) may be certified at three levels:
  1) Temporary Certification – must be a technician trainee who shall be given direct oversight by a certified Level 1 or Level 2 QCT while performing acceptance testing duties during the first 5 days of training. The trainee must complete qualification requirements within 30 production days after being granted temporary certification. A trainee who does not become qualified within 30 production days will not be re-eligible for temporary certification. A certified Level 1 or Level 2 QCT shall be at the plant at all times during production and shipment of mixture to monitor work of the temporarily certified technician.
  2) Level 1 – must demonstrate they are competent in performing the process control and acceptance tests and procedures related to hot mix asphalt production and successfully pass a written exam.
  3) Level 2 – must meet Level 1 requirements and must be capable of and responsible for making process control adjustments, and successfully pass a written exam.
    • Technician certification is valid for 3 years from the date on the technician’s certificate unless revoked or suspended. Eligible technicians may become certified through special training and testing approved by the Office of Materials and Research. Technicians who lose their certification due to falsification of test data will not be eligible for recertification in the future unless approved by the State Materials and Research Engineer.

b. Quality Control Management
  1) Designate at least one Level 2 QCT as manager of the quality control operation. The Quality Control Manager shall meet the following requirements:
    • Be accountable for actions of other QCT personnel
    • Ensure that all applicable sampling requirements and frequencies, test procedures, and Standard Operating Procedures are adhered to
    • Ensure that all reports, charts, and other documentation is completed as required

  2) Provide QCT personnel at the plant as follows:
    • If daily production for all mix types is to be greater than 250 tons (megagrams), have a QCT person at the plant at all times during production and shipment of mixture until all required acceptance tests have been completed
    • If daily production for all mix types will not be greater than 250 tons (megagrams) a QCT may be responsible for conducting tests at up to two plants, subject to random number sample selection
    • Have available at the plant or within immediate contact by phone or radio a Level 2 QCT responsible for making prompt process control adjustments as necessary to correct the mix

  3) Sampling, Testing, and Inspection Requirements.
  Provide all sample containers, extractants, forms, diaries, and other supplies subject to approval of the Engineer.
  Perform daily sampling, testing, and inspection of mixture production that meets the following requirements:
  (a) Randomly sample mixtures according to GSP 15, and GDT 73 (Method C) and test on a lot basis. In the event less than the specified number of samples are taken, obtain representative 6 in (150 mm) cores from the roadway at a location where the load not sampled was placed. Take enough cores to ensure minimum sample size requirements are met for each sample needed.
  (b) Maintain a printed copy of the computer generated random sampling data as a part of the project records.
  (c) Perform sampling, testing, and inspection duties of GSP 21.
  (d) Perform extraction or ignition test (GDT 83 or GDT 125) and extraction analysis (GDT 38). If the ignition oven is used, a printout of sample data including weights shall become a part of the project
records. For asphalt cement content only, digital printouts of liquid asphalt cement weights may be substituted in lieu of an extraction test for plants with digital recorders. Calculate the asphalt content from the ticket representing the mixture tested for gradation.

(e) Save extracted aggregate, opposite quarters, and remaining material (for possible referee testing) of each sample as follows:
   - Store in properly labeled, suitable containers
   - Secure in a protected environment
   - Store for three working days. If not obtained by the Department, within three days they may be discarded.

(f) Add the following information on load tickets from which a sample or temperature check is taken:
   - Mixture temperature
   - Signature of the QCT person performing the testing

(g) Calibrate the lime system when hydrated lime is included in the mixture:
   - Perform a minimum of twice weekly during production
   - Post results at the plant for review
   - Provide records of materials invoices upon request (including asphalt cement, aggregate, hydrated lime, etc.)

(h) Take action if acceptance test results are outside Mixture Control Tolerances of Section 828.
   - One sample out of tolerance
     1. Contact Level 2 - QCT to determine if a plant adjustment is needed
     2. Immediately run a process control sample. Make immediate plant adjustments if this sample is also out of tolerance
   - Two consecutive acceptance samples of the same mix type out of tolerance regardless of Lot or mix design level, or three consecutive acceptance samples out of tolerance regardless of mix type
     1. Stop plant production immediately
     2. Reject any mixture already in storage that:
        - Deviates more than 10 percent in gradation from the job mix formula based on the acceptance sample
        - Deviates more than 0.7 percent in asphalt content from the job mix formula based on the acceptance sample
     3. Make a plant correction to any mix type out of tolerance prior to resuming production
        - Do not send any mixture to the project before test results of a process control sample meets Mixture Control Tolerances
        - Reject any mixture produced at initial restarting that does not meet Mixture Control Tolerances

4) Comparison Testing and Quality Assurance Program
   Periodic comparison testing by the Department will be required of each QCT to monitor consistency of equipment and test procedures. The Department will take independent samples to monitor the Contractor's quality control program.
   a) Comparison Sampling and Testing

   NOTE: Determine mixture temperature at least once per hour of production for OGFC and PEM mixes.
Retain samples for comparison testing and referee testing if needed as described in Subsection 400.3.06.A.3.b.3. Discard these samples only if the Contractor's acceptance test results meet a 1.00 pay factor and the Department does not procure the samples within three working days. The Department will test comparison samples on a random basis. Results will be compared to the respective contractor acceptance tests and the maximum difference shall be as follows:

Table 6—Allowable Percent Difference Between Department and Contractor Acceptance Tests

<table>
<thead>
<tr>
<th>SIEVE SIZE</th>
<th>SURFACE</th>
<th>SUB-SURFACE</th>
</tr>
</thead>
<tbody>
<tr>
<td>1/2 in. (12.5 mm)</td>
<td>3.5%</td>
<td>4.0%</td>
</tr>
<tr>
<td>3/8 in. (9.5 mm)</td>
<td>3.5%</td>
<td>4.0%</td>
</tr>
<tr>
<td>No. 4 (4.75 mm)</td>
<td>2.5%</td>
<td>3.5%</td>
</tr>
<tr>
<td>No. 8 (2.36 mm)</td>
<td>2.5%</td>
<td>3.0%</td>
</tr>
<tr>
<td>No. 200 (75 µm)</td>
<td>2.0%</td>
<td>2.0%</td>
</tr>
<tr>
<td>A.C.</td>
<td>0.4%</td>
<td>0.5%</td>
</tr>
</tbody>
</table>

NOTE: Pavement courses to be overlaid with OGFC or PEM mixes are considered surface mixes.

(1) If test comparisons are within these tolerances:
   - Continue production
   - Use the Contractor's tests for acceptance of the lot

(2) If test comparisons are not within these tolerances:
   - Another Departmental technician will test the corresponding referee sample
   - Results of the referee sample will be compared to the respective contractor and Departmental tests using the tolerance for comparison samples given above.
     (a) If referee test results are within the above tolerances when compared to the Contractor acceptance test, use the Contractor's test for acceptance of the effected lot.
     (b) If referee test results are not within the above tolerances when compared to the Contractor acceptance test, the Department will review the Contractor's quality control methods and determine if a thorough investigation is needed.

b) Quality Assurance Sampling and Testing
   (1) Randomly take a minimum of two quality assurance samples from the lesser of five days or five lots of production regardless of mix type or number of projects.
   (2) Compare test deviation from job mix formula to Mixture Control Tolerances in Section 828. If results are outside these tolerances, another sample from the respective mix may be taken.

NOTE: For leveling courses less than 110 lb/yd² (60 kg/m²) that have quality assurance test results outside the Mixture Control Tolerances of Section 828, use the Department's test results only and applicable pay factors will apply.
If test results of the additional sample are not within Mixture Control Tolerances, the Department will take the following action:

- Take random samples from throughout the lot as in Subsection 400.3.06.A.3.b.3 and use these test results for acceptance and in calculations for the monthly plant rating. Applicable pay factors will apply and the contractor QCT test results will not be included in pay factor calculations nor in the monthly plant rating.
- Determine if the Contractor’s quality control program is satisfactory and require prompt corrective action by the Contractor if specification requirements are not being met.
- Determine if the QCT has not followed Departmental procedures or has provided erroneous information.
- Take samples of any in-place mixture represented by unacceptable QCT tests and use the additional sample results for acceptance and in calculations for the monthly plant rating and apply applicable pay factors. The Contractor QCT tests will not be included in the pay factor calculations nor in the monthly plant rating.

B. Compaction

Determine the mixture compaction using either GDT 39 or GDT 59. The compaction is accepted in lots defined in Subsection 400.3.06. A “Acceptance Plans for Gradation and Asphalt Cement Content” and is within the same lot boundaries as the mixture acceptance.

1. Calculate Pavement Mean Air Voids

The Department will calculate the pavement air voids placed within each lot as follows:

- One test per sub-lot.
- Average the results of all tests run on randomly selected sites in that lot.
- Select the random sites using GDT 73.

Density tests are not required for asphaltic concrete placed at 90 lbs/yd\(^2\) (50 kg/m\(^2\)) or less, 4.75 mm mix, and asphaltic concrete OGFC, PEM and mixes placed as variable depth or width leveling. Compact these courses to the Engineer’s satisfaction. Density tests will not be performed on turn-outs and driveways.

The targeted maximum Pavement Mean Air Void content for all Superpave and Stone Matrix Asphalt mixtures is 5.0 percent. Ensure that the maximum Pavement Mean Air Voids for all Superpave and Stone Matrix Asphalt mixtures does not exceed 7.0 percent. The maximum Pavement Mean Air Voids for 2 foot shoulder widening is 9.0 percent. The adjustment period for density shall be four lots or four production days, whichever is less, in order for the contractor to ensure maximum compactive effort has been achieved which will yield no more than the specified maximum allowed Mean Air Voids. If the contractor needs to adjust the mixture to improve density results, a change in the job mix formula may be requested for approval during the adjustment period so long as the following values are not exceeded:

- Coarse pay sieve ± 4%
- No. 8 (2.36 mm) sieve ± 2%
- No. 200 (75 µm) sieve ± 1%
- Asphalt Content ± 0.2%

All value changes must still be within specification limits.

If the Office of Materials and Research is satisfied that the contractor has exerted the maximum compactive effort and is not able to maintain Pavement Mean Air Voids at no more than 7.0%, the Engineer may establish a maximum target for Pavement Mean Air Voids.

Mixture placed during the adjustment period for density shall meet the requirements for a 0.90 pay factor in Table 12 of Subsection 400.5.01.C, “Calculate Mean Pavement Air Voids.” Mixture which does not meet these density requirements shall be paid for using the applicable pay factor.

If the mean air voids of the pavement placed within a lot exceeds 100% of the maximum target air voids, if established and the Engineer determines that the material need not be removed and replaced, the lot may be accepted at an adjusted unit price as determined by the Engineer.

2. Obtain Uniform Compaction
For a lot to receive a pay factor of 1.00 for compaction acceptance, the air void range cannot exceed 4 percent for new construction or 5 percent for resurfacing projects. The range is the difference between the highest and lowest acceptance test results within the affected lot. If the air void range exceeds these tolerances, apply a Pay Factor of 95%.

The 5% reduced pay factor for the compaction range does not apply in these instances:

- The mixture is placed during the adjustment period as defined in Subsection 400.5.01.A, “Materials Produced and Placed During the Adjustment Period.”
- All air void results within a given lot are less than 7.0%.
- A lot containing two sublot or less.
- On two foot trench widening.

C. Surface Tolerance

In this Specification, pavement courses to be overlaid with an Open-Graded Friction Course or PEM are considered surface courses. All Open-Graded Friction Courses or PEM are to be evaluated after the roadway has been opened to traffic for a minimum of 5 days and a maximum of 15 days. Other asphalt paving is subject to straightedge and visual inspection and irregularity correction as shown below:

1. Visual and Straightedge Inspection

Paving is subject to visual and straightedge inspection during and after construction operations until Final Acceptance. Locate surface irregularities as follows:

a. Keep a 10 ft (3 m) straightedge near the paving operation to measure surface irregularities on courses. Provide the straightedge and the labor for its use.

b. Inspect the base, intermediate, and surface course surfaces with the straightedge to detect irregularities.

c. Correct irregularities that exceed 3/16 in. in 10 ft (5 mm in 3 m) for base and intermediate courses, and 1/8 in. in 10 ft (3 mm in 3 m) for surface courses.

Mixture or operating techniques will be stopped if irregularities such as rippling, tearing, or pulling occur and the Engineer suspects a continuing equipment problem. Stop the paving operation and correct the problem. Correct surface course evaluations on individual Laser Road Profiler test sections, normally 1 mile (1 km) long.

2. Target Surface Smoothness

The Department will use the Laser Road Profiler method to conduct acceptance testing for surface course tolerance according to GDT 126. This testing will be performed only on:

- Surface courses on Projects with mainline traveled way measuring a minimum distance of 1 mile
- Ramps more than 0.5 mile (800 m) long

Achieve the smoothest possible ride during construction. Do not exceed the target Laser Road Profiler smoothness index as shown below:

<table>
<thead>
<tr>
<th>Construction Description</th>
<th>Smoothness Index</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphaltic concrete OGFC and PEM on interstates and asphaltic concrete OGFC and PEM on new construction on state routes</td>
<td>750</td>
</tr>
<tr>
<td>Asphaltic Concrete SMA and other resurfacing on interstates, asphaltic concrete OGFC and PEM resurfacing on state routes, and new construction</td>
<td>825</td>
</tr>
<tr>
<td>All other resurfacing on state routes (excluding LARP, PR, airports, etc.)</td>
<td>900</td>
</tr>
</tbody>
</table>

If the target values are not achieved, immediately adjust the operations to meet the target values.

Corrective work is required if the surface smoothness exceeds the Laser Road Profiler smoothness index shown below:
Table 8—Pavement Smoothness Requirements—Corrective Work

<table>
<thead>
<tr>
<th>Construction Description</th>
<th>Smoothness Index</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphaltic concrete OGFC and PEM on interstates and asphaltic concrete OGFC and PEM on new construction on state routes</td>
<td>825</td>
</tr>
<tr>
<td>Asphaltic Concrete SMA and other resurfacing on interstates, asphaltic concrete OGFC and PEM resurfacing on state routes, and new construction</td>
<td>900</td>
</tr>
<tr>
<td>All other resurfacing on state routes (excluding LARP, PR, airports, etc.)</td>
<td>1025</td>
</tr>
</tbody>
</table>

If surface tolerance deficiencies need correction, obtain the Engineer’s approval of the methods and type mix used.

3. Bridge Approach Ride Quality

The following are subject to a ride quality test by the Department for 100 ft. (30 m) of roadway approaching each end of a bridge using the Lightweight Profiler:

- A state road with 4 lanes or more
- A 2-lane state road with a current traffic count of 2,000 vpd or more
- Locations designated on the Plans

All other bridge approaches shall meet the 1/8 in. in 10 ft (3 mm in 3 m) straightedge requirement. Test ride quality as follows:

a. The Department will determine a profile index value according to test method GDT 134.

b. The Department will average the profile index value from the right and left wheelpath for each 100 ft (30 m) section for each lane. Keep the profile index value under 30 in/mile (475 mm/km).

c. Meet the profile index value for the 100 ft (30 m) section of roadway up to the joint with the approach slab.

d. Schedule the ride quality testing 5 days before needed by contacting the Office of Materials and Research. Clean and clear obstructions from the test area.

e. Correct the sections that do not meet the ride quality criteria of this Specification. After correction, these sections are subject to retesting with the Lightweight Profiler. The Engineer shall direct the type of correction method, which may include:

- Milling
- Grinding
- Removing and replacing the roadway

No additional compensation will be made.

The Department will perform ride quality testing up to two times on the bridge approaches at no cost to the Contractor. Additional profilograph testing will cost the Contractor $500 per test.

D. Reevaluation of Lots

When lots are reevaluated as shown in Subsection 106.03, “Samples, Tests, Cited Specifications,” sampling and testing is according to GDT 73. Request for reevaluation shall be made within 5 working days of notification of the lot results. The following procedures apply:

1. Mixture Acceptance

   The Department will take the same number of new tests on cores taken at a location where the load sampled was placed and will use only those core results for acceptance.

   The Department will use the mean of the deviations from the job mix formula for these tests to determine acceptance based on the appropriate column in the Asphalt Cement Content and Aggregate Gradation of Asphalt Concrete Mixture Acceptance Schedule—Table 9 or 10.

2. Compaction Acceptance

   The Department will reevaluate the lot through additional testing by cutting the same number of cores originally obtained and averaging these results with the results from the original density tests. The Department will use the
average to determine acceptance according to the Compaction Acceptance Schedule in Subsection 400.5.01.C, “Calculate Pavement Mean Air Voids”.
<table>
<thead>
<tr>
<th>Mixture Characteristics</th>
<th>Pay Factor</th>
<th>Mean of the Deviations from the Job Mix Formula</th>
</tr>
</thead>
<tbody>
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<td></td>
<td></td>
<td>1 Test</td>
</tr>
<tr>
<td><strong>Asphalt Cement Content</strong></td>
<td></td>
<td></td>
</tr>
<tr>
<td>(Extraction, Ignition)</td>
<td></td>
<td>1.00</td>
</tr>
<tr>
<td></td>
<td></td>
<td>0.95</td>
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<tr>
<td></td>
<td></td>
<td>0.90</td>
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<td></td>
<td></td>
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<tr>
<td></td>
<td></td>
<td>0.70</td>
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<tr>
<td></td>
<td></td>
<td>0.50</td>
</tr>
<tr>
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<td>1.00</td>
</tr>
<tr>
<td>(12.5 mm OGFC, 12.5 mm PEM, 12.5 mm Superpave)</td>
<td></td>
<td>0.98</td>
</tr>
<tr>
<td></td>
<td></td>
<td>0.95</td>
</tr>
<tr>
<td></td>
<td></td>
<td>0.90</td>
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<tr>
<td></td>
<td></td>
<td>0.85</td>
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<td></td>
<td></td>
<td>0.80</td>
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<tr>
<td><strong>3/8 in. (9.5 mm) Sieve</strong></td>
<td></td>
<td>1.00</td>
</tr>
<tr>
<td>(12.5 mm SMA)</td>
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<td>0.98</td>
</tr>
<tr>
<td></td>
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<td>0.95</td>
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</tr>
<tr>
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<td>1 Test</td>
</tr>
<tr>
<td>-------------------------</td>
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<td>(OGFC, PEM, Superpave</td>
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<tr>
<td>and 4.75 mm mixes)</td>
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No. 8 (2.36 mm) Sieve for OGFC and PEM mixes: When the mean of the deviations from the Job Mix Formula for a particular lot exceeds the tolerance for a 1.00 pay factor in the appropriate column, the lot will be paid for at 0.50 of the Contract Price.
<table>
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<tr>
<th>Mixture Characteristics</th>
<th>Pay Factor</th>
<th>1 Test</th>
<th>2 Tests</th>
<th>3 Tests</th>
<th>4 Tests</th>
<th>5 Tests</th>
<th>6 Tests</th>
<th>7 Tests</th>
<th>8 Tests</th>
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<td>0.49 - 0.52</td>
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<tr>
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<td>0.00 - 6.0</td>
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<td>0.00 - 4.1</td>
<td>0.00 - 3.8</td>
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<td>0.00 - 3.3</td>
</tr>
<tr>
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<td>6.2 - 6.8</td>
<td>5.3 - 5.8</td>
<td>4.7 - 5.1</td>
<td>4.2 - 4.6</td>
<td>3.9 - 4.2</td>
<td>3.6 - 3.9</td>
<td>3.4 - 3.7</td>
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<tr>
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<td>6.9 - 7.8</td>
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<td>4.4 - 4.7</td>
<td>4.1 - 4.4</td>
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<td>5.4 - 5.5</td>
<td>5.0 - 5.1</td>
<td>4.7 - 4.8</td>
</tr>
<tr>
<td>3/8 in. (9.5 mm) Sieve</td>
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<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>(19 mm Superpave, 12.5</td>
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<td>0.00 - 10.0</td>
<td>0.00 - 7.5</td>
<td>0.00 - 6.3</td>
<td>0.00 - 5.6</td>
<td>0.00 - 5.2</td>
<td>0.00 - 4.7</td>
<td>0.00 - 4.4</td>
<td>0.00 - 4.1</td>
</tr>
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<td>mm Superpave)</td>
<td>0.98</td>
<td>10.1 - 11.9</td>
<td>7.6 - 8.4</td>
<td>6.4 - 7.0</td>
<td>5.7 - 6.3</td>
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<td>4.5 - 5.0</td>
<td>4.2 - 4.6</td>
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<td>5.6 - 6.0</td>
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<td>6.9 - 7.3</td>
<td>6.5 - 6.8</td>
<td>6.1 - 6.5</td>
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<tr>
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<td>0.00 - 5.4</td>
<td>0.00 - 4.9</td>
<td>0.00 - 4.6</td>
<td>0.00 - 4.3</td>
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</table>
## Mixture Characteristics Pay Factor

### Mean of the Deviations from the Job Mix Formula

<table>
<thead>
<tr>
<th>Mixture Characteristics</th>
<th>Pay Factor</th>
<th>Mean of the Deviations from the Job Mix Formula</th>
<th>1 Test</th>
<th>2 Tests</th>
<th>3 Tests</th>
<th>4 Tests</th>
<th>5 Tests</th>
<th>6 Tests</th>
<th>7 Tests</th>
<th>8 Tests</th>
</tr>
</thead>
<tbody>
<tr>
<td>(9.5 mm Superpave)</td>
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<td>10.1 - 11.9</td>
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<td>6.5 - 6.8</td>
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<td></td>
<td></td>
</tr>
<tr>
<td>(All mixes except SMA)</td>
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<td></td>
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<tr>
<td>1.00</td>
<td>0.00 - 8.0</td>
<td>0.00 - 6.3</td>
<td>0.00 - 5.4</td>
<td>0.00 - 4.8</td>
<td>0.00 - 4.5</td>
<td>0.00 - 4.1</td>
<td>0.00 - 3.8</td>
<td>0.00 - 3.6</td>
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<tr>
<td>0.98</td>
<td>8.1 - 9.0</td>
<td>6.4 - 7.0</td>
<td>5.5 - 6.0</td>
<td>4.9 - 5.3</td>
<td>4.6 - 4.9</td>
<td>4.2 - 4.5</td>
<td>3.9 - 4.2</td>
<td>3.7 - 3.9</td>
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<tr>
<td>0.95</td>
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<td>7.1 - 7.7</td>
<td>6.1 - 6.6</td>
<td>5.4 - 5.8</td>
<td>5.0 - 5.4</td>
<td>4.6 - 4.9</td>
<td>4.3 - 4.6</td>
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<td>0.90</td>
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<td>7.8 - 8.5</td>
<td>6.7 - 7.2</td>
<td>5.9 - 6.4</td>
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<td>5.0 - 5.3</td>
<td>4.7 - 5.0</td>
<td>4.4 - 4.6</td>
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<td>7.3 - 7.5</td>
<td>6.5 - 6.8</td>
<td>5.9 - 6.3</td>
<td>5.4 - 5.7</td>
<td>5.1 - 5.3</td>
<td>4.7 - 4.9</td>
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<td>8.9 - 9.1</td>
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<td>6.9 - 7.2</td>
<td>6.4 - 6.6</td>
<td>5.8 - 6.1</td>
<td>5.4 - 5.7</td>
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<tr>
<td>No. 8 (2.36 mm) Sieve</td>
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<td>(19 mm SMA)</td>
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<td>1.00</td>
<td>0.00 - 6.0</td>
<td>0.00 - 4.7</td>
<td>0.00 - 4.1</td>
<td>0.00 - 3.6</td>
<td>0.00 - 3.4</td>
<td>0.00 - 3.1</td>
<td>0.00 - 2.9</td>
<td>0.00 - 2.4</td>
<td></td>
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</tr>
<tr>
<td>0.98</td>
<td>6.1 - 6.8</td>
<td>4.8 - 5.2</td>
<td>4.2 - 4.5</td>
<td>3.7 - 4.0</td>
<td>3.5 - 3.7</td>
<td>3.2 - 3.4</td>
<td>3.0 - 3.2</td>
<td>2.8 - 2.9</td>
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<tr>
<td>0.95</td>
<td>6.9 - 7.5</td>
<td>5.3 - 5.8</td>
<td>4.6 - 5.0</td>
<td>4.1 - 4.4</td>
<td>3.8 - 4.0</td>
<td>3.5 - 3.7</td>
<td>3.3 - 3.5</td>
<td>3.0 - 3.2</td>
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<tr>
<td>0.90</td>
<td>7.6 - 8.9</td>
<td>5.9 - 6.4</td>
<td>5.1 - 5.4</td>
<td>4.5 - 4.8</td>
<td>4.1 - 4.4</td>
<td>3.8 - 4.0</td>
<td>3.6 - 3.8</td>
<td>3.3 - 3.5</td>
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<tr>
<td>0.85</td>
<td>9.0 - 9.8</td>
<td>6.5 - 6.6</td>
<td>5.5 - 5.6</td>
<td>4.9 - 5.1</td>
<td>4.5 - 4.7</td>
<td>4.1 - 4.3</td>
<td>3.9 - 4.0</td>
<td>3.6 - 3.7</td>
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</tr>
<tr>
<td>0.75</td>
<td>9.9 - 10.5</td>
<td>6.7 - 6.8</td>
<td>5.7 - 5.9</td>
<td>5.2 - 5.4</td>
<td>4.8 - 5.0</td>
<td>4.4 - 4.6</td>
<td>4.1 - 4.3</td>
<td>3.8 - 4.0</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>
E. Segregated Mixture

Prevent mixture placement that yields a segregated mat by following production, storage, loading, placing, and handling procedures. Also, make needed plant modifications and provide necessary auxiliary equipment. (See Subsection 400.1.01, “Definitions.”)

If the mixture is segregated in the finished mat, the Department will take actions based on the degree of segregation. The actions are described below.

1. Unquestionably Unacceptable Segregation

   When the Engineer determines that the segregation in the finished mat is unquestionably unacceptable, follow these measures:
   a. Suspend Work and require the Contractor to take positive corrective action. The Department will evaluate the segregated areas to determine the extent of the corrective work to the in-place mat as follows:
      • Perform extraction and gradation analysis by taking 6 in (150 mm) cores from typical, visually unacceptable segregated areas.
      • Determine the corrective work according to Subsection 400.3.06.E.3.
   b. Require the Contractor to submit a written plan of measures and actions to prevent further segregation. Work will not continue until the plan is submitted to and approved by the Department.
   c. When work resumes, place a test section not to exceed 500 tons (500 Mg) of the affected mixture for the Department to evaluate. If a few loads show that corrective actions were not adequate, follow the measures above beginning with step 1.a. above. If the problem is solved, Work may continue.

2. Unacceptable Segregation Suspected

   When the Engineer observes segregation in the finished mat and suspects that it may be unacceptable, follow these measures:
   a. Allow work to continue at Contractor’s risk.
   b. Require Contractor to immediately and continually adjust operation until the visually apparent segregated areas are eliminated from the finished mat. The Department will immediately investigate to determine the severity of the apparent segregation as follows:
      • Take 6 in (150 mm) cores from typical areas of suspect segregation.
      • Test the cores for compliance with the mixture control tolerances in Section 828.
   When these tolerances are exceeded, suspend work for corrective action as outlined in Subsection 400.3.06.E.3.

3. Corrective Work

   a. Remove and replace (at the Contractor’s expense) any segregated area where the gradation on the control sieves is found to vary 10 percent or more from the approved job mix formula, the asphalt cement varies 1.0% or more from the approved job mix formula, or if in-place air voids exceed 13.5% based on GDT 39. The control sieves for each mix type are shown in Subsection 400.5.01.B “Determine Lot Acceptance.”
   b. Subsurface mixes. For subsurface mixes, limit removal and replacement to the full lane width and no less than 10 ft. (3 m) long and as approved by the Engineer.
   c. Surface Mixes. For surface mixes, ensure that removal and replacement is not less than the full width of the affected lane and no less than the length of the affected areas as determined by the Engineer.
   Surface tolerance requirements apply to the corrected areas for both subsurface and surface mixes.

400.3.07 Contractor Warranty and Maintenance

A. Contractor’s Record

Maintain a dated, written record of the most recent plant calibration. Keep this record available for the Engineer’s inspection at all times. Maintain records in the form of:

- Graphs
- Tables
- Charts
- Mechanically prepared data
400.4 Measurement

Thickness and spread rate tolerances for the various mixtures are specified in Subsection 400.4.A.2.b, Table 11, Thickness and Spread Rate Tolerance at Any Given Location. These tolerances are applied as outlined below:

A. Hot Mix Asphaltic Concrete Paid for by Weight

1. Plans Designate a Spread Rate
   a. Thickness Determinations. Thickness determinations are not required when the Plans designate a spread rate per square yard (meter).
      - If the spread rate exceeds the upper limits outlined in the Subsection 400.4.A.2.b, Table 11, “Thickness and Spread Rate Tolerance at Any Given Location”, the mix in excess will not be paid for.
      - If the rate of spread is less than the lower limit, correct the deficient course by overlaying the entire lot.
      - The mixture used for correcting deficient areas is paid for at the Contract Unit Price of the course being corrected and is subject to the Mixture Acceptance Schedule—Table 9 or 10.
   b. Recalculate the Total Spread Rate. After the deficient hot mix course has been corrected, the total spread rate for that lot is recalculated, and mix in excess of the upper tolerance limit as outlined in the Subsection 400.4.A.2.b, Table 11, “Thickness and Spread Rate Tolerance at Any Given Location” is not paid for.
      - The quantity of material placed on irregular areas such as driveways, turnouts, intersections, feather edge section, etc., is deducted from the final spread determination for each lot.

2. Plans Designate Thickness
   If the average thickness exceeds the tolerances specified in the Subsection 400.4.A.2.b, Table 11, “Thickness and Spread Rate Tolerance at Any Given Location”, the Engineer shall take cores to determine the area of excess thickness. Excess quantity will not be paid for.
   - If the average thickness is deficient by more than the tolerances specified in the Thickness and Spread Rate Tolerance at Any Given Location table below, the Engineer shall take additional cores to determine the area of deficient thickness. Correct areas with thickness deficiencies as follows:
     a. Overlay the deficient area with the same mixture type being corrected or with an approved surface mixture.
        - The overlay shall extend for a minimum of 300 ft (90 m) for the full width of the course.
     b. Ensure that the corrected surface course complies with Subsection 400.3.06.C.1, “Visual and Straightedge Inspection.” The mixture required to correct a deficient area is paid for at the Contract Unit Price of the course being corrected.
       - The mixture is subject to the Mixture Acceptance Schedule—Table 9 or 10. The quantity of the additional mixture shall not exceed the required calculated quantity used to increase the average thickness of the overlaid section to the maximum tolerance allowed under the following table.

   Table 11—Thickness and Spread Rate Tolerance at Any Given Location

<table>
<thead>
<tr>
<th>Course</th>
<th>Thickness Specified</th>
<th>Spread Rate Specified</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphaltic concrete base course</td>
<td>± 0.5 in (±13 mm)</td>
<td>+40 lbs, -50 lbs (+20 kg, -30 kg)</td>
</tr>
<tr>
<td>Intermediate and/or wearing course</td>
<td>± 0.25 in (± 6 mm)</td>
<td>+20 lbs, -25 lbs (+10 kg, -15 kg)</td>
</tr>
<tr>
<td>Overall of any combination of 1 and 2</td>
<td>± 0.5 in (±13 mm)</td>
<td>+40 lbs, -50 lbs (+20 kg, -30 kg)</td>
</tr>
</tbody>
</table>

Note 1: For asphaltic concrete 9.5 mm OGFC and 12.5 mm OGFC, control the spread rate per lot within 5 lbs/yd² (3 kg/m²) of the designated spread rate. For asphaltic concrete 12.5 mm PEM, control the spread rate per lot within 10 lbs/yd² (6 kg/m²) of the designated spread rate.

Note 2: Thickness and spread rate tolerances are provided to allow normal variations within a given lot. Do not continuously operate at a thickness of spread rate not specified.
When the Plans specify a thickness, the Engineer may take as many cores as necessary to determine the average thickness of the intermediate or surface course. The Engineer shall take a minimum of one core per 1,000 ft (300 m) per two lanes of roadway. Thickness will be determined by average measurements of each core according to GDT 42.

If the average exceeds the tolerances specified in the Subsection 400.4.A.2.b, Table 11, “Thickness and Spread Rate Tolerance at Any Given Location”, additional cores will be taken to determine the area of excess thickness and excess tonnage will not be paid for.

B. Hot Mix Asphaltic Concrete Paid for by Square Yard (Meter)

1. The thickness of the base course or the intermediate or surface course will be determined by the Department by cutting cores and the thickness will be determined by averaging the measurements of each core.

2. If any measurement is deficient in thickness more than the tolerances given in the table above, additional cores will be taken by the Department to determine the area of thickness deficiency. Correct thickness deficiency areas as follows:
   a. Overlay the deficient area with the same type mixtures being corrected or with surface mixture. Extend the overlay at least 300 ft (90 m) for the full width of the course.
   b. Ensure that the corrected surface course complies with Subsection 400.3.06.C.1, Visual and Straightedge Inspection.
   c. The mixture is subject to the Mixture Acceptance Schedule—Table 9 or 10.

3. No extra payment is made for mixtures used for correction.

4. No extra payment is made for thickness in excess of that specified.

**NOTE:** Thickness tolerances are provided to allow normal variations within a given lot. Do not continuously operate at a thickness not specified.

C. Asphaltic Concrete

Hot mix asphaltic concrete, complete in place and accepted, is measured in tons (megagrams) or square yards (meters) as indicated in the Proposal. If payment is by the ton (megagram), the actual weight is determined by weighing each loaded vehicle on the required motor truck scale as the material is hauled to the roadway, or by using recorded weights if a digital recording device is used.

The weight measured includes all materials. No deductions are made for the weight of the individual ingredients. The actual weight is the pay weight except when the aggregates used have a combined bulk specific gravity greater than 2.75. In this case the pay weight is determined according to the following formula:

\[
T_1 = T \times \left( \frac{\% \text{ AC} + \left( \frac{\% \text{ Aggregate} \times 2.75}{\text{combined bulk Specific Gravity}} \right)}{100} \right) + \% Y
\]

Where:

<table>
<thead>
<tr>
<th>T1</th>
<th>Pay weight, tonnage (Mg)</th>
</tr>
</thead>
<tbody>
<tr>
<td>T</td>
<td>Actual weight</td>
</tr>
<tr>
<td>% AC</td>
<td>Percent asphalt cement by weight of total mixture</td>
</tr>
<tr>
<td>% Aggregate</td>
<td>Percent aggregate by weight of total mixture</td>
</tr>
</tbody>
</table>
### Section 400—Hot Mix Asphaltic Concrete Construction

<table>
<thead>
<tr>
<th>Combined Bulk Sp. Gr.</th>
<th>Calculated combined bulk specific gravity of various mineral aggregates used in the mixture</th>
</tr>
</thead>
<tbody>
<tr>
<td>% Y</td>
<td>Percent hydrated lime by weight of mineral aggregate</td>
</tr>
</tbody>
</table>

D. **Bituminous Material**  
Bituminous material is not measured for separate payment.

E. **Hydrated Lime**  
When hydrated lime is used as an anti-stripping additive, it is not measured for separate payment.

F. **Field Laboratory**  
The field laboratory required in this Specification is not measured for separate payment.

G. **Asphaltic Concrete Leveling**  
Payment of hot mix asphaltic concrete leveling, regardless of the type mix, is full compensation for furnishing materials, bituminous materials, and hydrated lime (when required) for patching and repair of minor defects, surface preparation, cleaning, hauling, mixing, spreading, and rolling.

Mixture for leveling courses is subject to the acceptance schedule as stated in Subsection 400.3.06.A and Subsection 400.3.06.B.

H. **Asphaltic Concrete Patching**  
Hot mix asphaltic concrete patching, regardless of the type mix, is paid for at the Contract Unit Price per ton (Megagram), complete in place and accepted. Payment is full compensation for:

- Furnishing materials such as bituminous material and hydrated lime (when required)
- Preparing surface to be patched
- Cutting areas to be patched, trimmed, and cleaned
- Hauling, mixing, placing, and compacting the materials

### 400.4.01 Limits
When the asphaltic concrete is paid for by the square yard (meter) and multiple lifts are used, the number and thickness of the lifts are subject to the Engineer’s approval and are used to prorate the pay factor for the affected roadway section.

### 400.5 Payment
When materials or construction are not within the tolerances in this Specification, the Contract Price will be adjusted according to Subsection 106.03, “Samples, Tests, Cited Specifications” and Subsection 400.3.06, “Quality Acceptance.”

Hot mix asphaltic concrete of the various types are paid for at the Contract Unit Price per ton (megagram) or per square yard (meter). Payment is full compensation for furnishing and placing materials including asphalt cement, hydrated lime when required, approved additives, and for cleaning and repairing, preparing surfaces, hauling, mixing, spreading, rolling, and performing other operations to complete the Contract Item.

Payment will be made under: CONSTRUCTION COMPLETE

### 400.5.01 Adjustments
A. **Materials Produced and Placed During the Adjustment Period**  
An adjustment period is allowed at the start of mixing operations for each type of mix placed on the Contract except for Asphaltic Concrete OGFC or PEM. The adjustment period is provided to adjust or correct the mix and to establish the construction procedures and sequence of operations.
The adjustment period consists of the tons (megagrams) of the affected mix produced and placed on the first day of operation. If this quantity is less than 500 tons (500 Mg), the Engineer may combine the tons (megagrams) produced and placed on the first day of operation with the tons (megagrams) produced and placed on the next production day of the affected mix for the adjustment period.

The material produced and placed during the mixture adjustment period is one lot. If the mix is adjusted during this period, a new lot may be necessary, but a new adjustment period will not be permitted.

This material shall be paid for at 100 percent of the Contract Unit Price provided it meets the minimum requirements for a 1.00 pay factor for asphalt cement content and a 0.90 pay factor for gradation in the Mixture Acceptance Schedule—Table 9 or 10.

If the material placed during the adjustment period fails to meet the above requirements, it will be paid for using the applicable acceptance schedule. However, when mixture used for leveling at a spread rate of 90 lbs/yd² (50 kg/m²) or less is also used for the surface mix at a spread rate greater than 90 lbs/yd² (50 kg/m²), an additional adjustment period will be allowed for compaction only. This material will be paid for at a 1.00 pay factor provided it:

- Meets the minimum requirements for a 1.00 pay factor in the Mixture Acceptance Schedule—Table 9 or 10 for both asphalt content and gradation.
- Meets the minimum requirements for a 0.90 pay factor in Table 12 of Subsection 400.5.01C, “Calculate Mean Pavement Air Voids.”

Mixture which does not meet these requirements shall be paid for using the applicable acceptance schedule.

**B. Determine Lot Acceptance**

Pay factor adjustments are based on control sieves and asphalt cement content. The control sieves used in the mixture acceptance schedule for the various types of mix are indicated below:

<table>
<thead>
<tr>
<th>Control Sieves Used in the Mixture Acceptance Schedule</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphaltic concrete 25 mm Superpave 1/2 in., No. 8 (12.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 19 mm SMA 1/2 in., No. 8 (12.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 19 mm Superpave 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 12.5 mm Superpave 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 12.5 mm SMA 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 12.5 mm PEM 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 12.5 mm OGFC 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 9.5 mm Superpave No. 4, No. 8 (4.75 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 9.5 mm SMA No. 4, No. 8 (4.75 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 9.5 mm OGFC No. 4, No. 8 (4.75 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphaltic concrete 4.75 mm Mix No. 8 (2.36 mm) sieve and asphalt cement</td>
</tr>
</tbody>
</table>

For projects which do not have milling quantities established as a Pay Item, the Department will pay for 12.5 mm OGFC and PEM placed on ramps and end of project transitions under the appropriate mixture pay item, but the mix shall be subject to the same gradation and control sieve requirements as asphaltic concrete 9.5 mm OGFC. Add polymer-modified bituminous material, hydrated lime, and stabilizing fiber to this mix.

The Department will perform the following tasks:

1. Using the Mixture Acceptance Schedule—Table 9 or 10, determine the mean of the deviations from the job mix formula per test results per lot.
2. Determine this mean by averaging the actual numeric value of the individual deviations from the job mix formula; disregard whether the deviations are positive or negative amounts.
3. Use the Asphalt Cement Content and Aggregate Gradation of Asphalt Concrete Mixture Acceptance Schedule—Table 9 to determine acceptance of surface mixes and the Mixture Acceptance Schedule—Table 10 to determine acceptance of subsurface mixes.

On Contracts involving 1,000 tons (1000 Mg) or less of asphaltic concrete, the mixture is accepted for 100 percent payment of the asphaltic concrete Unit Price provided it meets the following:

1. Minimum requirements for a 1.00 pay factor for asphalt cement content and a 0.90 pay factor for gradation in the applicable Mixture Acceptance Schedule—Table 9 or 10.
2. Minimum requirements for a 0.90 pay factor in Table 12 of Subsection 400.5.01C, “Calculate Pavement Mean Air Voids.”

If the material placed on Contracts involving 1,000 tons (1000 Mg) or less of asphaltic concrete does not meet the above requirements, the material will be paid for using the applicable acceptance schedule.

C. Calculate Pavement Mean Air Voids

The Department will determine the percent of maximum air voids for each lot by dividing the pavement mean air voids by the maximum pavement mean air voids acceptable.

The Department will determine the payment for each lot by multiplying the Contract Unit Price by the adjusted pay factor shown in the following Air Voids Acceptance schedule:

<table>
<thead>
<tr>
<th>Pay Factor</th>
<th>Percent of Maximum Air Voids (Lot Average of Tests)</th>
<th>Percent of Maximum Air Voids (Lot Average all Tests) (for Revaluations)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.00</td>
<td>≤100</td>
<td>≤100</td>
</tr>
<tr>
<td>0.97</td>
<td>100.1 — 105</td>
<td>100.1 — 104</td>
</tr>
<tr>
<td>0.95</td>
<td>105.1 — 112</td>
<td>104.1 — 109</td>
</tr>
<tr>
<td>0.90</td>
<td>112.1 — 124</td>
<td>109.1 — 118</td>
</tr>
<tr>
<td>0.80</td>
<td>124.1 — 149</td>
<td>118.1 — 136</td>
</tr>
<tr>
<td>0.70</td>
<td>149.1 — 172</td>
<td>136.1 — 153</td>
</tr>
<tr>
<td>0.50</td>
<td>172.1 — 191</td>
<td>153.1 — 166</td>
</tr>
</tbody>
</table>

When the range tolerance is exceeded, the Department will apply a pay factor of 0.95 as described in Subsection 400.3.06.B.2.

D. Asphaltic Concrete For Temporary Detours

Hot mix asphaltic concrete placed on temporary detours that will not remain in place as part of the permanent pavement does not require hydrated lime. Hot mix used for this purpose is paid for at an adjusted Contract Price.

Where the Contract Price of the asphaltic concrete for permanent pavement is let by the ton (megagram), the Contract Price for the asphaltic concrete placed on temporary detours is adjusted by subtracting $0.75/ton ($0.85/mg) of mix used.

Where the Contract price of the mix in the permanent pavement is based on the square yard (meter), obtain the adjusted price for the same mix used on the temporary detour by subtracting $0.04/yd² ($0.05/ m²) per 1-in (25-mm) plan depth.

Further price adjustments required in Subsection 400.3.06, “Quality Acceptance,” are based on the appropriate adjusted Contract Price for mix used in the temporary detour work.

E. Determine Lot Payment

Determine the lot payment as follows:

1. When one of the pay factors for a specific acceptance lot is less than 1.0, determine the payment for the lot by multiplying the Contract Unit Price by the adjusted pay factor.
2. When two or more pay factors for a specific acceptance lot are less than 1.0, determine the adjusted payment by multiplying the Contract Unit Price by the lowest pay factor.
If the mean of the deviations from the job mix formula of the tests for a sieve or asphalt cement content exceeds the tolerances established in the Mixture Acceptance Schedule—Table 9 or 10 and if the Engineer determines that the material need not be removed and replaced, the lot may be accepted at an adjusted unit price as determined by the Engineer. If the pavement mean air voids exceed the tolerances established in the Air Voids Acceptance Schedule—
Table 12, remove and replace the materials at the Contractor’s expense.

If the Engineer determines that the material is not acceptable to leave in place, remove and replace the materials at the Contractor’s expense.
9. Crumb Rubber Modifier Supply System

Crumb rubber modifier may be substituted at the Contractor’s discretion to produce a PG 76-22 asphaltic cement at the production facility:

a. Use a separate feed system to store and proportion by weight of the total asphaltic cement, the required percentage of crumb rubber into the mixture.

b. Control the feeder system with a proportioning device meeting these Specifications:
   - Is accurate to within ± 5 percent of the amount required. Automatically adjusts the feed rate to maintain the material within this tolerance at all times.
   - Has a convenient and accurate means of calibration.
   - Provide in-process monitoring, consisting of either a digital display of output or a printout of feed rate, in pounds per minute, to verify feed rate. The supply system shall report the feed in 1 lb increments using load cells that will enable the user to monitor the depletion of the modifier. Monitoring the system volumetrically will not be allowed.
   - Interlocks with the aggregate weigh system and asphaltic cement pump to maintain the correct proportions for all rates of production and batch sizes.

c. Provide flow indicators or sensing devices for the system and interlock them with the plant controls to interrupt the mixture production if the crumb rubber introduction output rate is not within the ± 5 percent tolerance given above. This interlock will immediately notify the operator if the targeted rate exceeds introduction tolerances. All plant production will cease if the incorporation rate is not brought back within tolerance after 30 seconds. When the interlock system interrupts production and the plant has to be restarted, upon restarting operations; the modifier system shall run until a uniform feed can be observed on the output display. All mix produced prior to this point shall be rejected.

d. Introduce the crumb rubber modifier as follows:
   - When a batch type plant is used, add the rubber to the aggregate in the weigh hopper. Increase the batch dry mixing time by 15 to 20 seconds from the time the aggregate is completely emptied into the mixer to ensure the modifiers are uniformly distributed prior to the injection of asphalt cement into the mixer. Increase the batch wet mix time by 15 to 20 seconds to ensure the crumb rubber modifier is uniformly blended with the asphaltic cement.
• When a continuous or drier-drum type plant is used, add the rubber to the aggregate and uniformly disperse prior to the injection of asphalt cement. The point of introduction in the drum mixer will be approved by the Engineer prior to production. Ensure the crumb rubber modifier will not become entrained in the exhaust system of the drier or plant and will not be exposed to the drier flame at any point after induction.

e. No separate measurement and payment will be made if Contractor elects to utilize crumb rubber.
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  

Section 402—Hot Mix Recycled Asphaltic Concrete 

Delete Subsection 402.5 and substitute the following:

402.5 Payment  
The work performed and the materials furnished as described in this Specification will not be measured separately.

Payment will be made under:

Work and materials will be paid for under CONSTRUCTION COMPLETE. Includes providing materials, hauling and necessary crushing, processing, placing, rolling and finishing the recycled mixture, and providing labor, tools, equipment, and incidentals necessary to complete the work, including hauling and stockpiling RAP or RAS material.

402.5.01 Adjustments  
A. Materials Produced and Placed During the Adjustment Period  

An adjustment period is allowed at the start of mixing operations for each type of mix placed on the Contract. The adjustment period is provided to adjust or correct the mix and to establish the construction procedures and sequence of operations.

The adjustment period consists of the tons (megagrams) of the affected mix produced and placed on the first day of operation. If this quantity is less than 500 tons (500 Mg), the Engineer may combine the tons (megagrams) produced and placed on the first day of operation with the tons (megagrams) produced and placed on the next production day of the affected mix for the adjustment period.

The material produced and placed during the mixture adjustment period is one lot. If the mix is adjusted during this period, a new lot may be necessary, but a new adjustment period will not be permitted.

This material shall be paid for at 100 percent of the Contract Unit Price provided it meets the minimum requirements for a 1.00 pay factor for asphalt cement content and a 0.90 pay factor for gradation in the Mixture Acceptance Schedule—Table 9 or 10 of Subsection 400.3.06.

If the material placed during the adjustment period fails to meet the above requirements, it will be paid for using the applicable acceptance schedule. However, when mixture used for leveling at a spread rate of 90 lbs/yd² (50 kg/m²) or less is also used for the surface mix at a spread rate greater than 90 lbs/yd² (50 kg/m²), an additional adjustment period will be allowed for compaction only. This material will be paid for at a 1.00 pay factor provided it:

Meets the minimum requirements for a 1.00 pay factor in the Mixture Acceptance Schedule—Table 9 or 10 of Subsection 400.3.06 for both asphalt content and gradation.
Meets the minimum requirements for a 0.90 pay factor in Table 12 of Subsection 402.5.01.C, “Calculate Mean Pavement Air Voids.

Mixture which does not meet these requirements shall be paid for using the applicable acceptance schedule.

B. Determine Lot Acceptance

Pay factor adjustments are based on control sieves and asphalt cement content. The control sieves used in the mixture acceptance schedule for the various types of mix are indicated below:

<table>
<thead>
<tr>
<th>Control Sieves Used in the Mixture Acceptance Schedule</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalitic concrete 25 mm Superpave 1/2 in., No. 8 (12.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphalitic concrete 19 mm Superpave 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphalitic concrete 12.5 mm Superpave 3/8 in., No. 8 (9.5 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphalitic concrete 9.5 mm Superpave No. 4, No. 8 (4.75 mm, 2.36 mm) sieves and asphalt cement</td>
</tr>
<tr>
<td>Asphalitic concrete 4.75 mm Mix No. 8 (2.36 mm) sieve and asphalt cement</td>
</tr>
</tbody>
</table>

The Department will perform the following tasks:

1. Using the Mixture Acceptance Schedule—Table 9 or 10, of Subsection 400.3.06 to determine the mean of the deviations from the job mix formula per test results per lot.
2. Determine this mean by averaging the actual numeric value of the individual deviations from the job mix formula; disregard whether the deviations are positive or negative amounts.
3. Use the Asphalt Cement Content and Aggregate Gradation of Asphalt Concrete Mixture Acceptance Schedule—Table 9 of Subsection 400.3.06 to determine acceptance of surface mixes and the Mixture Acceptance Schedule—Table 10 of Subsection 400.3.06, to determine acceptance of subsurface mixes.

On Contracts involving 1,000 tons (1000 Mg) or less of asphaltic concrete, the mixture is accepted for 100 percent payment of the asphaltic concrete Unit Price provided it meets the following:

1. Minimum requirements for a 1.00 pay factor for asphalt cement content and a 0.90 pay factor for gradation in the applicable Mixture Acceptance Schedule—Table 9 or 10 of Subsection 400.3.06.
2. Minimum requirements for a 0.90 pay factor in Table 12 of Subsection 402.5.01.C, “Calculate Pavement Mean Air Voids.

If the material placed on Contracts involving 1,000 tons (1000 Mg) or less of asphaltic concrete does not meet the above requirements, the material will be paid for using the applicable acceptance schedule.

C. Calculate Pavement Mean Air Voids

The Department will determine the percent of maximum air voids for each lot by dividing the pavement mean air voids by the maximum pavement mean air voids acceptable.

The Department will determine the payment for each lot by multiplying the Contract Unit Price by the adjusted pay factor shown in the following Air Voids Acceptance schedule:

<table>
<thead>
<tr>
<th>Table 12 - Air Voids Acceptance Schedule</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pay Factor</td>
</tr>
<tr>
<td>1.00</td>
</tr>
<tr>
<td>0.97</td>
</tr>
</tbody>
</table>
When the range tolerance is exceeded, the Department will apply a pay factor of 0.95 as described in Subsection 400.3.06.B.2.

D. Asphaltic Concrete For Temporary Detours

Hot mix asphaltic concrete placed on temporary detours that will not remain in place as part of the permanent pavement does not require hydrated lime. Hot mix used for this purpose is paid for at an adjusted Contract Price.

Where the Contract Price of the asphaltic concrete for permanent pavement is let by the ton (megagram), the Contract Price for the asphaltic concrete placed on temporary detours is adjusted by subtracting $0.75/ton ($0.85/mg) of mix used.

Where the Contract price of the mix in the permanent pavement is based on the square yard (meter), obtain the adjusted price for the same mix used on the temporary detour by subtracting $0.04/yd² ($0.05/m²) per 1-in (25-mm) plan depth.

Further price adjustments required in Subsection 400.3.06, “Quality Acceptance,” are based on the appropriate adjusted Contract Price for mix used in the temporary detour work.

E. Determine Lot Payment

Determine the lot payment as follows:

1. When one of the pay factors for a specific acceptance lot is less than 1.0, determine the payment for the lot by multiplying the Contract Unit Price by the adjusted pay factor.

2. When two or more pay factors for a specific acceptance lot are less than 1.0, determine the adjusted payment by multiplying the Contract Unit Price by the lowest pay factor.

If the mean of the deviations from the job mix formula of the tests for a sieve or asphalt cement content exceeds the tolerances established in the Mixture Acceptance Schedule—Table 9 or 10 of Subsection 400.3.06 and if the Engineer determines that the material need not be removed and replaced, the lot may be accepted at an adjusted unit price as determined by the Engineer. If the pavement mean air voids exceed the tolerances established in the Air Voids Acceptance Schedule – Table 12, Subsection 402.5.01.C, remove and replace the materials at the Contractor’s expense.

If the Engineer determines that the material is not acceptable to leave in place, remove and replace the materials at the Contractor’s expense.

Office of Materials & Research/Office of Construction Bidding Administration
413.1 General Description
This work includes furnishing and applying a bituminous tack coat on a prepared road surface including cleaning the road surface.

413.1.01 Definitions
General Provisions 101 through 150.

413.1.02 Related References
A. Standard Specifications
   - Section 109—Measurement and Payment
   - Section 400—Hot Mix Asphalitic Concrete Construction
   - Section 424—Bituminous Surface Treatment
   - Section 427—Emulsified Asphalt Slurry Seal
   - Section 820—Asphalt Cement
   - Section 824—Cationic Asphalt Emulsion

B. Referenced Documents
   General Provisions 101 through 150.

413.1.03 Submittals
General Provisions 101 through 150.

413.2 Materials
Ensure that materials meet the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt cement, performance grade PG 58-22, PG 64-22, or PG 67-22</td>
<td>820.2.01</td>
</tr>
<tr>
<td>Cationic emulsified asphalt CRS-2h or CRS-3</td>
<td>824.2.01</td>
</tr>
</tbody>
</table>
Asphalt cement of performance grade PG 58-22, PG 64-22 or PG 67-22 is used for bituminous tack coat in work performed in Section 400. Use cationic emulsified asphalt as a special application material only if directed by the Engineer.

The Department may change the grade or type of bituminous materials without a change in the Contract Unit Price if the Engineer determines that the grade or type selected is not performing satisfactorily.

413.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

413.3 Construction Requirements

413.3.01 Personnel
General Provisions 101 through 150.

413.3.02 Equipment
Provide equipment in good repair, including the following units that meet the requirements of Subsection 424.3.02, Equipment:

- Power broom and blower
- Pressure distributor

413.3.03 Preparation
General Provisions 101 through 150.

413.3.04 Fabrication
General Provisions 101 through 150.

413.3.05 Construction

A. Seasonal and Weather Limitation
Do not apply tack coat if the existing surface is wet or frozen. Do not place emulsified asphalt if the air temperature in the shade is less than 40 °F (4 °C).

B. Application
Coat the entire areas to be paved with the tack coat unless directed otherwise by the Engineer. Apply tack coat with distributor spray bars instead of hand hoses, except in small areas that are inaccessible to spray bars.

C. Temperature of Material
Apply bituminous materials within the temperature ranges specified below.

<table>
<thead>
<tr>
<th>Bituminous Materials</th>
<th>Temperature of Application °F (°C)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt cement</td>
<td>350 - 400 (175 - 205)</td>
</tr>
<tr>
<td>CRS-2h</td>
<td>140 - 180 (60 - 80)</td>
</tr>
<tr>
<td>CRS-3</td>
<td>140 - 180 (60 - 80)</td>
</tr>
</tbody>
</table>

D. Cleaning
Immediately before applying the tack coat, clean the entire area free of loose dirt, clay, and other foreign materials.

E. Application Rate
The Engineer will determine the application rate of the bituminous tack coat.

F. Limitations and Areas Coated
Apply only enough tack coat to the prepared road surface that can be covered with the new pavement course the same working day the tack coat is applied.
G. Maintenance and Protection

After applying the tack coat material, allow it to break until it is tacky enough to receive the surface course. Do not allow traffic on the tack.

413.3.06 Quality Acceptance
General Provisions 101 through 150.

413.3.07 Contractor Warranty and Maintenance
General Provisions 101 through 150.

413.4 Measurement
Bituminous materials for tack coat applied and accepted are measured as outlined in Subsection 109.02, “Measurement of Bituminous Materials.”

Diluting emulsified tack coat is not ordinarily allowed except when used underneath slurry seal. The composition of diluted emulsified tack coat defined in Subsection 427.3.05, “Construction” is measured by the gallon (liter) of diluted mix.

413.4.01 Limits
General Provisions 101 through 150.

413.5 Payment
The accepted volume of bituminous material will be paid for at the Contract Unit Price per gallon (liter) for bituminous tack coat of the type and grade approved by the Engineer, complete in place. Payment is full compensation for preparing, cleaning, furnishing, hauling, applying material, and providing incidentals to complete the work.

Payment will be made under: CONSTRUCTION COMPLETE

Office of Materials & Research/Office of Construction Bidding Administration
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA
SPECIAL PROVISION
Section 424—Bituminous Surface Treatment

Delete Section 424 and substitute the following:

424.1 General Description
This work includes placing one or more applications of bituminous material and aggregate on a previously prepared base or pavement.

424.1.01 Definitions
- **Single Surface Treatment**: One application of bituminous material that is covered with aggregate.
- **Double Surface Treatment**: A bituminous material application that is covered with aggregate of the size specified in the proposal followed by a second bituminous material application that is covered with a second specified size aggregate.
- **Triple Surface Treatment**: A bituminous material application that is covered with a specified size aggregate followed by subsequent applications of bituminous material that are covered with successively smaller size nominal aggregates.

424.1.02 Related References
A. Standard Specifications
   - Section 105—Control of Work
   - Section 800—Coarse Aggregate
   - Section 802—Aggregates for Asphaltic Concrete
   - Section 820—Asphalt Cement
   - Section 824—Cationic Asphalt Emulsion

B. Referenced Documents
   - QPL 65

424.1.03 Submittals
General Provisions 101 through 150.

424.2 Materials
A. Bituminous Material
   Select the bituminous material from any type and grade listed in the materials table below. Notify the Engineer at least 10 days before ordering the bituminous material. The Engineer must approve the bituminous material choice.

   For a list of latex sources, see QPL 65.

   Ensure that materials meet the requirements of the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt Cement, Performance Grade PG 58-22 or PG 64-22*</td>
<td>820.2.01</td>
</tr>
<tr>
<td>Cationic Asphalt Emulsion, Grade CRS-2h or CRS-3*</td>
<td>824.2.01</td>
</tr>
<tr>
<td>Latex-Modified Cationic Asphalt Emulsion, Grade CRS-2L</td>
<td>824.2.02</td>
</tr>
</tbody>
</table>
B. Aggregates

The size and group of aggregates used in the surface treatment are specified in the Proposal under the appropriate Line Item.

Do not use unconsolidated limerock unless provided for in the Plans or Proposal.

Use Class B aggregates only where the surface treatment is used for shoulder construction or where it is to be overlaid with asphaltic concrete.

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Coarse Aggregate, Class A Crushed Stone or Crushed Slag, Group I or II</td>
<td>800.2.01</td>
</tr>
<tr>
<td>Fine Aggregate for Asphalitic Concrete*</td>
<td>802.2.01</td>
</tr>
</tbody>
</table>

*For sand seal application, use WA 10 washed screenings made from Group II aggregates.

424.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

424.3 Construction Requirements

424.3.01 Personnel
General Provisions 101 through 150.

424.3.02 Equipment

Have the Engineer approve equipment types and quantities before using equipment on the Project.

Ensure that the equipment used to construct the surface treatment:

- Produces work that complies with the standards in this section
- Is on the Project and in proper working order before construction begins and during construction.

A. Aggregate Spreader

The Department will inspect annually the aggregate spreader before it is used in the work. If the spreader is approved, the Department will attach an equipment certification sticker to the spreader.

Use a self-propelled aggregate spreader that can apply aggregate at the desired rate uniformly and accurately without corrugation, overlaps, or excess deficient areas.

Ensure that the spreader can spread courses to the required widths. Provide spreaders to promptly cover the full width of the asphalt application.

B. Pressure Distributor

The Department will inspect annually the pressure distributor before it is used in the work. If the distributor is approved, the Department will attach an equipment certification sticker to the distributor. The pressure distributor should be equipped as follows:

1. Mount the pressure distributor on pneumatic tires wide enough to prevent damage to the road surface.
2. Design, equip, maintain, and operate the distributor so that the bituminous material will be heated and applied evenly throughout the length of the spray bars. Ensure that it maintains a constant, uniform pressure on the nozzles.
3. Install screens between the tank and the nozzles and clean them frequently to prevent clogging.
4. Use an adjustable distributor that can deliver controlled amounts of bituminous material from 0.04 to 1.0 gal/yd², ± 0.02 gal/yd² (0.18 to 4.53 L/m², ± 0.10 L/m²) up to 24 ft (7.2 m) wide without atomization, streaking, or pulsation in the flow.
5. Use a distributor equipped with the following:
   - A tachometer and thermometers to indicate the application rate and the temperature of the tank contents
   - Measuring devices to accurately indicate the amount of bituminous material, in gallons (liters), in the distributor before and after each application
   - Full circulating spray bars that can be adjusted laterally to conform to a stringline and capable of vertical and horizontal adjustment.
• A positive shut-off control to prevent dripping bituminous material on the roadway
• A distributor tank equipped with a sample valve in a safe and convenient location to obtain bituminous material samples

C. Heating Equipment

Ensure that heating equipment will heat and maintain the bituminous material uniformly at the temperature required.
Provide an accurate thermometer.

D. Steel-Wheeled Rollers

Use self-propelled, tandem-type steel-wheeled rollers. The rollers shall weigh from 3 to 8 tons (3 to 7 Mg). Ensure that the roller weights within these limits can properly seat the aggregate without fracturing the aggregate particles. Equip the roller drums with scrapers to prevent pick up of material. Combination rollers with pneumatic-tired wheels that can be alternated with a steel drum are permitted as a substitute for steel-wheeled rollers.

E. Pneumatic-Tired Rollers

Use self-propelled, two axles, pneumatic-tired rollers with smooth-tread rubber tires aligned such that gaps between the tires on one axle are covered by the tires of the other axle. Equip the roller tires with scrapers and scrubbers to prevent pick up of material. Ensure that all tires are of the same size and ply rating and inflated to a minimum of 60 psi (415 kPa). Maintain tire pressure such that the difference in pressure between any two tires does not exceed 5 psi (35 kPa).
Provide ballast as directed by the Engineer.

F. Power Broom and Power Blower

Provide at least one power broom and one power blower, or a combination power broom and blower, that can remove dust or loose materials from the road surface.

424.3.03 Preparation

Firmly compact, finish, and prime new bases. Ensure that the bases conform to the lines, grades, and cross sections within the tolerances specified.

A. Removing Foreign Material

Use power brooms, power blowers, hand brooms, or other means to remove loose material, dust, dirt, clay, and other materials that prevent bituminous materials from adhering to the base.

Take special care to clean the outer edges thoroughly. Where necessary, use a motor grader blade to remove excess material off the paving edge.

B. Condition of Prime

Check the condition of prime as follows:
1. Ensure the prime is cured before placing the mat course.
2. Repair the prime if it is loose, soft, unbonded, removed, or damaged.
3. Remove concentrations of excess prime.
4. Perform additional rolling with a pneumatic-tired roller before surface treatment when directed by the Engineer.

424.3.04 Fabrication

General Provisions 101 through 150.

424.3.05 Construction

A. Observing Seasonal and Weather Limitations

Apply bituminous surface treatment only between April 15 and October 15 and only when:

• Ambient temperature has not been less than 50 °F (10 °C) for 48 hours immediately prior to application.
• No forecast of ambient temperature less than 50 °F (10 °C) for 48 hours immediately following application.
• Ambient temperature and road surface temperature is at least 60 °F (16 °C) and stable at the time of application.

No exceptions are permitted except as authorized by the Engineer.

Do not apply asphalt cement to a wet surface.

NOTE 1: When the relative humidity exceeds 80%, the ambient temperature exceeds 95 °F (35 °C), the pavement temperature exceeds 125 °F (52 °C) or the weather is windy or overcast, application of bituminous surface treatment will be at the discretion of the Engineer.
NOTE 2: If hot mix asphaltic concrete will be applied over the surface treatment, the Engineer may waive the seasonal limitations providing that traffic is not permitted on the surface treatment until it is covered with hot mix asphaltic concrete.

B. Using PG 64-22 or CRS-3

Only use PG 64-22 or CRS-3 when directed by the Engineer due to a problem with excessive aggregate pickup during high ambient temperature.

C. Observing Sequence of Operations and Quantities of Materials

The sequence of operations and quantities of materials are shown in Table 1, Table 2 and Table 3 (Table 1a—metric, Table 2a—metric and Table 3a—metric).

The Engineer will determine the material quantities to be used during construction and may change the minimum or maximum application rate of any course during construction if the total quantities are within the amounts shown in the Tables. Any deviation, or minus from the table quantities, will require a negotiated adjustment of the Contract price, which will be authorized by an approved Supplemental Agreement.

When a single application of bituminous surface treatment is used as a Crack-Relief Interlayer, use the quantities of materials shown in Table 2 (Table 2a—Metric).

When a sand seal application is Specified, use the quantities of materials shown in Table 3 (Table 3a—Metric).
### Section 424—Bituminous Surface Treatment – Table 1

<table>
<thead>
<tr>
<th>Stone Sizes</th>
<th>Application</th>
<th>Type Construction</th>
<th>Single</th>
<th>Double</th>
<th>Triple</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td></td>
<td>#89</td>
<td>#7</td>
<td>#6</td>
</tr>
<tr>
<td>1st appl.</td>
<td></td>
<td></td>
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<td>#6</td>
<td>#5</td>
</tr>
<tr>
<td>2nd appl.</td>
<td></td>
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<td>#89</td>
<td>#7</td>
<td>#7</td>
</tr>
<tr>
<td>3rd appl.</td>
<td></td>
<td></td>
<td>#89</td>
<td>#89</td>
<td>#89</td>
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</tbody>
</table>

<table>
<thead>
<tr>
<th></th>
<th>Control Tolerance</th>
<th>Control Tolerance</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>1st Application Bituminous Materials (gal/yd²)</td>
<td>± .02 .17–.19 .18–.25 .22–.30</td>
<td>± .02 .20–.27 .26–.34</td>
<td>± .02 .20–.30 .24–.34</td>
</tr>
<tr>
<td>PG58-22 or PG64-22</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .02 .20–.22 .21–.29 .25–.35</td>
<td>± .02 .23–.32 .30–.40</td>
<td>± .02 .23–.35 .28–.40</td>
</tr>
<tr>
<td>1st Application Stone (ft³/yd²)</td>
<td>± .03 .14–.18 .18–.26 .30–.42</td>
<td>± .03 .18–.26 .30–.42</td>
<td>± .03 .30–.42 .41–.53</td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>2nd Application Bituminous Materials (gal/yd²)</td>
<td>± .02 .18–.24 .24–.31</td>
<td>± .02 .20–.27 .20–.27</td>
<td>± .02 .20–.27 .20–.27</td>
</tr>
<tr>
<td>PG58-22 or PG64-22</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .02 .21–.28 .28–.36</td>
<td>± .02 .23–.32 .23–.32</td>
<td>± .02 .23–.32 .23–.32</td>
</tr>
<tr>
<td>2nd Application Stone (ft³/yd²)</td>
<td>± .03 .14–.18 .18–.26</td>
<td>± .03 .18–.26 .18–.26</td>
<td>± .03 .18–.26 .18–.26</td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>3rd Application Bituminous Materials (gal/yd²)</td>
<td>± .02 .18–.24 .18–.24</td>
<td>± .02 .18–.24 .18–.24</td>
<td>± .02 .18–.24 .18–.24</td>
</tr>
<tr>
<td>PG58-22 or PG64-22</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .02 .21–.28 .21–.28</td>
<td>± .02 .21–.28 .21–.28</td>
<td>± .02 .21–.28 .21–.28</td>
</tr>
<tr>
<td>3rd Application Stone (ft³/yd²)</td>
<td>± .03 .14–.18 .14–.18</td>
<td>± .03 .14–.18 .14–.18</td>
<td>± .03 .14–.18 .14–.18</td>
</tr>
<tr>
<td>Total Bituminous Materials (gal/yd²)</td>
<td>± .02 .17–.19 .18–.25 .22–.30</td>
<td>± .03 .38–.51 .50–.65</td>
<td>± .04 .58–.81 .62–.85</td>
</tr>
<tr>
<td>PG58-22 or PG64-22</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .02 .20–.22 .21–.29 .25–.35</td>
<td>± .03 .44–.60 .58–.76</td>
<td>± .04 .67–.95 .72–.10</td>
</tr>
<tr>
<td>Total Stone (ft³/yd²)</td>
<td>± .03 .14–.18 .18–.26 .30–.42</td>
<td>± .04 .32–.44 .48–.68</td>
<td>± .05 .62–.86 .73–.97</td>
</tr>
</tbody>
</table>

**Notes:**
- The bituminous material and stone for each application may be varied by the Engineer, at no increase in cost, outside of the minimum or maximum shown in the table provided the total of the materials is within the limits of the total minimum and total maximum of all courses.
- Maintain the control tolerances shown above or stop the work until the necessary corrections are made.
- Apply at least one seal coat to the mat course on the same day when multiple applications are specified.
### Section 424—Bituminous Surface Treatment, Crack-Relief Interlayer – Table 2

<table>
<thead>
<tr>
<th>Bituminous Material Application (gal/yd²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>PG 58-22 or PG 64-22</td>
<td>.20 – .25</td>
<td>± .02</td>
</tr>
<tr>
<td>CRS-2h, CRS-2L or CRS 3</td>
<td>.25 – .29</td>
<td>± .02</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Aggregate Application (ft³/yd²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>#7</td>
<td>.22 – .26</td>
<td>± .01</td>
</tr>
</tbody>
</table>

Notes:
- Target application rates for bituminous material and cover aggregate will be established by the Engineer within the limits shown in Table 2.
- Maintain the control tolerances shown above or stop the work until the necessary corrections are made.
- Cover the single surface treatment Crack-Relief Interlayer with HMA Leveling on the same day.

### Section 424—Bituminous Surface Treatment, Sand Seal – Table 3

<table>
<thead>
<tr>
<th>Bituminous Material Application (gal/yd²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>PG 58-22 or PG 64-22</td>
<td>.18 – .22</td>
<td>± .02</td>
</tr>
<tr>
<td>CRS-2h, CRS-2L or CRS 3</td>
<td>.20 – .26</td>
<td>± .02</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Aggregate Application (ft³/yd²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>WA 10 Washed Screenings</td>
<td>.10 – .14</td>
<td>± .02</td>
</tr>
</tbody>
</table>

Notes:
- Target application rates for bituminous material and seal sand will be established by the Engineer within the limits shown in Table 3.
- Maintain the control tolerances shown above or stop the work until the necessary corrections are made.
- Cover the coarse aggregate seal stone with seal sand on the same day.
### Section 424—Bituminous Surface Treatment – Table 1a (Metric)

<table>
<thead>
<tr>
<th>Application</th>
<th>Type Construction</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Single</td>
</tr>
<tr>
<td>Stone Sizes</td>
<td></td>
</tr>
<tr>
<td>1st appl.</td>
<td>#89</td>
</tr>
<tr>
<td>2nd appl.</td>
<td></td>
</tr>
<tr>
<td>3rd appl.</td>
<td></td>
</tr>
<tr>
<td>Control</td>
<td>Tolerance</td>
</tr>
<tr>
<td>1st Application Bituminous Materials (L/m²) PG58-22 or PG64-22</td>
<td>± .09</td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .09</td>
</tr>
<tr>
<td>1st Application Stone (m³/m²)</td>
<td>± .001</td>
</tr>
<tr>
<td>2nd Application Bituminous Materials (L/m²) PG58-22 or PG64-22</td>
<td>± .09</td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .09</td>
</tr>
<tr>
<td>2nd Application Stone (m³/m²)</td>
<td>± .001</td>
</tr>
<tr>
<td>3rd Application Bituminous Materials (L/m²) PG58-22 or PG64-22</td>
<td>± .09</td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .09</td>
</tr>
<tr>
<td>3rd Application Stone (m³/m²)</td>
<td>± .001</td>
</tr>
<tr>
<td>Total Bituminous Materials (L/m²) PG58-22 or PG64-22</td>
<td>± .09</td>
</tr>
<tr>
<td>CRS-2h, CRS-3</td>
<td>± .09</td>
</tr>
<tr>
<td>Total Stone (m³/m²)</td>
<td>± .001</td>
</tr>
</tbody>
</table>

Notes:
- The bituminous material and stone for each application may be varied by the Engineer, at no increase in cost, outside of the minimum or maximum shown in the table provided the total of the materials is within the limits of the total minimum and total maximum of all courses.
- Maintain the control tolerances shown above or stop the work until the necessary corrections are made.
- Apply at least one seal coat to the mat course on the same day when multiple applications are specified.
Section 424—Bituminous Surface Treatment, Crack-Relief Interlayer – Table 2a (Metric)

<table>
<thead>
<tr>
<th>Bituminous Material Application (L/m²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>PG 58-22 or PG 64-22</td>
<td>.91 – 1.13</td>
<td>± .09</td>
</tr>
<tr>
<td>CRS-2h, CRS-2L or CRS 3</td>
<td>1.13 – 1.31</td>
<td>± .09</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Aggregate Application (m³/m²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>#7</td>
<td>.007 – .009</td>
<td>± .0003</td>
</tr>
</tbody>
</table>

Notes:
- Target application rates for bituminous material and cover aggregate will be established by the Engineer within the limits shown in Table 2a (Metric).
- Maintain the control tolerances shown above or stop the work until the necessary corrections are made.
- Cover the single surface treatment Crack-Relief Interlayer with HMA Leveling on the same day.

Section 424—Bituminous Surface Treatment, Sand Seal – Table 3a (Metric)

<table>
<thead>
<tr>
<th>Bituminous Material Application (L/m²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>PG 58-22 or PG 64-22</td>
<td>.81 – 1.00</td>
<td>± .09</td>
</tr>
<tr>
<td>CRS-2h, CRS-2L or CRS 3</td>
<td>.90 – 1.18</td>
<td>± .09</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Aggregate Application (m³/m²)</th>
<th>Application Rate</th>
<th>Control Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>WA 10 Washed Screenings</td>
<td>.003 – .005</td>
<td>± .09</td>
</tr>
</tbody>
</table>

Notes:
- Target application rates for bituminous material and seal sand will be established by the Engineer within the limits shown in Table 3.
- Maintain the control tolerances shown above or stop the work until the necessary corrections are made.
- Cover the coarse aggregate seal stone with seal sand on the same day.

D. Heating Bituminous Material

Evenly heat the entire mass of bituminous material for each application under positive control. While the material is being applied, maintain it within the specified temperature range.

E. Applying Bituminous Material

The following are temperatures at which bituminous material shall be applied.

<table>
<thead>
<tr>
<th>Bituminous Material</th>
<th>Asphalt Cement</th>
<th>CRS-2h</th>
<th>CRS-3</th>
<th>CRS-2L</th>
</tr>
</thead>
<tbody>
<tr>
<td>Application temperature °F (°C)</td>
<td>275–350 (135–177)</td>
<td>140–180 (60–82)</td>
<td>140–180 (60–82)</td>
<td>140–180 (60–82)</td>
</tr>
</tbody>
</table>

NOTE 1: Do not store emulsified asphalts at temperatures exceeding 150 °F (65 °C) for any extended time.

NOTE 2: Do not place bituminous surface treatment on fresh asphaltic concrete, except for paved shoulders, until the asphaltic concrete has been in place at least 30 days.

The Engineer will designate the maximum area to which bituminous material may be applied at one time. Apply the material as follows:

1. After applying the bituminous material to the section, immediately cover it with the correct application rate of aggregate before beginning the next section.

Do not apply the bituminous material to the full width of the pavement unless the aggregate spreader can immediately cover the full width of the applied material.

NOTE: Never allow bituminous material to chill, set up, dry, or reach a condition that impairs the retention of cover aggregate before the aggregate is applied.

2. When a longitudinal joint is necessary:
   - Do not overlap the applications more than 4 in (100 mm).
• Do not leave any area uncovered.
• Never allow excess quantities of bituminous materials to build up.

3. On curves that require widening:
   a. Shoot the extra width on the outside first.
   b. Shoot the normal width with the distributor and follow the inside paving edge.

4. Ensure that the spray of bituminous material is uniform at all times. If the spray is not uniform:
   a. Stop the work.
   b. Change equipment, personnel, or methods to attain the required uniformity.
   c. Apply bituminous material at one-half the width of the roadway, if necessary.

5. If streaking develops:
   a. Stop the distributor and correct the problem before proceeding.
   b. Use a hand hose or a hand pouring pot to cover the streaked areas at approximately the same application rate of bituminous material.

6. If a part of the work cannot be reached by the distributor, treat it by hand hoses with nozzles.

7. Protect curbs, gutters, handrails, and other structures from discoloration by the bituminous material. Remove bituminous material that is sprayed or spilled on these structures.

8. Ensure that the bituminous material joins neatly in place by beginning and ending the asphalt application from a heavy paper or tight trough that is longer than the width of the treatment being applied. Place it to catch and hold the surplus material.

9. When cleaning and emptying the distributor, empty it where the bituminous material can be covered with dirt and completely disposed of without damaging the Rights-of-Way.

F. Spreading Aggregates

Spread the aggregates as follows:

1. Ensure that aggregates do not contain free moisture when spread.
2. Apply aggregate immediately after applying bituminous materials.
3. Uniformly spread the aggregate at the specified rate without corrugations, overlaps, excess, or deficient areas.
4. Move the spreader at a uniform speed, regardless of the grade.
5. Ensure that the distance that the aggregate free falls remains constant during spreading.
6. Remove corrugations. Operate the spreader to prevent overlap of aggregates. If overlap occurs, remove the excess aggregate before rolling.
7. Ensure a uniform aggregate spread by hand spotting and brooming as necessary.

G. Rolling

Observe the following guidelines for rolling bituminous surface treatment:

1. Synchronize the speed of the distributor and aggregate spreader with that of the rolling operation.
2. Use a minimum of two (2) individual rollers, one of which must be a pneumatic-tired roller meeting the requirements of Subsection 424.3.02.E.
3. If a steel-wheeled roller will fracture the aggregate, use pneumatic-tired rollers only.
4. Begin rolling within one minute after spreading the aggregate.
5. Operate rollers at speeds not exceeding 5 mph.
6. Proceed in a longitudinal direction, beginning at the outside edge of the aggregate application.
7. A roller pass is defined as one trip in a single direction.
8. Overlap each roller pass by approximately 1/2 the roller width.
9. Provide a minimum of three (3) roller passes per roller for each layer of aggregate to properly embed the aggregate particles.

Note: Unless a sufficient number of rollers are in operation to complete the above requirements, do not make subsequent applications of bituminous material until rolling of the previous application is completed.

H. Brooming
Use a revolving broom as necessary, supplemented by hand brooming, to remove or redistribute excess stone. Sweep the completed surface treatment within the first three hours of the next available workday following placement. Take care not to unseat bonded stone when sweeping.

I. Controlling Traffic

Do not allow traffic on the surface treatment until the bituminous material has cured sufficiently to ensure that the aggregate will not be loosened, dislodged, or whipped off by slow moving traffic.

Control traffic to speeds not exceeding 25 mph for a minimum of two hours after application of the seal stone and until the Engineer permits the road to be opened to normal traffic speeds.

Use pilot vehicles to control traffic speeds.

424.3.06 Quality Acceptance
General Provisions 101 through 150.

424.3.07 Contractor Warranty and Maintenance
Maintain and protect the surface course as specified in Section 105 until the Project has been accepted. Make repairs as the Engineer directs. The cost of maintenance, protection, and repair is included in the Unit Prices Bid for the Item for which they apply.

424.4 Measurement
The area to be measured is the number of square yards (meters) of each type of surface treatment completed and accepted.

424.4.01 Limits
The length is measured along the surface. The width is specified on the Plans, plus or minus any authorized changes.
Irregular areas are measured by the surface square yard (meter) within the lines shown on the Plans or authorized changes.

424.5 Payment
The accepted area of surface treatment will be paid for at the Contract Unit Price per square yard (meter) complete for each type and stone size specified.

Payment will be made under: CONSTRUCTION COMPLETE

Office of Materials & Research/Office of Construction Bidding Administration
Delete Section 500 and substitute the following:

**500.1 General Description**
This work consists of manufacturing and using Portland cement concrete to construct structures.

**500.1.01 Definitions**
General Provisions 101 through 150.

**500.1.02 Related References**
A. Standard Specifications
   - Section 104—Scope of Work
   - Section 211—Bridge Excavation and Backfill
   - Section 431—Grind Concrete Pavement
   - Section 507—Prestressed Concrete Bridge Members
   - Section 511—Reinforcement Steel
   - Section 530—Waterproofing Fabrics
   - Section 531—Dampproofing
   - Section 621—Concrete Barrier
   - Section 800—Coarse Aggregate
   - Section 801—Fine Aggregate
   - Section 830—Portland Cement
   - Section 836—Special Surface Coating for Concrete
   - Section 838—Graffiti-Proof Coating for Concrete
   - Section 853—Reinforcement and Tensioning Steel
   - Section 865—Manufacture of Prestressed Concrete Bridge Members
B. Referenced Documents
ASTM A 653/653M
ASTM A 924/924/M
ASTM A 681
ASTM C 685
ASTM D 260, Type I or Type II
AASHTO Specifications
AASHTO M 148 or C 309
AASHTO M 171
AASHTO M 194
AASHTO T 22
AASHTO T 126
AWS D 2.0
Laboratory Standard Operating Procedure. Quality Assurance for Ready Mix Concrete Plants in Georgia
Standard Operating Procedure for Ready Mix Concrete
American Iron and Steel Institute Specification for the Design of Cold Formed Steel Structural Members
Federal Specification TT-P-641d, Type II
Georgia Standards 4948 and 9031-L
QPL 10
QPL 17
QPL 23
GDT 134
DOT 525

500.1.03 Submittals
A. Concrete Mix Designs

The Contractor is responsible for all concrete mix designs. Ensure that concrete mixes contain enough cement to produce workability within the water- cement ratio specified in Table 1—Concrete Mix Table, below.

Design concrete mixes that meet the requirements of the Table 1—Concrete Mix Table, below. The Office of Materials and Research will determine the concrete properties using the applicable method in Section 500 of the Sampling, Testing, and Inspection Manual.

Table 1—Concrete Mix Table

<table>
<thead>
<tr>
<th>Class of Concrete</th>
<th>(2) Coarse Aggregate Size No.</th>
<th>(1 &amp; 6) Minimum Cement Factor lbs/yd³</th>
<th>Max Water/Cement ratio lbs/lb</th>
<th>(5) Slump acceptance Limits (in) Lower-Upper</th>
<th>(3 &amp; 7) Entrained Air Acceptance Limits (%) Lower-Upper</th>
<th>Minimum Compressive Strength at 28 days (psi)</th>
</tr>
</thead>
<tbody>
<tr>
<td>“AAA”</td>
<td>67,68</td>
<td>675</td>
<td>.440</td>
<td>2</td>
<td>4</td>
<td>2.5</td>
</tr>
<tr>
<td>“AA1”</td>
<td>67,68</td>
<td>675</td>
<td>.440</td>
<td>2</td>
<td>4</td>
<td>2.5</td>
</tr>
</tbody>
</table>
### Concrete Mix Specifications

<table>
<thead>
<tr>
<th>Class of Concrete</th>
<th>(2) Coarse Aggregate Size No.</th>
<th>(1 &amp; 6) Minimum Cement Factor kg/m³</th>
<th>Max Water/Cement ratio kg/kg</th>
<th>(5) Slump acceptance Limits (mm) Lower - Upper</th>
<th>(3 &amp; 7) Entrained Air Acceptance Limits (%) Lower - Upper</th>
<th>Minimum Compressive Strength at 28 days (MPa)</th>
</tr>
</thead>
<tbody>
<tr>
<td>&quot;AAA&quot;</td>
<td>67,68</td>
<td>400</td>
<td>.440</td>
<td>50 - 100</td>
<td>2.5 - 6.0</td>
<td>35</td>
</tr>
<tr>
<td>&quot;AA1&quot;</td>
<td>67,68</td>
<td>400</td>
<td>.440</td>
<td>50 - 100</td>
<td>2.5 - 6.0</td>
<td>30</td>
</tr>
<tr>
<td>&quot;AA&quot;</td>
<td>56,57,67</td>
<td>375</td>
<td>.445</td>
<td>50 - 100</td>
<td>3.5 - 7.0</td>
<td>25</td>
</tr>
<tr>
<td>&quot;A&quot;</td>
<td>56,57,67</td>
<td>360</td>
<td>.490</td>
<td>50 - 100</td>
<td>2.5 (3) - 6.0</td>
<td>20</td>
</tr>
<tr>
<td>&quot;B&quot;</td>
<td>56,57,67</td>
<td>280</td>
<td>.660</td>
<td>50 - 100</td>
<td>0.0 - 6.0</td>
<td>15</td>
</tr>
<tr>
<td>&quot;CS&quot;</td>
<td>56,57,67</td>
<td>165</td>
<td>1.400</td>
<td>50 - 90</td>
<td>3.0 - 7.0</td>
<td>7 (4)</td>
</tr>
</tbody>
</table>

**Notes:**

1. Portland cement may be partially replaced with fly ash as provided in Subsection 500.3.04.D.4 or with granulated iron blast furnace slag as provided in Subsection 500.3.04.D.5.

2. Specific size of coarse aggregate may be specified.

3. Lower limit is waived when air entrained concrete is not required.

4. The mixture will be capable of demonstrating a laboratory compressive strength at 28 days of 1000 psi (7 MPa) + 0.18 R*. Compressive strength will be determined based upon result of six cylinders prepared and tested in accordance with AASHTO T 22 and T 126.

   * Where R = Difference between the largest observed value and the smallest observed value for all compressive strength specimens at 28 days for a given combination of materials and mix proportions prepared together.

5. Designed slump may be altered by the Office of Materials and Research when Type “F” water reducers are used.

6. Minimum cement factor shall be increased by 50 lbs/yd³ (30 kg/m³) when size No. 7 coarse aggregate is used.

7. When Class A is specified for bridge deck concrete, the entrained air acceptance limits shall be 3.5% to 7.0%.

Submit all concrete mix designs to the Office of Materials and Research (OMR) for review. The Department will approve mixes that contain materials from approved sources and produce concrete that meets these Specifications.

Submit concrete mix design proportions for approval by one of the following methods:

1. Request Approval of Specific Proportions
When requesting approval of specific concrete mix design proportions for classes of concrete, include the following information:

- Source of each material
- Apparent specific gravity of the cement and the fly ash, if used
- Bulk specific gravity (saturated surface dry) of each aggregate
- Percent absorption of each aggregate
- Amount of each material required to produce a cubic yard (meter) of concrete
- Proportions of admixtures per cubic yard (meter) of concrete and any use limitations
- Proposed slump and air content of the design
- Evidence that the proposed mixture complies with Subsection 500.1.03.

Concrete mix designs that do not have a proven performance record and have not been used by the Department must meet minimum laboratory strength requirements.

2. Obtain Ready-Mix Design Proportions for commonly used materials

Get approved concrete mix designs from authorized ready-mix concrete plants. Ready-mix concrete plants approved according to Laboratory Standard Operating Procedure “Quality Assurance for Ready Mix Concrete Plants in Georgia” are authorized to submit concrete mix designs for approval. See QPL 10 for a list of approved plants.

3. Use Laboratory-Designed Proportions for commonly used materials

Use laboratory-designed concrete mix proportions from either of the following sources:

a. Laboratory-designed proportions are available for commonly used combinations of materials. Request these mixes in writing from the State Materials and Research Engineer. Request specific classes of concrete and specify the source of ingredients.

b. Select a combination of materials from approved sources and request that the laboratory determine a mix that meets requirements in the Table 1—Concrete Mix Table above. The laboratory will establish proportions for strength and workability under laboratory conditions.

B. Delivery Tickets

Have the concrete plant transmit delivery tickets (DOT Form 525) with each load of concrete delivered to the work site. Give the Engineer one of these delivery tickets.

Ensure that the following information is on the delivery ticket:

- Project designation
- Date
- Time
- Class and quantity of concrete
- Actual batch proportions
- Free moisture content of aggregates
- Quantity of water withheld
- Concrete mixing revolutions

If available forms do not provide the required information, ask the Engineer to provide one.

C. Formwork Plans

The Engineer may require detailed formwork plans for review. If so, prepare the formwork plans and submit them to the Engineer. In no case will the Contractor be relieved of responsibility for the formwork plans.

When constructing permanent steel bridge deck forms, submit bar support details and types to the Department for approval before placing the deck form reinforcement.

D. Falsework Plans

Submit, for review by the Engineer, detailed falsework plans for spans under which traffic flows.
The Engineer may require plans for spans that do not accommodate traffic.

E. Shop and Erection Drawings
Submit fabricators’ shop and erection drawings to the Engineer for review and approval. Indicate the following in the drawings:

- Grade of steel
- Physical and section properties for permanent steel bridge deck form sheets
- Locations where the forms are supported by steel beam flanges subject to tensile stresses

F. Hauling Vehicle Information
Before hauling starts on new bridges, submit the following information for each vehicle:

- Weight on each axle, empty
- Weight on each axle, fully loaded
- Center-to-center distances of axles
- Center-to-center distances of wheels measured parallel to each axle

G. Cold Weather Concrete Curing and Protection Plan
Secure the Engineer’s approval of a “Cold Weather Concrete Curing and Protection Plan” for bridges and structures. Emphasize protection for the underside of bridge decks when using metal forms and include the protection procedures to be used.

Protection procedures shall keep the concrete above 50 °F (10 °C) for 72 hours after placement and above freezing for 6 days after placement. Choose the protection method from Table 2 based on the expected temperature within 48 hours after concrete placement.

<table>
<thead>
<tr>
<th>Protection Procedure</th>
<th>Expected Temperatures Within 48 Hours</th>
</tr>
</thead>
<tbody>
<tr>
<td>Heated enclosures</td>
<td>Below 25 °F (-4 °C)</td>
</tr>
<tr>
<td>Commercial blankets</td>
<td>Below 25 °F (-4 °C)</td>
</tr>
<tr>
<td>Batt insulation</td>
<td>Below 25 °F (-4 °C)</td>
</tr>
<tr>
<td>Heavy-duty polyethylene</td>
<td>25 °F (-4°C) or above</td>
</tr>
</tbody>
</table>

500.2 Materials
Ensure that materials meet the Specification requirements of Table 3:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Coarse Aggregate (1)</td>
<td>800.2.01</td>
</tr>
<tr>
<td>Fine Aggregate Size No. 10</td>
<td>801.2.02</td>
</tr>
<tr>
<td>Dampproofing or Waterproofing Material (Bituminous)</td>
<td>826.2.01</td>
</tr>
<tr>
<td>Portland Cement (2)</td>
<td>830.2.01</td>
</tr>
<tr>
<td>Portland-Pozzolan Cement (2)</td>
<td>830.2.03</td>
</tr>
<tr>
<td>Admixtures:</td>
<td>831.2.01</td>
</tr>
<tr>
<td>Air-Entraining Admixtures</td>
<td>831.2.02</td>
</tr>
<tr>
<td>Retarding Admixtures</td>
<td>831.2.02</td>
</tr>
<tr>
<td>Water Reducing Admixtures</td>
<td>831.2.02</td>
</tr>
</tbody>
</table>
1. Use either Class A or Class B coarse aggregate of the designated size, except when using limestone or dolomite in bridge structures. When using limestone or dolomite, use Class A coarse aggregate.

2. Use Type I or Type II Portland cement or Type IP Portland-Pozzolan cement unless otherwise specified. Do not use air-entraining cement.

3. The gradation requirements of graded aggregate are modified to require 30% to 45% by weight passing the No. 10 (2.00 mm) sieve.

Construct bridge sections containing duct enclosures for stressing tendons using concrete with a maximum stone size of No. 7.

Use concrete manufactured at plants that qualify as approved sources according to the Standard Operating Procedure for Ready Mix Concrete. See QPL 10 for a list of approved plants.

For a list of approved deck oil protective surface treatment sources, see QPL 23.

500.2.01 Delivery, Storage, and Handling

A. Aggregate Stockpile

Stockpile aggregate as follows:

1. Keep stockpile areas firm, reasonably level, well-drained, clean, and free of sod or foreign matter.
2. Stockpile aggregate separately by type and source.
3. Form stockpiles using methods and equipment that do not cause the aggregate to segregate, become contaminated, or degrade. The Engineer may reject improperly formed stockpiles.
4. Stockpile aggregate long enough for the moisture content to stabilize.
5. Do not use aggregates stored in pits or silos that contain water.

B. Aggregate Handling

Operate aggregate handling equipment carefully to minimize segregation, breaks, spills, contamination, and mixing of the sizes and types of aggregates.

C. Cement Storage

Store cement as specified below. Reject all caked, lumpy, or contaminated cement.

1. Bulk Cement

   Use bulk cement unless the Engineer allows bag cement to be used.

   Store bulk cement in bins or silos designed for this purpose. Provide moisture-proof storage containers with a mechanism that allows cement to flow freely from the discharge opening.

2. Different Brands
Store and use cement of different brands and types, or from different mills separately.

D. Admixture Storage and Handling
Carefully store and dispense admixtures as recommended by the manufacturer to prevent contamination.

E. Concrete Handling and Placing
Handle and place concrete according to the following:

1. Haul Time Limitations
   Ensure that concrete reaches its final position in the forms within one hour after adding the cement to the aggregates. If retarders or water reducers are used, the allowable time limit increases to 1-1/2 hours. Test concrete immediately for acceptance tolerances before placing in forms using limits established in Table 1—Concrete Mix Table.

2. Placement Limitations
   After delivering the concrete to the job site or the staging area at the site or after mixing the concrete at the site, transport it carefully to the placement point to prevent excessive slump loss or segregation. Use any of the following equipment:
   • Buckets
   • Buggies
   • Pumps
   • Other approved means

F. Form Storage
Store forms off the ground.

G. Precast Unit Handling
Except as noted below, the applicable portions of Subsections 507.2.01, “Delivery, Storage, and Handling,” 507.3.05.A, “Prepare Bearing Areas,” 507.3.05.B, “Erecting PSC Bridge Members,” and 507.3.05.D, “Concrete Finish,” shall govern.

Handle precast, non-prestressed units as follows:

1. Do not lift the units from the casting bed until the concrete reaches a strength of at least 1,500 psi (10 MPa).
2. Do not transport or erect the units until they reach a strength of at least 3,000 psi (20 MPa).
3. Restrict live loads (including erection equipment) on the units until they reach a minimum strength of 4,500 psi (30 MPa).

500.3 Construction Requirements

500.3.01 Personnel
A. Supervision, Personnel, and Skilled Workers
   Provide enough supervision, personnel, and skilled workers to do the following:
   1. Properly produce, place, and finish concrete in each pour unit according to Subsection 500.3.05.P, Table 5—Minimum Placement Rates or as required by the Plans.
   2. Check screed clearances and tolerances before beginning deck pours.
   3. Place concrete without delays.

B. Plant Operator Certification
   Volumetric proportioning requires that the operator be certified by the Office of Materials and Research.

500.3.02 Equipment
A. Equipment Restrictions
   Do not use delivery, conveyance, or vibratory units that leak grout, water, oil, or gas.
Provide enough equipment, tools, and materials to properly produce, place, and finish concrete in each pour unit according to the Subsection 500.3.05.P, Table 5—Minimum Placement Rates or as required by the Plans.

The Engineer may prohibit equipment that delays concrete placement.

B. Volumetric Proportioning Equipment

When concrete ingredients are proportioned volumetrically, obtain the Engineer’s approval for the equipment and its calibration and operation.

Ensure the following:

- The equipment meets the specifications in ASTM C 685.
- The concrete producer conducts calibration tests at least every 6 months.
- The equipment is calibrated for each new concrete mix before production.

C. Batching Plant Equipment

Ensure that batching plants have the following equipment and that the equipment meets the standards listed.

1. Bins

   Ensure that bins and bin compartments meet the following standards:
   - Adequate capacity for the required concrete production
   - Supported on a rigid framework on a stable foundation capable of holding the bins securely
   - Designed to discharge efficiently and freely into the weigh hopper
   - Positive means of control that slows down and shuts off the material flow when the weigh hopper has the correct quantity.
   - Discharging mechanisms that prevent material leaks when closed
   - Leak-free aggregate storage bins
   - Divided aggregate storage bins for fine aggregate and each size of coarse aggregate
   - Partitioned aggregate storage bin compartment that prevents the materials from mixing
   - Leak-proof, moisture-proof cement bins with a vibrator or other mechanism to discharge cement

2. Weigh Hoppers

   Ensure that weigh hoppers meet the following standards:
   - Have suitable containers freely suspended from scales
   - Have adequate capacity to maintain the Subsection 500.3.05.P, Table 5—Minimum Placement Rates
   - Have a discharge mechanism that prevents material leaks when closed
   - Have vents to permit air to escape
   - Have vibrators or other equipment that ensures complete and efficient discharge of materials
   - Have a dust seal and a port or valve for sampling cement

3. Scales

   Scales used for weighing concrete materials shall have accuracy within plus or minus one percent under operating conditions.

   Ensure the following:
   - When directed by the Engineer, the owner demonstrates the accuracy of the scales.
   - Scales are kept clean and in good operating condition.
   - The scale operator can clearly see indicating devices.
   - The scale operator can easily access controls.

D. Mixers and Agitators

Ensure that mixers and agitators meet the following requirements:

1. General Requirements for Mixers and Agitators
Provide mixers and agitators that meet these requirements:

a. Capacity Plates
   Ensure that the mixer or agitator has a legible metal plate or plates attached in an easily visible location. The plates shall indicate the rated capacity in cubic yards (meters) for mixing and agitating.

b. Concrete Production
   The mixer shall produce concrete that meets the requirements in the Table 1—Concrete Mix Table.

c. Mixer Performance Test
   The mixer or agitator may be required to pass a mixer performance test. Mixer performance will be evaluated at the discretion of the Engineer.
   Mixer performance tests will include the following by the OMR:
   1) Taking samples of concrete at the one-quarter and three-quarter points of the batch discharge
   2) Measuring the slumps of each concrete sample
      If the two slump values differ by more than 2 in (50 mm), do not use the mixer or agitator until it meets the requirements of the test.
      The Engineer may permit the equipment to be used if the 2 in (50 mm) tolerance can be met by using a longer mixing time or a smaller batch.

2. Mixing Speed
   Follow these guidelines for mixing speed:
   - Do not exceed 150 revolutions at mixing speed.
   - Discharge all concrete from truck mixers before drum or blades reach 300 revolutions, including revolutions at agitating speed.
   - Use the mixing speed defined by the manufacturer for the mixing equipment.
   - If the manufacturer’s definition of mixing speed is not available, use a mixing speed of 6 to 18 revolutions per minute.

3. Mixer and Agitator Maintenance
   Maintain mixers and agitators as follows:
   a. When mixers and agitators are discharged, remove the entire contents before adding materials for the next batch.
   b. Clean mixers and agitators often to prevent concrete and grout accumulation.
   c. Do not discharge cleaning water into any pipe, catch basin, or structure.
   d. If cement or aggregates accumulate in mixers and agitators when cleaning water is discharged, remove them immediately at no expense to the Department.

4. Mixer Types
   Use stationary mixers or truck mixers.
   a. Stationary Mixers
      Ensure that stationary mixers meet the following standards:
      1) Combine the concrete ingredients into a homogeneous, uniform mass within the specified time and when loaded to capacity.
      2) Efficiently and uniformly discharge the concrete within the tolerances allowed in Subsection 500.3.02.D.1.e, “Mixer Performance Test.”
      3) Permit discharge only after the specified mixing time has elapsed using a locking device.
   b. Truck Mixers
      Ensure that truck mixers meet the following standards:
      - Meets the requirements listed in Subsection 500.3.02.D.4.a, “Stationary Mixers”
      - Has an approved revolution counting device in good operating condition
      - Does not haul more than the rated capacity in cubic yards (meters) as shown on the attached capacity plates

5. Agitator Types
Use truck agitators or truck mixers operating at agitating speed.
Ensure that agitators meet the following requirements:
  a. Keeps the mixed concrete in a homogeneous, uniform mass
  b. Efficiently and uniformly discharges the concrete within the tolerances allowed in Subsection 500.3.02.D.1.c, “Mixer Performance Test”

E. Concrete Buckets
   Keep concrete buckets clean and in good working condition.

F. Concrete Buggies
   Keep concrete buggies clean and in good working condition.

G. Concrete Pumps
   Concrete pumping equipment is subject to the Engineer’s approval. Use pumping equipment that has adequate capacity and is suitable for the proposed work.

H. Chutes and Troughs
   Do not use chutes longer than 50 ft (15 m) without the Engineer’s permission.
   Flush chutes and troughs with water after each run. Do not discharge this water into freshly placed concrete or into conveyance unit.
   Promptly remove hardened concrete from chutes and troughs.
   Ensure that chutes and troughs meet the following requirements:
   1. Metal or metal lined
   2. Slope not exceeding one vertical to three horizontal
   3. Baffles or a series of short lengths placed to reverse the direction of the concrete flow, when used on steep slopes

I. Pipes or Tubes
   Use pipes or tubes to place concrete when the operation requires dropping the concrete more than 5 ft (1.5 m). Thoroughly clean the pipes or tubes after each pour.
   Use pipes made of metal or other approved material and long enough to deposit the concrete as close to its final position as possible.

J. Vibrators
   Provide enough vibratory units, including at least one additional stand-by unit in good working condition, to compact concrete immediately after it is placed. Have a stand-by unit at the site before each pour is started.
   On Projects consisting entirely of small pours (10 yd³ [8 m³] or less), the Engineer may waive the stand-by requirement.
   Ensure that vibrators meet the following conditions:
   • Approved internal rotation-type design
   • A power supply that constantly vibrates the concrete at frequencies of not less than 4500 impulses per minute
   • A vibration intensity that visibly affects a mass of concrete with a 1 in (25 mm) slump through at least a 18 in (450 mm) radius

K. Screeds
   Do not use vibratory screeds (screeds that use a transverse strike-off motion) without the Engineer’s approval. Use screeds that are:
   • Mechanically operated
   • Designed and constructed to screed with the strike-off parallel to the center line
   • Readily adjustable
   • Capable of maintaining proper adjustment throughout the screeding operation
   The two screed types are:
1. Longitudinal Screeds
   Unless otherwise noted on the Plans, use longitudinal screeds only on pour lengths of 70 ft (20 m) or less.

2. Transverse Screeds
   Use transverse screeds on any pour, unless otherwise noted on the Plans. However, transverse screeds are required on pour lengths above 70 ft (20 m).

Support screeds outside the pour area that will receive a surface finish. Do not use intermediate supports or guides.

Adjust screeds to the camber specified on the Plans. Check the camber as often as necessary.

Have the Engineer approve the following for screeds and their supports:
- Weight
- Durability
- Adjustability
- Accuracy
- Mechanical condition
- Operational results

Furnish the equipment necessary to check screed clearances and tolerances before pouring decks.

L. Underwater Placement Equipment

Place concrete under water using the following underwater placement equipment:

1. Tremie
   Use a tremie when depositing concrete in water above 10 ft (3 m) deep. Ensure that tremie is:
   - At least 8 inches in (200 mm) diameter
   - Constructed in sections with watertight couplings

2. Bottom Dump Bucket
   Where the Engineer permits, use a bottom dump bucket in water up to 10 ft (3 m) deep.
   Ensure that the bottom of the bucket opens only when it touches the surface that receives the charge and that the top of the bucket has a lid or cover.

M. Fogging Equipment

To supply additional moisture to the concrete, use fogging equipment with the following characteristics:
- A heavy-duty pump capable of delivering 2 gal (7.6 L) of water per minute to a 0.062 in (1.6 mm) diameter tip at an air pressure of 100 psi (700 kPa).
  An example of a suitable pump is the Alemite Pump 7878-A.
- The ability to consume approximately 22 ft³/min (0.6 m³/min) of compressed air
- A 3/8 in (10 mm) inside diameter hose long enough to reach all areas of the deck
- An adjustable spray gun and tip to provide various patterns of atomized spray or fog for changing finishing conditions
  An example of a suitable spray gun is the Gun Jet No. 43 with a 120-2 Multee Jet Nozzle.

If necessary, substitute other equipment that is capable of equal performance.

500.3.03 Preparation

A. Pre-Pour Conference

Before beginning deck placement operations on each Project, and for individual deck pours of an unusual nature, the Engineer will schedule a pre-pour conference with Project supervisory personnel and a representative of the concrete supplier, if applicable.

Conference topics of discussion include the following:
- Reinforcing steel support method
• Final screed setting check
• Anticipated placement rate
• Personnel number
• Equipment type
• Curing methods
• Adverse weather placement procedures
• Emergency procedures
• Other Work-related details

500.3.04 Fabrication

A. Measure Materials

Measure materials as follows:

1. **Cement.** Weigh bulk cement on scales to plus or minus one percent of the designated weight. If the Engineer allows bag cement, proportion the batch to use only whole bags.

2. **Aggregates.** Weigh all aggregates on scales to plus or minus two percent of the designated weight. Apply the proper corrections for aggregate surface moisture.

3. **Water.** Measure water by volume or weight to within plus or minus one percent.
   a. Construct the measuring system to be independent of water pressure fluctuation.
   b. Ensure that measuring systems have outside taps and valves to facilitate plant calibrations.
   c. You may use recycled wash water provided that it meets the requirements of Subsection 880.2.02.

4. **Admixtures.** Measure admixtures by weight or volume within plus or minus three percent of the required amount.

B. Control Concrete Batching

Control batching as follows:

1. Mix batches of concrete according to the proportions of an approved mix design.

2. Ensure that concrete materials are from the designated sources.

3. Correct the batch weights to account for surface moisture in aggregates.


C. Prestressed Concrete Deck Panel Requirements

Do not use prestressed concrete deck panels unless approved by the Engineer.

D. Add Admixtures to Concrete

Additives are required when specified herein or as directed by the Engineer.

1. **Air-Entraining Admixtures**
   a. All bridge structure concrete uses air-entraining additives, except for seal concrete and non-exposed footings.
   b. The Contractor may use air-entraining additives in other concrete to improve workability when job or material conditions dictate.

   When using air-entraining additives as an option to improve workability or when required, do not exceed the upper limit of the entrained air content requirement in the Table 1—Concrete Mix Table.

2. **Retarding Admixtures**

   Use concrete-retarding additives in bridge concrete when the average temperature is above 65 °F (18 °C) (the average of the expected high and the predicted low).

   a. Normally, concrete-retarding additives are not required for bridge curbs, handrails, crosswalks, or other appurtenances constructed separately from the decks.
   b. The Engineer may waive the use of retarders in substructure concrete when concrete can be placed within one hour after batching.

3. **Water-Reducing Admixtures**
The Contractor may use water-reducing admixtures in Class AA concrete for bridge decks when conditions do not require a retarder. The Contractor may use water-reducing admixtures in other concrete when job or material conditions dictate a reduction in water requirements or when minimal set retardation is desired. The laboratory may allow Type F water-reducing admixtures when the Contractor requests it. The Contractor may construct bridge sections containing duct enclosures for stressing tendons with concrete using Type F (AASHTO M 194) water reducer as approved by the laboratory.

4. Fly Ash
The Contractor may use fly ash as an additive in concrete to promote workability and plasticity. The Contractor may use fly ash as a partial replacement for Portland cement in concrete if the following limits are met:
   a. Replace no more than 15 percent of the cement by weight.
   b. Replace cement with fly ash at the rate of 1.0 to 1.5 lbs (1.0 to 1.5 kg) of fly ash to 1.0 lb (1.0 kg) of cement.
   c. Ensure that the fly ash mix meets the requirements of Subsection 500.1.03.A, Subsection 830.2.03, “Portland Pozzolan Cement” and Subsection 831.2.03.A, “Fly Ash”.
   d. Calculate water-cement ratio based on the total cementitious material in the mix including fly ash.
   e. Do not use Type IP cement in mixes containing fly ash.

5. Granulated Iron Blast-Furnace Slag
If high-early strengths are unnecessary, the Contractor may use granulated iron blast-furnace slag as a partial replacement for Portland cement in concrete if the following limits are met:
   a. Replace no more than 50 percent of the cement by weight.
   b. Replace the cement with slag at the rate of 1.0 lb (1.0 kg) of slag to 1.0 lb (1.0 kg) of cement.
   c. Ensure that the slag mix meets the requirements of Subsection 500.1.03.A.3, Subsection 830.2.02, “Portland Blast-Furnace Cement” and Subsection 831.2.03.A.3, “Granulated Iron Blast-Furnace Slag”.
   d. Calculate the water-cement ratio based on the total cementitious material in the mix including granulated iron-blast furnace slag.
   e. Do not use Type IP cement or fly ash in slag mixes.

E. Mix Concrete

1. Central-Mixed Concrete
   Mix central-mixed concrete as follows:
   a. Establish the mixing time.
      The Engineer will determine the mixing time for central mixed concrete, but the minimum mixing time will be one minute for stationary mixers of up to 1 yd³ (1 m³) capacity. Mixing time may be adjusted in the following situations:
      - The Engineer will increase the minimum time by 15 seconds for each additional cubic yard (meter) or fraction thereof.
      - For mixers with a capacity above 3 yd³ (2 m³), the minimum mixing time may be 90 seconds if the resulting mixture is homogeneous and meets the requirements of Subsection 500.3.02.D.1.c, “Mixer Performance Test.”
      - The Engineer may waive mixing time requirements for stationary mixers of improved types or new designs that produce homogeneous concrete in less time than that established for a particular capacity by the foregoing. For these types of mixers, the Engineer may establish a minimum mixing time of one minute.
   b. Start the mixing time when all cement and aggregates have been placed in the mixer.
   c. Add some water to the mixer before adding the cement and aggregates, but ensure all water is in the mixer by the end of the first 1/4 of the specified mixing time.

2. Shrink-Mixed Concrete
   Mix shrink-mixed concrete as follows:
   a. Mix the batches as specified in Subsection 500.3.02.D.2, “Mixers and Agitators.”
   b. Do the initial mixing in a stationary mixer for at least 30 seconds to thoroughly mix the ingredients. Do the final mixing in truck mixers.
c. Discharge all concrete before the drum or blades exceed 300 revolutions.
d. Do not allow truck mixing at mixing speed to exceed 100 drum or blade revolutions except as allowed when adding water according to Subsection 500.3.05.M, “Add Water to Concrete.”

3. Transit-Mixed Concrete
Mix transit-mixed concrete as follows:

a. For concrete mixed completely in a truck mixer, place all concrete ingredients into the mixer at the concrete plant except the quantity of water that may be withheld according to Subsection 500.3.05.M, “Add Water to Concrete.”
b. After loading the truck, begin operating at either agitating or mixing speed; however, start the mixing speed within 30 minutes after loading the truck mixer.
c. Mix the concrete for 70 to 150 revolutions at mixing speed.
   For revolutions above those specified for mixing speed, use agitating speed.
d. Discharge all concrete before exceeding 300 drum or blade revolutions.

F. Concrete Used in Construction

1. Requirements
Use Type I or Type II Portland cement or Type IP Portland-Pozzolan cement for bridge construction, unless otherwise specified.

NOTE 1: Do not use air-entraining cement.

NOTE 2: Do not use accelerators (24-hour accelerated strength concrete) that contain chlorides in any bridges where the concrete containing the additive will contact the reinforcing steel.

a. Concrete Types: Use the tabulated results from the Table 1—Concrete Mix Table for the classes and specific requirements for each class of concrete. Use the appropriate class of concrete shown in the Plans or Specifications for each component of a structure, of the type as follows:
   • Class AAA–Prestressed concrete
   • Class AA1–Precast concrete as called for on the Plans
   If approved by the Engineer, you may use this class as high early-strength concrete and may use Type III cement in concrete used for this purpose.
   The Engineer may also specify the rate of compressive strength development when this concrete is used

NOTE: The Department will not add compensation to the Contractor for Class AA1 concrete when it is used at the request of the Contractor.

b. Class AA–Bridge superstructure concrete or precast concrete as called for on the Plans

c. Class A–General purposes

NOTE: Do not air-entrain Class A concrete deposited in water (seal concrete). Ensure that the concrete has 10 percent additional cement and sufficient water to provide a 6- to 8-in (150- to 200-mm) slump.

d. Class B–Massive sections or lightly reinforced sections or miscellaneous non-structural concrete

e. Class CS–(Portland cement concrete subbase). Use this class as a subbase where required by the Plans. Concrete subbase may be composed of a mixture of Portland cement and graded aggregate or Portland cement, aggregate, and sand.

2. Acceptance of Design
Determine laboratory acceptance strength by at least 8 compressive test specimens prepared and cured according to AASHTO T 126.

a. Make the specimens from two or more separate trial batches.
b. Make an equal number of specimens from each batch.
c. Calculate the minimum average strength or acceptance strength (X) as follows:
\[ X = \gamma c + 2.0s \]

Where:

\( \gamma c \) = required minimum compressive strength for each class of concrete from the Table 1—Concrete Mix Table

\( s \) = average standard deviation of all 28-day specimens made in the field representing concrete of a given class from all ready-mix plants

Use the standard deviations shown in Table 4:

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<thead>
<tr>
<th>Class of Concrete</th>
<th>Standard Deviation (s)</th>
</tr>
</thead>
<tbody>
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<td>Psi</td>
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<tr>
<td>B</td>
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<td>A</td>
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<tr>
<td>AA1</td>
<td>540</td>
</tr>
<tr>
<td>AAA</td>
<td>500</td>
</tr>
</tbody>
</table>

500.3.05 Construction

A. Meet General Responsibilities

General construction responsibilities include:

1. Batch, mix, deliver, and place concrete according to the Specifications.
2. Have enough production and placement capacity to continuously mix, place, and finish the concrete in each pour unit during daylight hours.
   If necessary, place concrete at night when adequate lighting facilities exist and the Engineer approves of the operations and facilities.
3. If a pour cannot be completed, do the following:
   a. Form an approved construction joint.
   b. Remove the partial pour.
   c. Take other remedial measures directed by the Engineer at no additional expense to the Department.

B. Construct Falsework

Accept responsibility for the design, construction, protection, and performance of falsework. Repair or remove and replace (as the Engineer directs) concrete, other material, or portions of the structure that are damaged or destroyed due to falsework failure.

Construct falsework for prestressed post-tensioned concrete structures according to the Contract Special Provisions.

Construct falsework for structures other than post-tensioned box girders as follows:

1. Meet Design Criteria

   Ensure that falsework structural components that have similar functions in an individual permanent span have the same geometric properties and are made of the same materials.

   When designing and centering formwork, treat concrete as a liquid, and use the following weights:
   - 150 lbs/ft³ (23.6 kN/m³) for vertical loading
   - 85 lbs/ft³ (13.4 kN/m³) for horizontal loading
   - 75 lbs/ft² (3.6 kN/m²) live load for deck placement operations

   Use the following falsework design criteria:
   - Design and construct falsework logically so the Bridge Design Office can analyze it using a commonly accepted structural design theory.
   - Avoid exceeding safe working values for material stresses.
• Provide support for the imposed loads, without settling or deforming and a way to compensate for settlement, if it occurs.

2. Support Falsework
Support falsework using one of these methods:
• Support on piling driven and removed as directed
• Found on a footing approved by the Engineer

3. Construct Falsework
Construct and set falsework to provide the finished structure the specified camber and finished grade. Place “telltales” at locations directed by the Engineer to observe how much the falsework settles.

C. Meet Form Design Criteria
Ensure that forms meet the following design criteria:
• Provide wet concrete and other loads and forces of construction support without bulging between the supports or bracing and without deviating from the lines and contours shown on the Plans.
• Meet the design criteria for falsework in Subsection 500.3.05.B.1, “Meet Design Criteria.”
• Account for the use of retarded concrete.

Ensure that bracing, ties, and supports are placed accurately.

If the formwork appears to be inadequately supported, tied, or braced (before or during concrete placement), the Engineer may require that the Work stop until the defects are corrected.

D. Use Acceptable Form Materials
Except as noted, fabricate forms from the following materials:
• Lumber
• Plywood
• Metal
• Plastic
• Combinations of these

Use material free of defects that materially affect form strength or materially impair the accuracy or appearance of the concrete surface.

Use the form materials as follows:

1. Lumber Forms
Construct wood forms as follows:
   a. Size and dress the lumber.
   b. Use lumber at least 1 in (25 mm) thick.
   c. Use lumber for header forms used as screed supports and for curb face forms at least 2 in (50 mm) thick.
   d. Avoid using scrap material or doing patchwork.
   e. Stagger all joints but those between abutting panels.
   f. Line the lumber used to form outside vertical surfaces of exterior beams or girders with an approved form liner.
   g. Use chamfer strips mill-produced from high-quality lumber, free of defects.
   h. Dress and finish chamfer strips on all three sides.
   i. Size chamfer strips to the proper dimensions.

2. Plywood Forms
Construct plywood forms as follows:
   a. If plywood is the type made for general concrete forms and is at least 5/8 in (16 mm) thick, use it in place of 1 in (25 mm) thick lumber to construct forms, if necessary.
   b. Ensure that plywood used to form open joints and to line forms is at least 1/4 in (6 mm) thick.
c. When nailing plywood directly to form studs, do not space the studs more than 16 in (400 mm) apart.

d. Use plywood in full sheets wherever practical. Do not do patchwork with small, irregular pieces.

e. Have the Engineer inspect and approve plywood sheet layout.

3. Metal or Plastic Forms

Construct metal or plastic forms as follows:

a. Use metal or plastic to form concrete only if the Engineer approves the forms and if the forms produce satisfactory results.

b. Use metal forms that produce finished concrete equal to or superior to concrete made from comparable wooden forms.

c. Countersink bolts and rivets in the surfaces of metal forms that touch concrete.

d. Grind welds smooth in the surfaces of metal forms to provide a smooth plane surface.

4. Other Material Uses

Use tempered fiberboard for form liners when necessary if it is at least 1/4 in (6 mm) thick. Use tempered fiberboard 1/8 in (3 mm) thick only to form open joints. Support the fiberboard with suitable spacers arranged properly.

Use approved synthetic materials for forming open joints and for other special uses, if necessary.

E. Construct Form Supports

Construct form supports using metal ties, anchors, and hangers as follows:

1. Construct supports that will remain in the finished concrete so they can be removed from the concrete face to a depth of at least 1 in (25 mm) without damaging the concrete.

2. Weld form supports to girder or beam flanges in continuous or cantilever spans only in the flange areas which are in compression.

3. When ordinary wire ties or snap ties are permitted, cut them back at least 3/8 in (10 mm) from the face of the concrete.

4. Design metal tie fittings that minimize the cavities made when they are removed. Fill all cavities after removing metal tie fittings.

F. Construct Temporary Forms

Construct temporary forms as follows:

1. Construct and maintain forms in a mortar-tight condition.

2. Construct forms so that they can be removed easily without damaging the concrete, unless using forms that will remain in place.

3. Build, line, and brace forms so that the formed concrete surface conforms with the dimensions, lines, and grades shown on the Plans.

4. Build headwall forms for skewed pipe parallel to the roadway centerline or at right angles to the radius on curves. Construct headwall forms as follows:
   a. Lay enough pipe to extend through the headwall form.
   b. After the concrete is poured and hardened, carefully cut and dress the protruding pipe ends so no ragged edges remain.

The Contractor may choose, as an alternate to the above method, to build a circular form that exactly fits the pipe circumference and face of the headwall form.

5. Construct form liner using plywood or other approved form liner as follows:
   a. Use form liner in large sheets. Do not do patchwork.
   b. Avoid irregular joint location in form liners.
   c. Have the Engineer inspect and approve the proposed liner layout.

6. Bevel forms at beam copings, girders, and other projections to ease removal.

7. Place chamfer strips to chamfer exposed edges of the concrete by the required amount. Use ¾ in (19 mm) chamfers unless otherwise shown on the Plans.
8. Patch with tin or other metal only in those areas of the superstructure lying between and including the inside faces of the exterior beams.


10. Immediately before erecting forms or just before placing bar reinforcement steel, coat forms with a clear oil or other bond breaker to keep concrete from sticking to the forms.
   a. Do not allow the substance to stain or soften the concrete surface.
   b. Do not apply by reaching or pouring through previously placed reinforcement steel.

11. Wait to place concrete in any form until the Department inspects and approves the form. Inspection and approval does not diminish the responsibility to produce concrete surfaces free of warping, bulging, or other defects.

12. When removing forms, remove chamfer strips, blocks, and bracing.

13. Do not leave any part of a wooden form in the concrete.

14. If concrete surfaces do not meet finish specifications, correct the problems with the following steps, as directed by the Engineer:
   - Repair the defects using approved methods.
   - Remove and replace the affected portion of the Work.

G. Reuse Forms

Reuse forms and form material in good condition and satisfactory as determined by the Engineer. Do not use forms or form materials that are warped, cracked, split, bulging, have separated plies, or have unsatisfactory form liner.

Ensure that used forms are mortar tight and produce a finished concrete equivalent to that produced by new forms.

H. Construct Permanent Steel Bridge Deck Forms for Concrete Deck Slabs

Unless otherwise designated on the Plans, construct and use permanent steel bridge deck forms for concrete bridge deck slabs according to these Specifications. Do not use permanent steel bridge deck forms in panels where longitudinal deck construction joints are located between stringers.

Provide a structurally satisfactory slab when using permanent steel bridge deck forms.

1. Fabricate permanent steel bridge deck forms and supports from steel that conforms to ASTM A 653/653M Designation SS, Grade 80/550, Coating Designation G-165/Z-500 and ASTM A 924/924M.

2. Design permanent steel bridge deck forms as follows:
   a. Account for the dead load of the following:
      - Form
      - Reinforcement steel
      - Plastic concrete
   b. Add 50 lbs/ft² (2.4 kN/m²) for construction loads.
   c. Ensure that the unit working stress in the steel sheet does not exceed 0.725 of the specified minimum yield strength for the material furnished. However, do not allow the unit working stress to exceed 36,000 psi (250 MPa).
   d. Account for deflection under the weight of the forms, the plastic concrete, and the reinforcement as follows:
      1) If deflection exceeds 1/180 of the design span or 1/2 in (13 mm), whichever is less, use intermediate supports.
      2) Do not base deflection on a total load of less than 120 lbs/ft² (5.7 kN/m²).
   e. Base the permissible form camber on the actual dead load condition.
   f. Do not use camber to compensate for deflection that exceeds the above limits.
   g. Compute the form sheets design span using the clear span of the form, plus 2 in (50 mm), measured parallel to the form flutes.
   h. Compute physical design properties according to the requirements of the latest published edition of the American Iron and Steel Institute Specification for the Design of Cold Formed Steel Structural Members.
i. Ensure that all bottom reinforcement has a minimum concrete cover of 1 in (25 mm) as shown in Figure 1 (Figure 1 metric).

![Figure 1](image1.png)

**Figure 1**

**Figure 1 (metric)**

j. Maintain the Plan dimensions of both layers of primary deck reinforcement from the top surface of the concrete deck.

k. Do not use precast mortar blocks to support the deck reinforcement.

l. Do not treat permanent steel bridge deck forms as lateral bracing for the compression flanges of supporting structural members.

3. Do not weld to flanges in tension or to structural steel bridge elements fabricated from non-weldable steel grades. Have welders certified by the Department weld metal deck forms or supports for metal deck forms.

I. Install Forms

Install and maintain forms in a mortar-tight condition and according to approved fabrication and erection Plans.

Place transverse construction joints at the bottom of a flute. Field drill 1/4 in (6mm) weep holes no less than 12 in (300 mm) on center along the line of the joint.

1. Highway Bridge Forms

   Install highway bridge forms using either Method 1 or Method 2:

   - **Method 1.** Place forms so the ribs of the forms align with how the bottom transverse reinforcing in the slab is spaced.
   - **Method 2.** Place forms with a 1 in (25 mm) minimum clearance between the top of the form and the bottom of the main deck reinforcement. See Figure 1 (Figure 1 metric).
2. Railroad Bridge Forms

Install railroad bridge forms as follows:

a. Place the forms so the tops of the form ribs adjacent to the beam flange are at the bottom of the deck slab specified by the Plans.

b. Maintain the full slab depth detailed on the Plans.

c. Do not allow form ribs to project above the Plan bottom of the deck slab.

d. Do not place form sheets directly on top of the stringer or floor beam flanges.

e. Securely fasten form sheets to form supports using self-drilling screw fasteners, not by welding. If the Engineer approves, use fastener pins driven into place by a power tool.

f. Ensure that form sheets have a minimum bearing length of 1 in (25 mm) at each end.

g. Do not leave loose sheets or accessories on the deck at the end of a day’s work.

h. Place form supports so that they contact the flange of the stringer or floor beam.

i. Attach form supports using welds, bolts, clips, or other approved means.

j. Do not weld form supports to the flanges of non-weldable steel or to portions of the flange subject to tensile stresses.

k. Ensure that welding and welds comply with AWS D 2.0 for fillet welds. However, 1/8 in (3 mm) fillet welds are permitted.

J. Repair Damaged Forms

Repair permanently exposed form metal to the Engineer’s satisfaction if the galvanized coating is damaged.

1. Clean the damaged area.

2. Go over the damaged area with a wire brush.

3. Paint the area with two coats of zinc oxide-zinc dust primer that meet Federal Specification TT-P-641d, Type II and has no color added.

4. Do not touch up minor heat discoloration in weld areas.

K. Construct Runways

Provide runways into a deck pour area for moving buggies. If the Engineer approves, use runways to bridge a previous pour that has not reached the minimum strength or age requirements in Subsection 500.3.05.AF.4, “Live Loads—Pouring Equipment.”

Construct and support runways to protect the forms and the reinforcement steel position.

L. Construct Work Bridges

Provide a work bridge on deck pours. Support the bridge outside the area of the pour receiving a surface finish. If two or more spans will be poured on the same day, the Engineer may require two work bridges.

Design and construct work bridges to meet the following:

- Do not allow the bridge to sag into the fresh concrete.

- Construct the bridge so that transverse finish and curing material can be applied easily regardless of the screed type.

M. Add Water to Concrete

Add water to the concrete at the concrete plant. Do not add indiscriminate amounts of water at the job site.

If placement conditions require concrete of a more workable consistency, add small amounts of water at the job site if approved by the Engineer.

Add water at the job site as follows:

1. Determine the quantity of water required to provide the necessary consistency.

The Engineer will not approve additions of water that cause the total amount of water to exceed the maximum water/cement ratio established in the Table 1—Concrete Mix Table.
The Engineer will reject concrete with water added to it that produces a higher slump than specified in the Table 1—Concrete Mix Table.

2. Do not add water to concrete that has begun to set because of excessive mixing or to concrete that has exceeded mixing or haul time limitations.

3. When adding the water, carefully control the conditions.

4. Position the delivery so the measuring operation is not affected.

5. Measure the water carefully.

6. Inject the water into the mixer forcefully to facilitate uniform mixing.

7. Add water before discharging an appreciable amount of concrete.

8. Do not add more water after concrete discharge begins.

9. After adding the water, mix the concrete an additional 30 revolutions.

10. Finish mixing the concrete before the total revolutions at mixing speed exceed 150.

N. **Volumetrically Proportion Concrete**

Concrete ingredients may be proportioned volumetrically when non-air entrained concrete is used in miscellaneous concrete, non-exposed footings, or culverts smaller than bridge culvert size.

O. **Prepare for Concrete Placement**

Prepare for concrete placement as follows:

1. Ensure that an adequate supply of concrete will be furnished and placed to meet the requirements specified in Subsection 500.3.05.P, Table 5—Minimum Placement Rates.

2. To ensure a full bond between prestressed concrete deck panels and the cast-in-place concrete, clean the panel before placing the slab concrete.

3. Immediately before placing cast-in-place slab concrete, saturate the prestressed concrete deck panels with water.

4. Immediately before placing concrete in the forms, the concrete will be measured for acceptance tolerances. Acceptance tolerances for each class of concrete are listed in the Table 1—Concrete Mix Table. Conduct the applicable tests according to the procedures in the Sampling, Testing, and Inspection information.

P. **Meet the Minimum Placement Rates**

If concrete is not produced, placed, and finished according to the minimum placement rates, the Engineer may reject the pour. Concrete pours of a similar nature and size will not be allowed until the problem is corrected and the placement rate met.

The minimum placement rates are listed in Table 5:

**Table 5—Minimum Placement Rates for Bridges, Culverts and Retaining Walls**

1. Bridge Substructure

<table>
<thead>
<tr>
<th>Pour Size in Cubic Yards (Meters)</th>
<th>Minimum Placement Rate in Cubic Yards (Meters) per Hour</th>
</tr>
</thead>
<tbody>
<tr>
<td>0-25 (0-19)</td>
<td>10 (8)</td>
</tr>
<tr>
<td>26-50 (20-39)</td>
<td>15 (12)</td>
</tr>
<tr>
<td>51-75 (40-59)</td>
<td>20 (15)</td>
</tr>
<tr>
<td>76-100 (60-75)</td>
<td>25 (20)</td>
</tr>
<tr>
<td>101 and over (76 and over)</td>
<td>30 (25) or as designated on the Plans or in the Special Provisions</td>
</tr>
</tbody>
</table>

2. Bridge Superstructure

<table>
<thead>
<tr>
<th>Pour Size in Cubic Yards (Meters)</th>
<th>Minimum Placement Rate in Cubic Yards (Meters) per Hour</th>
</tr>
</thead>
<tbody>
<tr>
<td>0–25 (0-19)</td>
<td>15 (12)</td>
</tr>
<tr>
<td>26–50 (20–39)</td>
<td>20 (15)</td>
</tr>
</tbody>
</table>
Pour handrail, parapet, curb, and barriers at a rate satisfactory to the Engineer.

3. Culverts

<table>
<thead>
<tr>
<th>Structure</th>
<th>Minimum Placement Rate in Cubic Yards (Meters) per Hour</th>
</tr>
</thead>
<tbody>
<tr>
<td>Footings and slabs</td>
<td>Same as for bridge substructures</td>
</tr>
<tr>
<td>Sidewalls and wingwalls</td>
<td>Use placement rates so that fresh concrete is not placed on concrete that has attained its initial set. Cover all concrete with fresh concrete within 45 minutes.</td>
</tr>
</tbody>
</table>

4. Retaining Walls

<table>
<thead>
<tr>
<th>Structure</th>
<th>Minimum Placement Rate in Cubic Yards (Meters) per Hour</th>
</tr>
</thead>
<tbody>
<tr>
<td>Footings</td>
<td>Same as for bridge substructures</td>
</tr>
<tr>
<td>Walls</td>
<td>Same as for culvert sidewalls and wingwalls</td>
</tr>
</tbody>
</table>

Q. Place Concrete

Place concrete as follows:

1. Do not allow aluminum to touch the concrete while mixing, transporting, handling, or placing it.
2. Transport, handle, and place concrete quickly so that it reaches its final position in the forms within the haul time limitations in Subsection 500.2.01.E.1, “Haul Time Limitations.”
3. Manipulate the delivery or conveyance unit to avoid vibration damaging to partially set concrete.
4. Immediately before placing the concrete, thoroughly clean and wet the forms.
5. Place concrete as close as possible to its final position in the forms.
6. Use chutes, troughs, or tubes to pour the concrete in the forms, without displacing reinforcement steel.
7. Modify or stop using the equipment if chutes, troughs, or tubes cause honeycombed or otherwise inferior concrete.
8. When placing concrete by pumping, operate the pumping equipment so that the concrete is produced in a continuous stream without air pockets.

NOTE: Convey and place concrete by pumping only when specified in the Contract or when authorized by the Engineer.

9. When concrete placement requires dropping the concrete more than 5 ft (1.5 m), use pipes or tubes to place the concrete.
   Do not allow concrete to free-fall more than 5 ft (1.5 m) from the pipe or tube.
10. Place concrete in horizontal layers no more than 18 in (0.5 m) thick.
11. Place and compact succeeding batches in each layer before the preceding batch takes its initial set.
12. Place each succeeding layer before the underlying layer sets.
13. Consolidate the concrete to avoid cold joints between layers.
14. If the forms sag or bulge while concrete is being placed, remove the concrete causing the distortion and the concrete in adjoining areas if the Engineer requires. Removal prevents cold joints and displaced or damaged reinforcement.
15. Work the concrete around reinforcement bars without displacing them.
17. Vibrate concrete where it is deposited and vibrate other concrete while it is fresh. Vibrate as follows:
   a. Insert and withdraw vibrators slowly.
   b. Manipulate vibrators to work the concrete around reinforcement and embedded fixtures and into corners of forms.
c. Vibrate sufficiently to compact the concrete but avoid causing the concrete to segregate.

d. Stop vibrating before local areas of grout are formed.

e. Apply vibrators no farther apart than twice the radius through which the vibration is visibly effective.

f. Do not use vibrators or any other means that could cause segregation to move masses of concrete in the forms.

g. Do not apply vibrators to sections of concrete that are no longer plastic.

h. Vibrate concrete-filled steel grid floors by applying the vibrators to the steel.

i. Vibrate concrete for precast or prestressed units as specified above in steps a through g, unless the Engineer approves alternate methods.

j. Stop vibration when a mortar line appears on the face of the form and when the coarse aggregate particles are submerged in the concrete mortar.

18. Supplement vibration with spading to ensure smooth surfaces and dense concrete along form faces and in locations difficult to reach with vibrators.

19. After concrete sets initially, do not disturb the forms or the projecting reinforcing bars.

R. Create Construction Joints

Place construction joints according to the Plans or as directed by the Engineer.

If an emergency affects continuous placement, the Engineer will decide if a construction joint is allowed. If allowed, the Engineer will provide instructions about where and how to make the joint.

The Engineer may eliminate certain construction joints if placement, finishing and forming methods can produce satisfactory results.

Create construction joints as follows:

1. Remove mortar splashed on form surfaces and projecting reinforcement steel before concrete reaches its initial set.
   a. Do not puddle dried mortar chips and dust into the plastic concrete.
   b. If excess mortar is not removed from reinforcement steel before the concrete reaches its initial set, delay cleaning until the concrete is thoroughly hardened.

2. If joining fresh concrete and hardened concrete, clean the hardened surface of laitance and incompletely bonded, loose, or foreign material.

   Ensure that laitance is completely removed from the following:
   - Joints between decks and curbs
   - Tops of seal courses
   - Construction joints in concrete exposed to sea water

3. Ensure that the surface of the concrete is dry before pouring the concrete against it.

4. Immediately before placing fresh concrete, tighten the forms against the existing concrete.

5. Use tremies or pumps to coat areas where fresh concrete will be poured with mortar or cement grout.

6. Begin placing concrete immediately after placing the mortar or grout.

7. Apply enough vibration to blend the material with the concrete at the construction joint.

S. Protect Fresh Concrete

Do not drive pile, blast, or perform other operations that vibrate the formwork or the concrete noticeably before the concrete reaches a strength of 2,000 psi (15 MPa) and is 3 days old.

Protect fresh concrete from rainfall with waterproof material such as tarpaulins or plastic film. Ensure that the waterproof material is ready before pouring and is sufficient to cover the area of the pour.

T. Place Bridge Deck Concrete

Do not use calcium chloride or any other admixture containing chloride salts in concrete placed on permanent steel bridge deck forms.

Ensure that the tolerances are accurate for bar reinforcement placement in cast-in-place concrete so the top clearance to the bar reinforcement complies with Subsection 511.3.05.G.6, “Bridge Deck Slab Tolerances.”
Place bridge deck concrete according to the Contract Specifications and as follows:

1. Before pouring decks, set substantial bulkheads or headers and shape them to the required deck surface cross section.
2. Ensure that pouring sequences, procedures, and mixes comply with the Plans and Specifications.
3. Pour the deck according to the numbered sequence as follows:
   a. Unless otherwise shown on the Plans, pour each deck in one continuous operation.
   b. When dividing deck pours within any one complete unit (a simple span or a continuous or cantilever unit), pour and finish the concrete in the numbered sequence shown on the Plans, beginning with the lowest number.
   c. Make pours with the same number before pours with higher numbers. Make pours with the same number in any sequence.
      The numbered sequence shown on the Plans also applies to sidewalk pours, but it need not apply to curb, parapet, and handrail pours.
   d. Pour diaphragms between steel or prestressed concrete roadway beams at least 24 hours before pouring the deck slab.
   e. Unless otherwise authorized by the Engineer, pour all diaphragms within a complete unit before pouring decks.
   f. When constructing concrete T-Beams, place girder stems in uniform layers before placing slabs.
   g. If T-Beam spans are supported without intermediate false bents, begin deck placement as soon as the first four stems are placed. After the first four stems, avoid getting more than three stems ahead of the advancing line of the deck pour and lagging by more than the space between stems.
   h. If T-Beam spans are supported by intermediate false bents, place decks and stems the same as for T-Beam spans supported without intermediate false bents. However, ensure that the slab is placed before a cold joint develops between the stem and slab.
4. Do not make the deck pour until any previously poured concrete in the complete unit has set for 24 hours.
   This requirement may be waived under certain conditions if the succeeding pour can be completed (except for final finishing) within four hours of the initial placement of the day. The Engineer must give written approval for this requirement to be waived.
   Unless otherwise shown on the Plans, do not place handrail, sidewalks, parapets, and curbs in a complete unit until all the deck slabs in the unit have been poured.
5. Ensure that the pour is the same as the overlap direction (as shown in the shop drawings).
6. Use the following deck pour method:
   a. If there is super-elevation, begin deck pours on either the high or the low side.
   b. Dump each batch against previously placed concrete.
   c. Pour at a rate that ensures fresh concrete along the advancing line of the pour.
   d. Vibrate or tamp concrete dumped on fresh concrete to make the grout flow as follows:
      • Forward with or slightly ahead of the concrete
      • Along the bottoms and sides of the forms
      • Around the reinforcement steel
7. Once the concrete is poured, vibrate it enough to avoid honeycomb and voids, especially at the following locations:
   • Construction joints
   • Expansion joints
   • Valleys and ends of form sheets
   Scrub the concrete as follows:
   a. Use finishing devices operating parallel to the center line. As pouring proceeds, keep the concrete surface scrubbed to the required grade.
   b. Fill depressions ahead of the scrub, and keep a small roll of grout on the leading edge of the scrub. Perform further scrubbing with minimum disturbance to the surface already brought to the grade.
   c. Take care during the placement and scrubbing to obtain sound concrete at the construction joint located where the slab joins the curb, parapet, or sidewalk.
   d. Do not place excess grout on the leading edge of the scrub and do not allow it to remain in this area.
   e. Use either a longitudinal scrub or a transverse scrub.
• **Longitudinal Screed**
  Before doing the final screeding, place enough concrete in front of the screeding position to deflect the dead load.

• **Transverse Screed**
  On beam or girder-supported spans with skew angles of 65° or less, place and operate the truss or beam supporting the strike-off parallel to the skew and make the advancing pour line parallel to the skew.
  On beam or girder-supported spans with skew angles between 65° and 90°, position the screed either on the skew or at right angles to the bridge center line.
  On superstructures supported by non-deflecting falsework and on beam- or girder-supported spans with a total dead load deflection no more than 1/2 in (13 mm), position the screed at right angles to the bridge center line and make the advancing line of pour at right angles to the bridge center line.

  f. As the pouring proceeds, keep the concrete surface screeded to the required grade.
  g. Fill depressions ahead of the screed. Keep a small roll of grout on the leading edge of the screed.
  h. Continue to screed without disturbing the surface already brought to the required grade.
  i. Avoid producing unsound concrete where the slab joins the curb, parapet, or sidewalk. Remove excess grout from the leading edge of the screed at these construction joints.

8. Edge joints to be sealed, including dummy joints, as follows:
   a. Edge before the initial set or after the final set.
   b. If edging before the initial set, use edging tools of the proper radius as shown on the Plans.
   c. Carefully remove concrete from pouring operations on adjacent pours to achieve the required rounded edge.
   d. If edging after the final set, allow the joints to harden. After at least 12 hours, grind joints to approximate the plan radius either by hand or by mechanically operated grinding stones.
   e. To achieve full and uniform bearing, finish areas that are recessed for receiving joint members.

9. Finish bridge decks as follows:
   a. As soon as the concrete is hard enough and standing water and moisture sheen disappear, give the concrete a final finish by belting, brooming, or dragging.
      • Belt longitudinally using a wet canvas belt. Limit belting to spans no longer than 40 ft (12 m).
      • Drag transversely or longitudinally with a wet burlap drag.
      • Broom transversely using a stiff-bristled broom.
   b. Finish the following areas carefully:
      • Gutter lines
      • Joints
      • Drains
   c. After belting, dragging, or brooming and when shown on the Plans, groove the bridge deck and approach slabs perpendicular to the center line as follows:
      1) Do not begin grooving until the bridge deck is cured according to Subsection 500.3.05.Z, “Cure Concrete.”
      2) If necessary, groove in conjunction with planing required to make the surface corrections specified in Subsection 500.3.06.D, “Bridge Deck Surface Check.” Wait until the concrete is hard enough to support the equipment without distorting.
      3) Cut grooves into the hardened concrete using a mechanical saw device capable of producing grooves 0.125 in (3 mm) wide, 0.125 in (3 mm) deep, and 0.750 in (19 mm) apart, center-to-center.
      4) Extend the grooves across the slab to within 1ft (300 mm) of the gutter lines.

U. **Place Concrete Parapet on Bridge Decks**

Place concrete barrier or parapets on bridge decks. The slip form method with an approved self-propelled extrusion machine as specified in Section 621 is optional.
V. Place Seal Concrete

Deposit concrete in water only when required by the Plans or when considered necessary by the Engineer.

When depositing the seal concrete, follow these guidelines:

- Keep the water as motionless as possible.
- Place the concrete continuously from beginning to end.
- Ensure that the concrete surface remains as horizontal as possible.

Place seal concrete as follows:

1. Place seal concrete carefully in a compacted mass as near to its final position as possible using a tremie, a bottom dump bucket, or other approved means.
   a. Use tremies to place seal concrete as follows:
      1) Support tremies so that the discharge end can move freely over the entire top surface of the work.
      2) Support tremies so that they can lower rapidly to stop or retard the flow of concrete.
      3) At the beginning of the work, close the discharge end to keep water out of the tube.
      4) Keep the tube sealed.
      5) Keep the tremie tube full to the bottom of the hopper.
      6) When dumping a batch into the hopper, induce concrete flow by slightly raising the discharge end and keeping it within the previously deposited concrete. This maintains a seal and forces the concrete to flow into position by hydraulic head.
   b. Use bottom-dump buckets to place seal concrete as follows:
      1) Ensure that the bottom-dump bucket is level full.
      2) Open the bucket only when it rests on the surface that will receive the charge.
      3) In lowering and raising the bucket, do not move the water unnecessarily.
   c. When approved by the Engineer, place seal concrete by pumping.

2. Wait at least 24 hours after placement to begin dewatering seal concrete, unless the Engineer determines a longer waiting period is necessary.

3. Remove laitance from the seal concrete before placing the footing.

4. Bore seals under spread footings the entire depth of the seal as specified for foundations in Subsection 211.3.05.C, “Boring of Foundations and Seals.”

5. If laitance buildup on seals under spread footings exceeds 1/4 in/ft (20 mm/m) of seal depth, the Engineer may decide to core the seal to determine acceptability.

6. When placing concrete exposed to sea water, control the water content to produce concrete of maximum density and create construction joints and prepare their surfaces according to the requirements of Subsection 500.3.05.R, “Create Construction Joints.”

W. Pour CS Concrete

Pour CS concrete as follows:

1. Meet CS concrete depth and surface finish requirements.
   - Ensure that the minimum depth is the same as shown on the Plans.
   - Do not vary the depth variation more than 1 in (25 mm).
   - Ensure that the surface finish is generally smooth and uniform.
   - Smooth or fill float marks, voids, and other deformities exceeding 1/2 in (13 mm) before placing approach slabs.

2. To prevent bonding:
   a. Lay clean polyethylene sheeting uniformly over the CS concrete in the approach slab area before placing the slabs.
   b. Use new, unused polyethylene sheeting free of holes, rips, and tears.
c. Use polyethylene bond-breaking material at least 8 mils (0.2 mm) thick with an overlap of at least 6 in (150 mm).

3. Maintain polyethylene sheeting in good condition throughout the construction process.
   Repair or replace sheeting deemed unsatisfactory as directed by the Engineer.

4. Cure CS concrete with the polyethylene sheeting used for bond breaking.

X. Pour Concrete in Cold Weather

When pouring concrete in cold weather, keep the concrete temperature at the point of delivery at least 50 °F (10 °C). Do not use accelerator-containing chlorides.

Mix and pour concrete in cold weather as follows:

1. Keep concrete materials at the right temperatures.
   - Do not use materials in concrete mix that contain frozen lumps.
   - Do not incorporate water and aggregates into the mix with temperatures more than 150 °F (65 °C).
   - If aggregates or water temperatures are above 100 °F (40 °C), discharge the aggregates and water into the mixer and allow the temperatures to equalize before adding the cement.
   - Heat aggregate with steam, hot water coils, or other methods that do not damage the aggregates. Do not heat aggregates with direct flame.

2. Protect the poured concrete.
   - Keep concrete above 50 °F (10 °C) for at least 72 hours after placement.
   - Protect concrete from freezing for 6 days after placement.

Y. Pour Concrete in Hot Weather

Reduce hazards and difficulties related to placing and finishing concrete in hot weather before pouring. The Engineer may require measures to prevent concrete workability reduction, losses from cement hydration, evaporation, drying, or elevated concrete temperatures.

1. Place Concrete
   Cool forms and reinforcement with water immediately before placing concrete. Meet the minimum placement rates specified in Subsection 500.3.05.P, Table 5—Minimum Placement Rates.

2. Keep Concrete Cool
   Keep concrete cool as follows:
   a. Keep the concrete used for construction at no more than 90 °F (32.2 °C) when measured at the point of discharge from the delivery unit.
   b. If the concrete temperature might exceed 90 °F (32.2 °C) during concrete placement, begin placement when the air temperature cools if the Engineer requires.
   c. Cool the aggregates by fogging or other means that do not affect moisture content.
   d. Use chipped or crushed ice in the mix as a portion of the mixing water on a pound (kilogram) basis. If using ice, ensure that the ice melts before the batch is discharged from the mixing unit.
   e. If necessary, cool water by refrigeration to provide a lower concrete temperature.

3. Finish Concrete
   Do not “splash on” water to aid screeding or finishing operations.
   For bridge decks, fog the surface when required, according to Subsection 500.3.05.Z.3, “Bridge Deck Curing.”
   If needed, use wind screens to prevent thermal or shrinkage cracks caused by rapid concrete surface drying.

Z. Cure Concrete

Concrete curing is an integral part of the concrete placement operation. Improperly cured concrete will be considered defective.

If the Engineer determines that curing procedures do not comply with these Specifications, stop placing concrete. Resume concrete placement after taking remedial measures to ensure proper curing.
Begin curing unformed surfaces when the water sheen disappears from the surface or immediately after applying the surface finish. Continue curing for 5 days.

Cure the formed surfaces after removing the forms. Remove them within 5 days after placing concrete. Continue curing until the concrete is 5 days old (from the time it is poured).

Cure concrete surfaces exposed to air using methods that prevent premature drying or moisture loss. Ensure that curing conditions are the same throughout separate curing areas.

Use either or a combination of the two methods specified for curing concrete except bridge decks. Cure bridge decks as described in Subsection 500.3.05.Z.3, “Bridge Deck Curing.”

1. General Curing—Supplying Additional Moisture
   Do not use a method that causes the concrete to be alternately wet and dry.
   Cure concrete properly by supplying additional moisture through ponding, sprinkling, or fogging and then retaining the moisture as follows:
   a. Use cotton mats, burlap, sand, hay, or straw coverings.
      Cover with at least 2 in (50 mm) of sand. Cover with at least 3 in (75 mm) of hay or straw.
   b. Do not use sawdust or coverings that cause unsightly discoloration of concrete.
   c. Place coverings after completing the finishing operations when there is no danger of surface damage.
   d. Keep coverings moist continuously.

2. General Curing—Preventing Moisture Loss
   Keep concrete moist before and during the rubbing from the Type III—Rubbed Finish.
   Start curing immediately after the rub using approved waterproof paper, plastic sheets, or membrane-forming curing compounds, except when curing compounds are prohibited.
   a. Waterproof Paper or Plastic Sheets
      Ensure that the sheets and paper meet the requirements of AASHTO M 171 and use them as follows:
      • Use the widest possible widths.
      • Lap adjacent sheets at least 6 in (150 mm).
      • Seal the laps with tape, mastic, glue, or other approved methods to form a waterproof cover of the entire area.
      • Keep the curing material from being displaced by wind.
      • Immediately replace or repair sheets or paper that tear, break, or become damaged during the curing period.
   b. Membrane-Forming Curing Compounds
      Use as the curing agent AASHTO M 148, membrane-forming curing compounds, Type 1-D, Class A or B, or Type 2, Class A or B, white pigmented. Use the curing agent as follows:
      • Do not use membrane-forming curing compounds on bridge decks or prestressed concrete bridge members, or in construction joint areas.
      • When the water sheen disappears from the concrete surface, apply the curing compound uniformly to unformed areas.
      • Apply the compound to formed surfaces if the forms are removed during the 5-day curing period.
      • Cure the areas to be rubbed with liquid membrane-forming compounds for curing concrete, Type1-D, Class A or B (non-acrylic).
      • Apply curing compound with fine-spraying equipment.
      • Thoroughly agitate the compounds just before using them.
      • Spray the surface again immediately after the first application at right angles to the first application. Apply at least 1 gal (1 L) for each 150 ft² (3.7 m²) of surface.
      • Do not apply curing compound to the following:
        —Joints where a concrete bond is required
3. Bridge Deck Curing
   Cure bridge deck concrete as follows:
   a. Immediately after the water sheen disappears and the surface finish is applied, fog the surface to keep a film of water on the surface.
   b. If surface damage occurs, delay fogging.
   c. Keep the surface wet until after applying the sheet curing covers.
   d. Thoroughly soak curing covers on the fabric side.
   e. As soon as the concrete sets enough to prevent damage, apply the covers with the white-poly side up.
   f. Use two-layer sheet curing material for bridge concrete according to AASHTO M 171.
      For the bottom layer, use a polyethylene film. For the top layer, use a white, burlap polyethylene sheet or a white, co-polymer-coated, absorbent, non-woven synthetic fabric.
   g. Ensure that sheet curing material for bridge concrete meets Specification requirements for reflection and moisture retention and has no holes or tears.
   h. Use enough sheet curing material to cover the deck surface.
   i. Place the curing covers so that adjoining sheets overlap at least 18 in (450 mm).
   j. Weight all laps and side edges to prevent cover displacement before curing is completed.
   k. Weight and overlap covers so the curing sheets maintain intimate contact with the concrete surface.
   l. If there is no moisture under the curing covers during the 5-day curing period, apply additional moisture.

4. Parapet, Sidewalk, End Post, and Curb Face Curing
   The surface of parapets, sidewalk, end post, and horizontal and vertical faces of curbs are not considered part of the bridge deck. Cure these structures using the general curing methods in Subsections 500.3.05.Z.1, “General Curing—Supplying Additional Moisture,” and 500.3.05.Z.2, “General Curing—Preventing Moisture Loss,” unless the surfaces will receive a special surface coating (Subsection 500.3.05.AB.4, “Type III—Special Surface Coating Finish”).
   Do not cure surfaces receiving a special surface coating with membrane-forming curing compounds.
   Do not cure surfaces receiving protection surface treatment (75 percent boiled linseed oil and 25 percent mineral spirits solution) with membrane-forming curing compounds that contain acrylics.

AA. Prevent Plastic Shrinkage Cracking
   Take precautions to prevent plastic shrinkage cracking of concrete by doing the following:
   • Provide wind screens
   • Provide fogging equipment
   • Apply temporary wet coverings before moisture loss begins
   The Engineer will evaluate the effects of plastic shrinkage cracks and will require repair of cracks that create structural defects and corrode reinforcement steel.

AB. Finish Concrete
   Concrete surface finishes are classified according to whether the surfaces are formed or unformed. Refer to Table 6.
   When other Sections of the Specifications for concrete work state that the requirements of Section 500 apply, finish the concrete according to the other sections.
Table 6—Concrete Finish Types

<table>
<thead>
<tr>
<th>Surface</th>
<th>Finish Type</th>
</tr>
</thead>
<tbody>
<tr>
<td>Formed</td>
<td>Type I—Ordinary Formed Surface Finish</td>
</tr>
<tr>
<td></td>
<td>Type II—Special Formed Surface Finish</td>
</tr>
<tr>
<td></td>
<td>Type III—Rubbed Finish</td>
</tr>
<tr>
<td></td>
<td>Type III—Special Surface Coating Finish</td>
</tr>
<tr>
<td>Unformed</td>
<td>Type IV—Floated Surface Finish</td>
</tr>
<tr>
<td></td>
<td>Type V—Sidewalk Finish</td>
</tr>
<tr>
<td></td>
<td>Type VI—Stair Tread Finish</td>
</tr>
</tbody>
</table>

Except for bridge deck finishes, which are covered in Subsection 500.3.05.T, “Place Bridge Deck Concrete,” step 9, finish all structural concrete surfaces with one or more of the finishes described here, unless otherwise shown on the Plans.

1. Type I—Ordinary Formed Surface Finish
   Complete formed concrete surfaces with this finish. However, leave concrete exposed directly to sea water undisturbed unless the Engineer requires additional work. See Subsection 500.3.05.V, “Place Seal Concrete,” step 6.
   Achieve a Type I finish as follows:
   a. Immediately after removing the forms, remove fins and surface irregularities.
   b. Fill or point up the following:
      • Cavities produced by forms or ties
      • Holes
      • Broken corners or edges
      • Defects
      • Honeycombed edges
   c. Remove and patch honeycombed areas to sound concrete.
   d. Use patch mortar that consists of the same sand and cement as the concrete. Use the sand and cement in the same ratio as in the concrete.
      Use epoxy mortars in areas where heat generation and moisture will not decrease patch performance.
   e. Cure the patches using one of the general curing methods specified in Subsection 500.3.05.Z.1, “General Curing—Supplying Additional Moisture” and 500.3.05.Z.2, “General Curing—Preventing Moisture Loss.”
   f. Produce a sound and uniform finish.
   g. If the Type I finish is not satisfactory, give the surfaces a Type III—Rubbed Finish where the Engineer considers it necessary to achieve a uniform and pleasing appearance.

2. Type II—Special Formed Surface Finish
   Give a Type II finish to the following:
   • Exposed portions of pipe headwalls and culverts
   • Parapets and wingwalls
   • Ends of culvert slabs and walls
   Achieve a Type II finish as follows:
a. Use a form liner unless the forms are made of plywood or steel.

b. Rub only when necessary if the surface has a pleasing, uniform appearance after completing the Type I finish and blending all pointed and patched areas.

c. If the surface finish is not satisfactory, give surfaces the Type III—Rubbed Finish where the Engineer considers it necessary to achieve a uniform and pleasing appearance.

3. Type III—Rubbed Finish

Apply a Type III finish to bridge areas checked in the table of Bridge areas Requiring a Type III Finish, below and to exposed areas of retaining walls, unless the Plans specify otherwise.

Achieve a rubbed finish as follows:

a. Begin the first rub immediately after removing forms, completing the Type I finish, and ensuring that all patches are thoroughly set, but before applying the required curing compound.

   If finishing is postponed or there is not enough labor to keep it up-to-date, the Engineer will order a stop to any other work until the finishing is satisfactory.

b. Rub chamfered surfaces only once, but not during the first rubbing. Rub chamfered surfaces during either the second or the final rubbing.

c. To rub, wet the moist concrete on the curing surface with a brush and rub with a medium-coarse carborundum stone or equal abrasive until a paste comes to the surface.

   Keep the entire concrete surface moist during rubbing to assure adequate curing.

d. Continue rubbing until all form marks and projections disappear, leaving a smooth, dense surface with no pits or irregularities.

e. Spread the paste material carefully and uniformly over the entire surface and leave it.

f. No earlier than 24 hours after the first rub, do the final rub with a fine carborundum stone or equal abrasive, leaving a smoothly textured surface that is uniform in color.

g. Finish the final rub before applying protective surface treatment required by the Plans.

h. Do not “whitewash” finished areas by using separately mixed grout or paste on the rubbing stone or by spreading it on the surface to be rubbed.

i. Thoroughly clean and blend into the surrounding surfaces any areas that are disfigured by drips from concrete placement or rubbing.
4. **Type III—Special Surface Coating Finish**

A Type III—Special Surface Coating Finish may be substituted for a Type III—Rubbed Finish.

The special surface coating finish consists of either a Class A or a Class B coating system, applied to produce a masonry-like textured finish on concrete surfaces.

For contiguous structures, whether in the same Contract or in separate Contracts, use the same brand of special surface coating.

If contiguous structures are in separate contracts, coordinate the Work with the other Contractor so that coating is applied as near as possible to the same time.

If contractors cannot coordinate Work, the one who finishes the work last shall use the same brand or shall recoat all contiguous areas to provide a uniform appearance.

Achieve a special surface coating finish as follows:

a. Ensure that surface coating material meets the requirements of Section 836.

   Select coating material from the QPL 17.

b. Do not use form oils that affect the bonding of surface coatings.

c. Do not use wax-based or other curing compounds incompatible with surface coatings.

<table>
<thead>
<tr>
<th>Bridge Areas Requiring a Type III Finish (X)</th>
<th>Single Bridge Over Stream</th>
<th>Multiple Bridges Over Stream</th>
<th>Single Bridge Over Railroad</th>
<th>Multiple Bridges Over Railroad</th>
<th>Single Bridge Over Traffic Artery</th>
<th>Multiple Bridges Over Traffic Artery</th>
<th>Railroad Bridge Over Traffic Artery</th>
<th>Pedestrian Bridge Over Traffic Artery</th>
</tr>
</thead>
<tbody>
<tr>
<td>All exposed substructure areas, except tops and bottoms of caps. (5)</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Outside surface of any exterior concrete beam, Lt. or Rt. (1), (2)</td>
<td></td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Outside surface of any exterior concrete beam, Lt. and Rt. (1), (3)</td>
<td></td>
<td></td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Vertical surfaces of overhangs, curb, or sidewalk.</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>All vertical surfaces outside of exterior beam, Lt. or Rt. (2)</td>
<td></td>
<td></td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>All vertical surfaces outside of exterior beam, Lt. or Rt. (3)</td>
<td></td>
<td></td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>End bent cap beyond outside beam or girder.</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>End bent end walls beyond outside beam or girder.</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
</tr>
<tr>
<td>End posts and end bent wingwalls all exposed surfaces.</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>Traffic face of curbs.</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>Entire handrails and posts, hand rail parapet, and barriers. (4), (5)</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>All other locations specified on Special Provisions.</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td>X</td>
<td></td>
<td>X</td>
</tr>
</tbody>
</table>
Have the coating manufacturer or the laboratory determine compatibility.

d. Use the coating color required in Section 836.
e. On surfaces that will receive a coating finish, do not cure with membrane-curing compound or remove forms with bond-breaking agents or excessive oil.
f. Apply coatings as follows:
   • Class A coatings at a rate that develops a 1 1/16 in (.5 mm) thick coating.
   • Apply Class B coatings at a maximum rate of 60 ft² per gallon (1.5 m² per liter).
   • Ensure that the temperatures of the air, concrete, and compound are above 50 °F (10 °C).
   • Apply a test section as directed by the Engineer to determine the acceptance of a coating under field conditions.
   • Apply the coatings using a method that produces an acceptable finish, such as spraying, rolling, or a combination of these.

  g. Protect coated surfaces from rain or freezing temperatures for 24 hours after application.
h. Ensure that the final coating produces a smoothly textured surface that is uniform in color, thickness, and appearance.
i. Remove and reapply coatings that chip, crack, blister, peel, or present an unsatisfactory appearance.
j. If the final appearance is unsatisfactory, apply a rubbed finish to slip-formed and formed walls and barriers.

5. Type IV—Floated Surface Finish
   Use a Type IV finish only on the horizontal surfaces of the following:
   • Curbs and sidewalks
   • Tops of caps and footings
   • Surface of slope paving
   • Other similar structures

   Apply the Type IV finish as follows:
   a. After compacting the surface and screeding to the correct cross sections, float the surface with a wood float.
   b. While floating the surface, bring enough mortar to the surface to achieve the desired finish, but do not reduce the wearing quality of the surface.
   c. Make the final finish with a wood float or stiff-bristle broom.
   d. If brooming, make the marks transverse to the traffic.

6. Type V—Sidewalk Finish
   Apply a Type V finish as follows:
   a. After placing and compacting the concrete, strike it off and give it a Type IV finish.
   b. Use an edging tool on all edges and along expansion joints unless the Plans require chamfers.
   c. Mark off sidewalk surfaces in blocks with suitable grooving tools when required by the Plans or the Engineer.
   d. Extend the rubbed finish on the traffic face of the curb to include the horizontal area of sidewalk between the curb corner and the longitudinal sidewalk groove.

7. Type VI—Stair Tread Finish
   Achieve a Type IV finish using a stiff-bristled broom.

AC. Remove Forms

Do not remove forms and their supports, including falsework, until the Engineer approves. Use a removal method approved by the Engineer. Approval does not relieve responsibility for the safety of the Work.

1. Form Removal Time
   Use a removal time shown on the Plans or specified by the Engineer.
   Use Table 7 to help establish when forms can be removed safely. However, do not count days where the temperature at any time during the day is at or below 40 °F (4 °C), unless the cold weather concrete protective measures described in Subsection 500.1.03.G, “Cold Weather Concrete Curing and Protection Plan” were used.
Table 7—Estimate of Form Removal Time

<table>
<thead>
<tr>
<th>Form</th>
<th>Time Required</th>
</tr>
</thead>
<tbody>
<tr>
<td>Bottom of beams</td>
<td>10 days</td>
</tr>
<tr>
<td>Bottom of caps, trestle pile bents</td>
<td>4 days</td>
</tr>
<tr>
<td>Bottom of all other caps</td>
<td>7 days</td>
</tr>
<tr>
<td>Overhangs and slabs, including culverts</td>
<td>7 days</td>
</tr>
<tr>
<td>Columns and retaining walls</td>
<td>18 to 48 hours</td>
</tr>
<tr>
<td>Sides of beams, posts, rails, caps, footings, wingwalls, and parapets</td>
<td>12 to 24 hours</td>
</tr>
<tr>
<td>Bottoms of cast-in-place rails and diaphragms</td>
<td>48 hours</td>
</tr>
<tr>
<td>Front face of curbs</td>
<td>3 hours</td>
</tr>
</tbody>
</table>

If using high-early strength concrete, the Engineer may reduce the time limitations if the concrete develops satisfactory strengths.

2. Form Removal Method
Remove forms and falsework without injuring the concrete surface or overstressing the concrete members.
Ensure that the stress from the weight of the removal process is transferred gradually and uniformly to the concrete.
At the Contractor’s request, time of removal may be controlled by field tests on cylinders, subject to the following conditions:
   a. No tests will be performed until concrete is at least 3 days old.
   b. Required strengths will be shown on the Plans, as noted elsewhere in these Specifications, or as determined by the Engineer.
   c. The Engineer may specify a minimum time in conjunction with minimum strength requirements.
   d. Falsework and forms for culverts may be removed at such time as 75% of the concrete design strength is achieved.

AD. Apply Protective Surface Treatment
When the Plans specify a protective surface treatment, apply a boiled linseed oil mixture of 75 percent boiled linseed oil and 25 percent mineral spirits by volume to the concrete surfaces.
Use linseed oil that meets the requirements of ASTM D 260, Type I or Type II. Use a quality commercial mineral spirit that passes infrared spectroscopic analysis to the satisfaction of the laboratory.
Unless otherwise noted on the Plans or the manufacturer’s recommendations, apply the mixture as a preservative seal coat to the top surfaces of bridge decks, curbs, and sidewalks and to the inside vertical faces of curbs, parapets, and end posts. Protect metal handrailing and metal handrail posts from treatment.
Apply the protective surface treatment as follows:

CAUTION: Because the linseed oil-petroleum spirits mixture has a low flash point and is readily flammable, protect the mixture from fire, especially cigarettes and sparks. Prohibit traffic from the treated area until the Engineer determines the concrete has regained its dry appearance.

1. Do not place the protective surface treatment until concrete work, including final rubbing, is completed and expansion joint sealing compound is placed.
2. Do not apply the treatment until the concrete is at least 14 days old.
3. Unless otherwise permitted by the Engineer, apply the treatment when the temperature of the concrete and air is at least 50 °F (10 °C).
4. Apply in time to allow the treatment to dry thoroughly before allowing traffic, including haul traffic, on the structure.
If the structure meets the following exceptions, apply the treatment after using the structure for hauling.

- Temperature limitations prohibit application.
  The Engineer will send a written notification to the Contractor (or Bridge Contractor) if temperature requirements prohibit application.
- The structure is absolutely required for hauling to complete a Contract.
  Request a written approval from the Engineer if hauling across a structure before the treatment is placed.

5. If applying the treatment after using the structure for hauling, thoroughly clean the surfaces to be treated to allow the treatment to penetrate completely.

6. If there are separate bridge and roadway Contracts, have the roadway Contractor clean the surfaces immediately upon request by the Engineer.

7. Prepare the surface for the treatment as follows:
   a. Clean off oil, grime, and loose particles that prevent the mixture from penetrating.
   b. Ensure that the concrete surfaces have at least 48 hours to dry after rainfall or wet cleaning operations.
   c. Immediately before applying the treatment, direct an air blast over the surfaces to remove dust.
   d. Mask the exposed plates of joints.

8. Apply the mixture by hand or by spraying in one application at the rate of 1 gal (1 L) of mixture per 37.5 yd² (8.5 m²).
   a. Thoroughly clean the inside of spraying equipment before putting the surface treatment in.
   b. Keep spray nozzles within 18 in (600 mm) of the concrete unless otherwise directed by the Engineer, Plans, or manufacturer.

AE. Apply Graffiti-Proof Coating

When the Plans specify a graffiti-proof coating, apply the coating system to concrete surfaces or over special surface coatings. Use material that complies with Section 838.

Apply the coating as follows:
1. Clean loose particles, dirt, grease, oil, and other foreign particles off the surface.
2. Apply the coating according to the manufacturer’s recommendations for:
   - Weather conditions
   - Material preparation
   - Coating application
   - Number of coats

AF. Expose New Concrete to Loads

Prohibit dead or live loads during or after construction except as described in this section. If using high early strength concrete, the Engineer may reduce time limitations if the concrete develops adequate strength.

1. Dead Loads on the Substructure
   After pouring footings, do not begin work on columns or piers for at least 12 hours.
   After pouring columns, do not begin cap construction for at least 24 hours.
   Do not place beams on caps or place falsework and forming for concrete T-Beam construction before the cap concrete reaches a minimum strength of 2,500 psi (17 MPa).

2. Dead Loads on the Superstructure
   If necessary, stockpile construction materials on decks within a complete unit (a simple span or continuous or cantilever unit) if the following conditions exist:
   - The deck concrete of the complete unit reaches its 28-day cylinder strength.
   - The deck concrete is at least 10 days old.
   - The curbs are at least 5 days old.
   The Engineer must approve the location, height, and spread of the loads.
On composite-design bridges (those that have prestressed concrete beams or steel beams with shear connectors), do not pour curbs, parapets, or sidewalks until the deck concrete reaches a minimum strength of 1,500 psi (10 MPa) or is at least 3 days old.

3. Dead Loads on Concrete Box Culverts
Do not backfill any section of a concrete box culvert until the last concrete placed in that section is at least 14 days old, unless early cylinder breaks indicate otherwise.
If early cylinder breaks indicate that design strength has been achieved, backfill sections of culverts when the concrete placed last is at least 7 days old.

4. Live Loads—Pouring Equipment
Do not allow power-operated concrete buggies to cross a deck until the concrete reaches a minimum strength of 1,500 psi (10 MPa) or is at least 3 days old.
Allow hand-operated buggies to cross after the concrete is 24 hours old.

5. Live Loads—Mixing and Lifting Equipment
Do not place mixers on a deck in a complete unit (a simple span or continuous or cantilever unit) until the deck concrete of the complete unit reaches its 28-day cylinder strength and is at least 10 days old.
When deck concrete reaches its 28-day cylinder strength and is at least 10 days old, allow mixer trucks on the unit during the curb concrete pour only if the pour is completed within 45 minutes of being started.
Do not allow any equipment on the unit for 5 days after curb pours.
The Engineer may allow concrete placement procedures that use heavy lifting equipment on the decks if the following conditions exist:
• The deck concrete reaches its 28-day cylinder strength.
• The deck concrete is at least 14 days old.
• The curbs on the deck are at least 10 days old.

6. Live Loads—Hauling over Bridges
Use a new bridge for hauling only if no other practical haul routes are available and only if the Engineer permits it.

a. Govern hauling by the restrictions and requirements listed in Table 8. If any of the restrictions and requirements are violated, the Engineer will limit loads to the following:
• Single 32,000 lb (14 515 kg) axle when the bridge design loading is HS 20-44 and/or Military Loading
• Single 24,000 lb (10 886 kg) axle when the bridge design loading is HS 15-44 or H 15-44

<table>
<thead>
<tr>
<th>Axle Criteria</th>
<th>Bridge Design Loading</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>HS 20-44 and/or Military Loading</td>
</tr>
<tr>
<td>Maximum Axle Load Per Axle</td>
<td>60,000 lbs (27 216 kg)</td>
</tr>
<tr>
<td>Maximum Axle Load on Dual Axles Per Axle</td>
<td>45,000 lbs (20 412 kg)</td>
</tr>
<tr>
<td>Maximum Total Load</td>
<td>100,000 lbs (45 360 kg)</td>
</tr>
</tbody>
</table>

b. Ensure that bridge concrete, including curbs, parapets, barriers and sidewalks, is at least 14 days old and has a minimum compressive strength of 3,000 psi (20 MPa).
c. Apply the linseed oil special protective treatment, if required see (Subsection 500.3.05.AD, “Apply Protective Surface Treatment”).
d. After applying the protective treatment (if required), apply water-repellent silicone materials to the handrail, handrail posts, end posts, and curb faces before hauling begins.
e. Do not allow more than one vehicle at a time on a simple or multiple-span unit.
f. Ensure that vehicle speeds, loaded or unloaded, do not exceed 5 miles/hr (8 km/hr) when the following loads occur:
• Bridges designed for HS 20-44 and/or Military Loading:
  — Loads on single axles exceed 32,000 lbs (14,515 kg)
  — Loads on each dual axle exceed 24,000 lbs (10,886 kg)

• Bridges designed for HS 15-44 or H 15-44 loading:
  — Loads on single axles exceed 24,000 lbs (10,886 kg)
  — Loads on each dual axle exceed 16,000 lbs (7,257 kg)

When axle loads do not exceed these loads, ensure that vehicle speeds are 15 mph (24 kph) or less.

  g. Place temporary guides on beams so wheels will track directly.
  h. Keep earth approaches smooth and level with the bridge floor or approach slab to minimize impact.

Stabilize sandy and other unstable soils (at no expense to the Department) with crushed stone or other suitable material for at least 10 ft (3 m) from the end of the bridge or approach slab.

  i. Protect the ends of bridges or approach slabs with a timber strip at least 4 in (100 mm) wide, cut to rest on either the paving rest of the bridge end or the pavement subgrade at the end of the approach slab. Keep the strip in place for protection during incidental hauling. Remove it before constructing the adjacent pavement.

Keep the top of each timber strip flush with the top of the concrete surface. Fit the strip tightly against the end of the bridge or approach slab. If the timber strip is displaced, stop hauling until the strip is reset or replaced.

  j. Clean spills off the bridge floor.

**AG. Complete Corrective Work**

After the Department gives the deck surface a Ride Quality Test described in Subsection 500.3.06.E, “Ride Quality Test,” complete corrective work at no cost to the Department and before doing the final surface texturing.

Complete corrective work as follows:

1. Plane the deck according to Section 431.
2. Limit concrete removal by planing so that the final bar cover is not less than the Plan cover minus 1/2 in (13 mm).
3. If the final bar cover limits cannot be met, perform the corrective work as directed by the Engineer.
4. Ensure that the final riding surface complies with this Specification and the requirements for a grooved finish.
5. If necessary, use a bump grinder to correct bumps with a profile base line of 5 ft (1.5 m) or less.
6. Have planed decks retested as described in Subsection 500.3.06.E, “Ride Quality Test,” to ensure that the ride quality meets the requirements of this Specification.

**AH. Plane the Deck**

Schedule the ride quality test at least 5 days before needed by contacting the Office of Materials and Research. Ensure that the area to be tested is clean and clear of obstructions.

When possible, delay expansion joint installation and temporarily bridge the joint to operate Lightweight Profiler and planning equipment across the joint.

Planning responsibilities are shown in Table 9:

**Table 9—Planning Responsibilities**

<table>
<thead>
<tr>
<th>Area Planed</th>
<th>Person Responsible</th>
</tr>
</thead>
<tbody>
<tr>
<td>Bridge decks</td>
<td>Bridge Contractor</td>
</tr>
<tr>
<td>Approach slabs constructed under the bridge</td>
<td>Bridge Contractor</td>
</tr>
<tr>
<td>Approach slabs constructed under the roadway</td>
<td>Roadway Contractor</td>
</tr>
</tbody>
</table>

**AI. Perform Retaining Wall Incidentals**

Retaining wall incidentals are as follows:

1. Drainage

   Unless otherwise shown on the Plans or in the Special Provisions, ensure that drainage for retaining walls is either Alternate A or Alternate B on Georgia Standards 4948 and 9031-L.
Ensure that the Number 10 concrete sand complies with Subsection 801.2.02, “Fine Aggregate for Portland cement Concrete of All Types and for Mortar” and has a permeability coefficient of at least 100 ft (30 m) per day.
The Engineer may waive the grading requirement for Number 10 concrete sand if the permeability coefficient of the material does not exceed 500 ft (150 m) per day.
Omit the drainage blanket and stone for retaining walls only when the height does not exceed 6 ft (1.8 m).
When the Plans specify different drainage details, furnish, place, or build the various items according to the Plan requirements.

2.防水防潮

当防水和防潮在计划中被指定时，应遵守第530和531节的要求。

A. 将安装硬件

当计划要求安装硬件时，涉及的公用事业公司将提供这些物品。

B. 宽基和路面

当使用波特兰水泥混凝土的窄段来拓宽现有基座或基座和路面时，使用B类混凝土，如计划所示或由工程师指示。

C. 开放结构到交通

开放一个结构到运输，而不是运输交通，当所有的混凝土在桥面、栏杆或路边（人行道）达到其28天的试块强度并且至少14天大。

500.3.06质量接受

A. 强度要求试验

当现场试验标本未达到强度要求时，根据第1节—混凝土混合表确定混凝土在位的最终接受或拒绝。

在承包商的要求下，部门将确定形体由现场试验混凝土圆柱体。

测试受下列条件影响:
1. 测试将在混凝土至少3天后进行。
2. 图纸将显示所需的强度。
3. 在承包商的要求下，工程师可能指定至少所需时间的最小强度。

B. 蜂窝区域检查

如果有蜂窝区域，其尺寸超过钢筋，工程师可以拒绝整个倒置。

C. 桥梁甲板混凝土检验

工程师将仔细观察在所有桥面甲板施工阶段使用的所有施工方法。这些阶段包括以下:

- 金属形式安装
- 钢筋位置和固定
- 混凝土项目组成
- 混凝土搅拌程序
- 桥梁甲板表面

提供必要的设施供工程师安全和方便地检查混凝土。

混凝土检查程序如下:
1. After the deck concrete has been in place for at least two days, the Engineer will sound a hammer on at least two areas of the deck for each slab pour. This test checks for concrete soundness and form bonding. The two areas will encompass at least 10 percent of the total area of the deck pour.

2. The Engineer will sound other areas of the deck randomly.

3. If the Engineer doubts the soundness of an area, or if the Engineer decides that the concrete placement procedures used call for an inspection of the underside of the deck, remove at least one section of the forms for each span in the Contract.

4. Remove the form section after the pour is strong enough and when the Engineer desires to provide visual evidence that the concrete mix and the placement procedures are acceptable.

5. Remove another form section if the Engineer decides changes in the concrete mix or in the placement procedures warrant additional inspection.

6. Where form sections are removed, do not necessarily replace the forms, but repair the adjacent metal forms and supports neatly and securely.

7. When the form is removed, the Engineer will examine the concrete surfaces for cavities, honeycombing, and other defects.

8. If the Engineer finds irregularities but determines that the irregularities do not justify rejection of the Work, repair the concrete as the Engineer directs and give it an ordinary surface finish according to the Contract Specifications.

9. If the concrete where the form is removed is not acceptable, remove additional forms as necessary to inspect and repair the slab.

10. Modify the construction methods as required by the Engineer to create satisfactory slab concrete.

11. Remove or repair all unsatisfactory concrete as the Engineer directs.

If the construction methods used and the inspection results indicate that the slabs have sound concrete, the Engineer may moderate the amount of random sounding and form removal after a substantial amount of slab has been constructed and inspected.

D. Bridge Deck Surface Check

After the final strike-off of the concrete and as close behind the final strike-off as possible, the Engineer will check the surface with a 10 ft (3 m) straightedge.

Attach the straightedge to a broom-type handle for easy control and use.

Bridges and approach slabs must meet a 1/8 inch in 10 ft (3 mm in 3 m) straightedge check made longitudinally and transversely.

E. Ride Quality Test

After the bridge decks and approach slabs are completed, the Department will perform a Ride Quality Test using the Lightweight Profiler and a profile index value determined according to GDT 134.

The Department will conduct the test as follows:

1. Obtain Profile Index Values for bridge decks and approach slabs for:
   - State roads with four lanes or more
   - State roads with 2 lanes where the current traffic count is 2,000 vehicles per day or higher
   - Other roads designated on the Plans

   Bridges and approach slabs must meet the straightedge check limits described in Subsection 500.3.06.D, “Bridge Deck Surface Check.”

2. Obtain profiles in the wheel paths and in safety areas to within 6 ft (1.8 m) of barrier or curb lines.

3. Average the profile index values for bridge decks including the approach slabs for the left and right wheel path for each lane.

   The average value must not exceed 15 in/mile (235 mm/km) for each lane.

   After the test is complete, correct individual bumps or depressions that exceed 2/10 in (5 mm) from the blanking band on the profiler trace.

   The deck surface must then meet a 1/8 inch in 10 ft (3 mm in 3 m) straightedge check made transversely.
Correct bridge decks and approach slabs that do not pass the Ride Quality Test as described in Subsection 500.3.05.AG, “Complete Corrective Work.”

500.3.07 Contractor Warranty and Maintenance
General Provisions 101 through 150.

500.4 Measurement
No measurement to be included for this item.

500.5 Payment
Payment will be made under CONSTRUCTION COMPLETE. Payment is full compensation for all things, including incidentals, and direct and indirect costs, to complete the Work.

500.5.01 Adjustments
A. Contractor Costs
Assume the following costs:
1. Costs related to rejected concrete and removing rejected concrete
2. Costs of forming an approved construction joint, removing a partial pour, or completing other remedial measures requested by the Engineer unless the fault lies solely with the Department
3. Costs of repairing, removing, and replacing falsework as directed by the Engineer
4. Costs of repairing, removing, or replacing forms
5. Costs of air-blown mortar to repair honeycombed areas, if required by the Engineer
6. Costs of using a higher class of concrete to widen existing bases or bases and pavements

B. Ride Quality Testing
The Department will conduct ride quality testing of bridge decks and approach slabs only twice per bridge at no cost to the Contractor.

The Department will conduct additional ride quality testing at the cost of $500 per test.

C. Plastic Shrinkage Crack Repair
The Engineer will determine how to repair cracks caused by plastic shrinking. Repair cracks at no cost to the Department.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

VOID FOR BIDDING

DO NOT UNSTAPLE THIS BOOKLET....ENTER ALL REQUIRED INFORMATION
------------------------------------ EITHER BY HAND OR BY STAMP.

DATE OF OPENING : March 19, 2010 CALL ORDER : 001

CONTRACT ID : B13668-10-000-0

PCN
PROJECTS AND CONTRACT NO.
------------
0006043.01000 CSNHS-0006-00(043)

COUNTY : CHEROKEE

CODE__________ ISSUED TO:

1. _______________________________________________________________________
2. _______________________________________________________________________
3. _______________________________________________________________________

PART 2 OF 2
THE CONTRACTOR SHALL RETURN ALL PARTS OF THE PROPOSAL WITH HIS BID UNLESS BIDDING ELECTRONICALLY
Section 509—Prestressing Concrete by Post Tensioning

Delete Subsection 509.1 and substitute the following:

509.1 General Description
This work consists of prestressing concrete by post-tensioning cast-in-place concrete. The work includes furnishing, placing, and tensioning prestressing steel according to the Plan details and these Specifications.

509.1.01 Definitions
Working Force and Working Stress: The force and stress remaining in the prestressing steel after the following losses:

- Creep and shrinkage of concrete
- Elastic compression of concrete
- Creep of steel
- Loss in post-tensioned prestressing steel from the sequence of stressing
- Friction and anchor set (see Subsection 509.3.05.J, “Post-Tension the Tendons,” steps 18 to 19)
- Other losses peculiar to the method, technique, or system of prestressing (see Subsection 509.3.05.J, “Post-Tension the Tendons,” step 21)

509.1.02 Related References
A. Standard Specifications
   - Section 501—Steel Structures
   - Section 535—Painting Structures
B. Referenced Documents
   - AASHTO Specifications for Highway Bridge, Article 9.16.1
   - AASHTO Specifications for Highway Bridge, Article 9.16.2
   - ASTM C 109
   - ASTM A 416
   - ASTM A 722
509.1.03 Submittals

A. Coupler Use and Location

The use and location of couplers in bars entering into the prestressing work is subject to the Engineer’s approval.

B. Alternate Stressing or Anchorage Block Drawings and Calculations

When using stressing or anchorage blocks not shown on the Plans, submit shop drawings and calculations for the blocks to Bridge and Structural Design when submitting the prestressing system calculations and shop drawings.

C. Design Calculations

Submit design calculations for the proposed post-tensioning system to Bridge and Structural Design for Department review and approval. Design calculations may be on letter size sheets.

Submit calculations for the size and spacing of the reinforcing around the ducts, as shown in Figure 1 (metric), to Bridge and Structural Design. Include the following in the calculations:

- Required jacking force and elongation of tendons during tensioning
  Using the initial jacking force, design the reinforcing to prevent ducts from pulling out because of the effects of web curvature and slope.
- Stresses in anchorages and distribution plates
  Ensure that the calculations account for reinforcing to prevent the peeling of anchorages from the top and bottom slab. See Figure 2 for minimum reinforcing requirements for tying ducts to the deck reinforcing.
- Stress-strain curves typical of the prestressing steel to be furnished
- Seating losses
- Temporary overstresses
- Reinforcing in the concrete to resist tensioning loads

Determine bearing offsets and expansion joint gaps and adjust for construction sequence, prestress shortening, and temperature.
* A stirrup group is one pair of overlapping “U” shaped bars.
  • Stirrups shall enclose vertical web reinforcement.
  • No more than 3 ducts shall be enclosed by a stirrup group.
  • Min. bar size: No. 4 (No. 13 M) bar.
  • Max. longitudinal bar spacing: 24 in. (600 mm)
D. Certificates of Compliance

The Department will accept certificates of compliance for cements to be used. The Department reserves the right, however, to sample and test the cement before its use and at any time during the progress of the work.

E. Certified Mill Test Reports

Submit certified mill test reports for high tensile prestressing steel to the Project Engineer.

F. Shop Drawings

Submit Shop Drawings for review and approval according to Subsection 501.1.03.B, “Shop Drawings.” Place a title block in the lower right-hand corner of the drawings that includes the following:

- Project number
- Sheet numbering for the Shop Drawings
- Structure name
- Contractor and fabricator names

Submit Shop Drawings on 23 in by 36 in (575 mm by 900 mm) sheets with a 1-1/2 in (38 mm) left margin and a 1/2 in (13 mm) top, bottom, and right margins.

The Shop Drawings shall include the following:

1. Fully dimensional views showing all projections, recesses, notches, openings, blockouts, and pertinent design details
2. Details of mild steel reinforcing showing size, spacing, and location, including special reinforcing required as determined by the design calculations but not shown on the Plans
3. Details of ducts for post-tensioning tendons showing size, type, and horizontal and vertical profiles
4. Details of duct supports, grout tubes, and vents showing size, type, and location
5. Details of the relative positions of reinforcing steel, ducts, and anchorages
6. Details of the anchorage systems for the proposed post-tensioning system
7. A table giving jacking sequence, jacking forces, and initial elongation of the tendons at each erection stage for post-tensioning
8. Details and a complete description of the post-tensioning system to be used for permanent tendons
9. Details of the prestressing, including:
   • Method, sequence, and procedure for prestressing and securing tendons
   • Procedure for releasing tendons
   • Equipment supplier and type
   • Tendon size and properties
   • Anchorage plates and assemblies
10. Working drawings and bar schedules for each prestressing system
11. Details of reinforcing or coil ties under anchorage plates
12. Details for usage of high-strength steel bar (furnished by the bar manufacturer)
13. Friction factors used in the prestressing system of deformed bars

As an option, shop drawings may be submitted on plan sheet sizes of 12” x 18” (305 mm x 457 mm) or 11” x 17” (279 mm x 432 mm) for review and approval. Information contained on these sheets must be legible.

After shop drawings have been approved, submit an electronic file that is compatible with Bentley Microstation J (Version 7) Cadd operating system, or an electronic file in Adobe Acrobat Portable Document Format (.pdf) to the Engineer. For bridges carrying railroads only, after shop drawings have been approved, submit one full size set of reproducible drawings to the Department.

G. Ram Calibration Charts

Before using rams in the work, furnish the Engineer with a certified chart from the calibration for each ram.

H. Designs and Details of Distribution Reinforcing Steel

The Department plans for anchorages show only a minimum amount of distribution reinforcing steel.

Design and detail the reinforcement needed to prevent bursting, peeling, and splitting. Submit the designs and details to the Engineer for review and approval.

I. Gauge Readings and Elongations

Keep a record of gauge pressures or readings and elongations at the end of each jacking operation and submit it to the Engineer for review and approval.

J. Grouting Operations Plan

Submit to the Engineer a grouting operations plan at least 6 weeks in advance of any scheduled grouting operations. The Engineer will forward the grouting operations plan to the Office of Materials and Research for approval. Written approval of the grouting operations plan by the Office of Materials and Research is required before any grouting of the permanent structure takes place.

At a minimum, the plan will address and provide procedures for the following:

1. Provide names and proof or experience/training for the grouting crew and the crew supervisor.
2. Type, quantity, and brand of materials used in grouting including all certifications required.
3. Type of equipment furnished, including capacity in relation to demand and working condition, as well as back-up equipment.
4. General grouting procedures.
5. Duct cleaning methods prior to grouting.
6. Mixing and pumping procedures.
7. Direction of grouting.
8. Sequence of use of the inlets and outlets pipes.

*Delete Subsection 509.2 and substitute the following:*

**509.2 Materials**

Ensure that materials meet the requirements of the following Specifications:
### A. Steel

Do not use strands from more than one source within the same tensioning operation.

Strands that differ in size from ASTM A 416 are to be submitted for prior approval.

High strength steel bars shall meet ASTM A 722 Type II, and SI through S# supplemental requirements and have manufacturers details for their use.

Ensure all bars within any member are of same grade.

Bar couplers and locations are to be approved prior to use and shall have tensile strength not less than manufacturers minimum for strength of bar.

Allow the Department 60 calendar days before installing prestressing steel to test the steel and approve the materials furnished.

Use the anchor devices and distribution plates recommended by the manufacturer of the prestressing system.

### B. Post-Tensioning Grouts

Use only post-tensioning grouts meeting the requirements of this subsection. Submit to the Engineer a written certification from the manufacturer that the product meets the requirements of this subsection. The Engineer may request that the manufacturer also submit certified test reports from an independent laboratory audited by the Cement Concrete Reference Laboratory (CCRL) which shows the material meets all the requirements specified herein.

1. The grout shall not contain aluminum or other components which produce hydrogen, carbon dioxide or oxygen gas.

2. The grout shall meet or exceed the specified physical properties stated as determined by the following standard and modified test methods. Conduct all grout tests with grout mixed to produce the minimum time of efflux. Establish the water content to produce the minimum and maximum time of efflux.

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Value</th>
<th>Test Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Total Chloride Ions</td>
<td>Max. 0.08% by weight of cementitious material</td>
<td>ASTM C 1152</td>
</tr>
<tr>
<td>Volume Change</td>
<td>0.0% to +0.1% at 24 hours &lt;+0.2% at 28 days</td>
<td>ASTM C 1090</td>
</tr>
<tr>
<td>Expansion</td>
<td>≤2.0% for up to 3 hours</td>
<td>ASTM C 940</td>
</tr>
<tr>
<td>Compressive Strength at 28 days</td>
<td>≥5000 psi (35 MPa)</td>
<td>ASTM C 942</td>
</tr>
<tr>
<td>Wet Density – Laboratory</td>
<td>Report maximum and minimum obtained test value lbs/ft³</td>
<td>ASTM C 185</td>
</tr>
<tr>
<td>Property</td>
<td>Test Value</td>
<td>Test Method</td>
</tr>
<tr>
<td>--------------------------------</td>
<td>------------------------------------------------</td>
<td>---------------</td>
</tr>
<tr>
<td>Initial Set</td>
<td>Min. 3 hours Max. 12 hours</td>
<td>ASTM C 953</td>
</tr>
<tr>
<td>Time of Efflux$^{(1)}$</td>
<td></td>
<td></td>
</tr>
<tr>
<td>(a) Immediately after mixing</td>
<td>Min. 20 seconds Max. 30 seconds</td>
<td>ASTM C 939</td>
</tr>
<tr>
<td></td>
<td>or Min. 9 seconds Max. 20 seconds</td>
<td>ASTM C 939$^{(2)}$</td>
</tr>
<tr>
<td>(b) 30 minutes after mixing</td>
<td>Max. 30 seconds</td>
<td>ASTM C 939</td>
</tr>
<tr>
<td>with remixing for 30 seconds</td>
<td>or Max. 30 seconds</td>
<td>ASTM C 939$^{(2)}$</td>
</tr>
<tr>
<td>Bleeding @ 3 hours</td>
<td>Max. 0.0%</td>
<td>ASTM C 940$^{(3)}$</td>
</tr>
<tr>
<td>Permeability at 28 days</td>
<td>Max. 2,500 coulombs at 30 V for 6 hours</td>
<td>ASTM C 1202</td>
</tr>
</tbody>
</table>

(1) Adjustment to flow rates will be achieved by strict compliance with the manufacturer’s recommendations. The time of efflux is the time to fill a one liter container placed directly under the flow cone.

(2) Modify ASTM C 939 test by filling the cone to the top instead of to the standard level.

(3) ASTM C 940 modified per PTI Specification subsection 4.4.6.1, Wick Induced Bleed Test.

Have the Engineer approve grout for filling recesses or encasing anchoring devices. Use a type recommended by the manufacturer for highly stressed steel.

Add the following to Subsection 509.2.01:

D. Grout

Grouts shall be prepackaged in moisture proof containers. Store grout in a location that is both dry and convenient to the work. Storage in the open must be on a raised platform and with adequate waterproof covering. Grout bags shall indicate the following:

1. Type of application
2. Date of manufacture
3. Lot number
4. Mixing instruction

Provide to the Engineer the manufacturer’s Quality Control Data Sheet for each lot number and shipment sent to the job site. Materials with a total time from manufacture to usage in excess of six (6) months shall be retested and certified by the supplier before use or removed and replaced.

Add the following to Subsection 509.3.01:

B. Grouting Supervisor

Ensure the supervisor has verifiable documentation of three years of experience in construction of grouted post tensioned structures and has successfully completed training in a grouting technician certification program, such as, the American Segmental Bridge Institute’s grouting certification program or an approved equal training program.
Delete Subsection 509.3.02.C and substitute the following:

C. Grouting Equipment

Provide grouting equipment consisting of measuring devices for water, a high speed shear colloidal mixer, a storage hopper and a pump with all the necessary connecting hoses, valves, and pressure gauge.

Provide pumping equipment with sufficient capacity to ensure continuous grouting of the largest tendon on the Project in 20 minutes.

1. Mixer and Storage Hopper

Provide a high speed shear colloidal mixer capable of continuous mixing producing a homogeneous and stable grout free of lumps and undispersed cement. The grout machinery will have a charging tank for blending and a holding tank.

- The blending tank must be equipped with a high shear colloidal mixer.
- The holding tank must be kept agitated and at least partially full at all times during the pumping operation to prevent air from being drawn into the post-tensioning duct.

Add water during the initial mixing by use of a flow meter or calibrated water reservoir with measuring accuracy of ± 1.0 ounces (30 ml) or better.

2. Grout Pumping Equipment

Provide pumping equipment capable of continuous operation which will include a system for circulating the grout when actual grouting is not in progress.

- The equipment will be capable of maintaining pressure on completely grouted ducts and will be fitted with a valve that can be closed off without loss of pressure in the duct.
- Grout pumps will be positive displacement type, will provide a continuous flow of grout and will be able to maintain a discharge pressure of at least 145 psi (1 MPa).
- Pump seals adequate to prevent oil, air, or other foreign substances out of the grout and to prevent loss of grout or water.
- Pressure gauge with a maximum full scale reading of 300 psi (2 MPa) installed at some point in the grout line between the pump outlet and the duct inlet to establish grout pressure at the pump.
- Screen with 0.125 in (3 mm) maximum clear openings to screen the grout before it is introduced into the grout pump.

3. Vacuum Grouting Equipment

Provide vacuum grouting equipment at the jobsite concurrently with all pressure grouting operations.

- The equipment will be the volumetric measuring type with the ability to measure a void and supply a measured volume of grout to fill the void.

4. Standby Equipment

Provide flushing equipment capable of pumping 300 psi (2 MPa) gauge and flushing out partially-grouted ducts.

- A different power source for the flushing equipment than the grouting equipment.
Delete Subsections 509.3.05.K, L and M, and substitute the following:

K. Mix the Grout

Maximum grout temperature will not exceed 90 °F (32.2 °C). Use chilled water and/or pre-cooling of bagged material to maintain mixed grout temperature below the maximum allowed temperature.

Grouting operations are prohibited when the ambient temperature is below 40 °F (4 °C) or is 40 °F (4 °C) and falling. Remove any standing water from ducts using compressed air, if freezing temperatures are forecast.

Grout for use with prestressing concrete bridge members includes a mixture of prepackaged material and water, as follows:

- Prepackaged material—Use prepackaged material that meets the requirements of Subsection 509.2.01.D.
  The Department reserves the right to sample and test the prepackaged material before its use and at any time during the work.
- Water—Use potable water or other water that meets the requirements of Subsection 880.2.01.

Mix the prepackaged material and water in accordance with the manufacturer’s recommendation and as follows:

1. Mix the grout with a metered amount of water.
2. When adding water, do not exceed the manufacturer’s recommendations.
3. The materials will be mixed to produce a homogeneous grout.
4. Continuously agitate the grout until it is pumped.
5. Do not add water to increase grout flowability that has decreased because grout use is delayed.

The Engineer may determine grout pumpability according to ASTM C 939. When using this method, efflux time for the grout sample immediately after mixing will not be less than the efflux time as established in subsection 509.2.B.

L. Prepare Ducts for Grouting

Prepare the ducts for grouting by flushing the metal ducts with compressed air.

1. Clear ducts of water and debris at a pressure no greater than allowed for grouting the tendon.
2. Use oil-free compressed air to blow out ducts.

M. Grout the Duct

Bond prestressing steel to the concrete by filling the space between the duct and the tendon with grout. Grout tendons in accordance with the Grouting Operations Plan.

Grout the duct as follows:

1. Open the grout and vent openings.
2. Unless approved otherwise by the Engineer, pump grout at a rate of between 16 linear feet (5 m) and 50 linear feet (15 m) of duct per minute. Ensure that the pumping pressure at the grout inlet does not exceed 245 psi (1.7 MPa).
3. Allow the grout to flow from the first vent after the inlet pipe to remove any residual water or entrapped air.
4. Once water or air is removed, cap or otherwise close the vent. Close the remaining vents in sequence in the same manner.
5. If the grouting pressure exceeds 245 psi (1.7 MPa) gauge, inject grout at a vent that has been or is ready to be capped.
   a. Maintain a one-way grout flow while injecting.
   b. Fit the vent used for injection with a positive shutoff.
   c. If a one-way flow of grout cannot be maintained, immediately flush the grout out of the duct with water.

6. Pump grout through the duct and waste it continuously at the outlet pipe until the following happens:
   • No visible slugs of water or air are ejected.
   • The measured grout efflux time will not be less than the efflux time measured at the pump or minimum acceptable efflux time as established in subsection 509.2.B.

7. To insure that the duct remains filled with grout:
   a. Close the outlet.
   b. Hold pumping pressure for an additional 1 minute and then close the inlet under pressure.
   c. Do not remove or open plugs, caps, or valves used to close off the outlet or inlet until the grout has set.

Add the following to Subsection 509.3.06.

D. Tendon Grouting

At least 24 hours after completion of the grouting of tendon ducts, and no more than 7 days after grouting, investigate the ducts for voids, as follows:

1. After the grout has cured, open grout injection and exit ports by drilling through the ports into the duct cavity to probe for any void.

2. Sound all grout caps for voids. Unless grout caps are determined to have voids, do not remove or drill the cap.

3. If voids are found, completely fill the void with grout by secondary grouting of the duct with vacuum grouting process that determines the size of the void and measures filling of the void.

4. If no voids are found, clean and backfill the drilled hole with Type V epoxy selected from QPL 15—Epoxy Resin Adhesives. Use an injection tube to extend to the bottom of the drilled hole. During drilling operations use equipment that will automatically cut-off when steel is encountered.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

Supplemental Specification

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 550—Storm Drain Pipe, Pipe-Arch Culverts, and Side Drain Pipe

Delete Section 550 and Substitute the following:

550.1 General Description
This work includes furnishing and installing the following:
- Storm drain pipe
- Pipe-arch and elliptical culverts
- Side drain pipe flared end sections
- Tapered pipe inlets
Install structures according to the Specifications and the details shown on the Plans, or as directed by the Engineer.

550.1.01 Definitions
General Provisions 101 through 150.

550.1.02 Related References
A. Standard Specifications
   - Section 205—Roadway Excavation
   - Section 207—Excavation and Backfill for Minor Structures
   - Section 208—Embankments
   - Section 645—Repair of Galvanized Coatings
   - Section 815—Graded Aggregate
   - Section 834—Masonry Materials
   - Section 840—Corrugated Aluminum Alloy Pipe
   - Section 841—Iron Pipe
   - Section 843—Concrete Pipe
   - Section 844—Steel Pipe
   - Section 845—Smooth Lined Corrugated High Density Polyethylene (HDPE) Culvert Pipe
   - Section 846—Polyvinyl chloride (PVC) Drain Pipe
   - Section 847—Miscellaneous Pipe

Office of Urban Design
Section 848—Pipe Appurtenances

B. Referenced Documents

General Provisions 101 through 150.
GDOT Manual on Drainage Design for Highways
Ga. Std. 1030D
Ga. Std. 1030P

550.1.03 Submittals
General Provisions 101 through 150.

550.2 Materials

Ensure that materials meet the requirements of the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Backfill Materials</td>
<td>207</td>
</tr>
<tr>
<td>Graded Aggregate</td>
<td>815.2.01</td>
</tr>
<tr>
<td>Reinforced Concrete Pipe</td>
<td>843.2.01</td>
</tr>
<tr>
<td>Nonreinforced Concrete Pipe</td>
<td>843.2.02</td>
</tr>
<tr>
<td>Mortar And Grout</td>
<td>834.2.03</td>
</tr>
<tr>
<td>Bituminous Plastic Cement</td>
<td>848.2.05</td>
</tr>
<tr>
<td>Rubber Type Gasket Joints (Concrete Pipe)</td>
<td>848.2.01</td>
</tr>
<tr>
<td>Preformed Plastic Gaskets</td>
<td>848.2.06</td>
</tr>
<tr>
<td>Corrugated Steel Pipe</td>
<td>844.2.01</td>
</tr>
<tr>
<td>Bituminous Coated Corrugated Steel Pipe</td>
<td>844.2.02</td>
</tr>
<tr>
<td>Corrugated Aluminum Alloy Pipe</td>
<td>840.2.01</td>
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<td>Bituminous Coated Corrugated Aluminum Pipe</td>
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<td>Aluminized Type 2 Corrugated Steel Pipe</td>
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<td>Polyvinyl Chloride (PVC) Corrugated Smooth Interior Drain Pipe</td>
<td>846.2.02</td>
</tr>
<tr>
<td>Miscellaneous Pipe</td>
<td>847</td>
</tr>
</tbody>
</table>

Use any of the following types of pipe:

- Reinforced concrete
- Nonreinforced concrete
- Corrugated steel or Aluminum
- Smooth-lined corrugated high density polyethylene (HDPE)
- Ductile iron
- Polyvinyl Chloride (PVC) Profile Wall Drain Pipe
- Polyvinyl Chloride (PVC) Corrugated Smooth Interior Drain Pipe
Use the type of pipe designated on the Plans, or acceptable alternate types when applicable. For a display of acceptable alternate pipe types see Selection Guideline for Culvert, Slope and Underdrain Pipe in Chapter 10 – Material Selection of the Department’s Manual on Drainage Design for Highways. This document summarizes general applications for pipe.

For concrete, corrugated steel and aluminum pipe see Ga. Std. 1030D for minimum thicknesses, minimum cover, maximum fill, allowable pipe diameters and trench construction detail.

For HDPE and PVC pipe see Ga. Std. 1030P for minimum cover, maximum fill, allowable pipe diameters and trench construction detail.

550.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

550.3 Construction Requirements

550.3.01 Personnel
General Provisions 101 through 150.

550.3.02 Equipment
General Provisions 101 through 150.

550.3.03 Preparation and Backfill
Before installing pipe, shape the foundation material as shown on the Plans.

Prepare structure excavations and foundation according to Section 207. Except, for HDPE and PVC pipe use the following requirements for backfill:

1. Cross drain applications use material that meets Subsection 815.2.01.
2. Longitudinal and side drain applications use material according to Section 207. Except, when Type I backfill material is required use Class II B2 soil or better per Subsection 810.2.01.

550.3.04 Fabrication
General Provisions 101 through 150.

550.3.05 Construction
A. Drainage

Provide necessary temporary drainage. Periodically remove any debris or silt that constricts the pipe flow to maintain drainage throughout the life of the Contract.

B. Damage

Before allowing construction traffic over a culvert, protect the structure by providing sufficient depth and width of compacted backfill. Repair damage or displacement from construction traffic or erosion that occurs after installing and backfilling at no additional cost to the Department.

C. Installation

1. Concrete Pipe

   Lay sections in a prepared trench with the socket ends pointing upstream. To join sections, use any of the following joint types:
   
   • Mortar
   • Bituminous plastic cement
   • Rubber gasket
   • Preformed flexible sealant

   If using mortar and bituminous plastic cement joints:

   a. Fill the annular space with the joint material and wipe the inside of each joint smooth.
b. Construct mortar joints in the same manner, but thoroughly wet the annular space before filling it with joint material.
c. Install rubber and preformed flexible sealant joints according to the manufacturer’s recommendations.

2. Ductile Iron Pipe
   Lay pipe sections in a prepared trench, with bells pointing upstream. Construct joints according to Subsection 841.2.02.A.

3. Corrugated Aluminum or Steel Pipe and Pipe-Arches
   Lay pipe sections in a prepared trench, with outside laps of circumferential joints pointing upstream and longitudinal joints at the sides. Join the sections with coupling bands, fastened by two or more bolts. Before backfilling the structure:
   a. Repair exposed base metal in metal coating according to Section 645.
   b. Recoat exposed base metal in bituminous coating with asphalt.

4. Smooth-Lined Corrugated HDPE Pipe
   Install smooth-lined corrugated HDPE pipe according to ASTM D 2321. Use fitting and couplings that comply with the joint performance criteria of AASHTO Standard Specifications for Highway Bridges, Division II. Ensure that all joints are “silt tight” as stated in the AASHTO bridge specifications.

5. Specials (Wyes, Tees, and Bends)
   Install wyes, tees, and bends as shown on the Plans or as directed.

6. Tapered Pipe Inlets
   Locate and install tapered pipe inlet end sections as shown on the Plans or as directed.

7. Elongation
   Elongate metal pipe as shown on the Plans. Order the elongation of the vertical axis of the pipe to be done in the shop.
   Have the manufacturer ship metal pipe with wire ties in the pipe ends. Remove wire-ties immediately after completing the fill.

8. Flared End Sections
   Use flared end sections on the inlet, outlet, or on both ends of storm drain pipe, according to Plan details.

9. PVC Drain Pipe
   Install PVC drain pipe according to ASTM D 2321. Use fittings and couplings that comply with the joint performance criteria of AASHTO Standard Specifications for Highway Bridges, Division II. Ensure that all joints are “silt tight” as stated in the AASHTO bridge specifications.

550.3.06 Quality Acceptance

Clean pipe before final acceptance of the Work.

The Department may conduct video surveillance on storm drain (cross drain and longitudinal drain) installations after all activities are complete that may damage the pipe, but before the placement of the base and paving when applicable. If video surveillance shows problems such as pipe deformation, cracking, or joint separation, the Contractor shall repair or replace these pipes at no cost to the Department.

Use a nine-point mandrel to test 100% of the installed length of smooth-lined corrugated HDPE or PVC drain pipe for deformation. Use a mandrel that has an effective diameter equal to 95% of the base inside diameter. Provide the Engineer with a proving ring to verify the mandrel size. Mandrel testing shall not be paid for separately.

Ensure that smooth-lined corrugated HDPE or PVC drain pipe installations have a maximum of 5% deflection when checked after completing all construction activities that may damage the pipe, but before placing the base and paving when applicable. Remove and replace pipe with over 5% deflection at no cost to the Department.

550.3.07 Contractor Warranty and Maintenance

General Provisions 101 through 150.
550.4 Measurement
No measurement to be included for this item.

A. Excavation and Backfill – not measured separately.
B. Flat Bottom and Circular Pipe (All Types) – not measured separately.
D. Multiple Installations – not measured separately.
E. Tapered Pipe Inlets – not measured separately.
F. Flared-End Sections – not measured separately.
G. Smooth-Flow Pipe – not measured separately.
H. Elliptical Pipe – not measured separately.

550.4.01 Limits
Excavation and normal backfill are not measured for payment.

550.5 Payment
Payment will be made under CONSTRUCTION COMPLETE. Includes backfill, pipe installations, excavating, furnishing, and hauling materials; installing, cutting pipe where necessary; repairing or replacing damaged sections; making necessary connections; strutting, elongating, providing temporary drainage; joining an extension to an existing structure where required; and removing, disposing of, or using excavated material as directed by the Engineer.

1. Smooth Flow Pipe
   Payment will be made under CONSTRUCTION COMPLETE. Includes furnishing labor, materials, tools, O-ring mechanical joints, equipment, and incidentals to complete this Item, including removing and disposing excavation material.

2. Flared-End Sections
   Payment will be made under CONSTRUCTION COMPLETE.
   Payment will also include sawing, removing, and replacing existing pavement removed to install a new drainage structure.
Section 550—Storm Drain Pipe, Pipe-Arch Culverts, and Side Drain Pipe

Delete any and all references to Smooth Lined Corrugated High Density (HDPE) Polyethylene Culvert Pipe, Polyvinyl Chloride (PVC) Profile Wall Drain Pipe or Polyvinyl Chloride (PVC) Corrugated Smooth Interior Drain Pipe.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number:  CSNHS-0006-00 (043)
P.I. Number:  0006043
Cherokee County

Add the following:

Section 620—Temporary Barrier

620.1 General Description
This work provides for Method 1 and Method 2 temporary barrier systems.

620.1.01 Definitions
Method 1- Method of furnishing, placing, maintaining, moving, and reusing where required, and removing temporary barrier of the length and at the locations shown on the Plans. Method 1 barrier is not suitable on bridges where the distance from the centerline of the barrier to the free edge of the bridge deck is less than or equal to 6'-0" (1.8 m) measured normal to the barrier.

Method 2- Method of furnishing, placing, maintaining, moving, and reusing where required, and removing manufactured barrier of the length, and at the locations shown on the Plans. Method 2 barrier is to be used on bridges and bridge approaches where the distance from the centerline of the barrier to the free edge of the bridge deck is less than or equal to 6'-0" (1.8 m) measured normal to the barrier.

620.1.02 Related References
A. Standard Specifications
   Section 500—Concrete Structures
   Section 501—Steel Structures
   Section 511—Reinforcement Steel

B. Referenced Documents
   General Provisions 101 through 150

620.1.03 Submittals
Method 1 - Submit certification from the manufacturer that the proposed barrier and its interconnecting hardware replicates an NCHRP-350 “Test Level 3” approved documented in an acceptance letter from FHWA or certification that the barrier meets the requirements of Ga. Std. 4961. Submit all certification documents to the engineer prior to delivery of the barrier to the project.

Method 2 - Submit certification from the manufacturer that the proposed barrier and its interconnecting hardware replicates an NCHRP 350 “Test Level 3” approved documented in an acceptance letter from FHWA and that the barrier does not deflect more than 1'-0" (300mm) under NCHRP test conditions. Attach the acceptance letter stating that the proposed is in compliance with NCHRP 350 “Test Level 3” and that the barrier meets the deflection criteria to the certification. Submit all certification documents to the engineer prior to delivery of the barrier to the project.
Section 620 – Temporary Barrier

620.2 Materials
A. Method 1
Supply a temporary barrier.
Ensure that materials are in accordance with the manufacturer’s recommendations, specifications, and details or that the materials meet the requirements of the Standard Specifications and Ga. Std. 4961.

B. Method 2
Supply a temporary barrier.
Ensure that materials used in the barrier are in accordance with the manufacturer’s recommendations, Specifications, and details.

620.2.01 Delivery, Storage, and Handling
A. General
Deliver, store, and handle barrier in accordance with the manufacturer’s recommendations.
Repair damage to the barrier and its connections in accordance with the manufacturer’s recommendations at no additional cost to the Department prior to acceptance for use by the Department.

620.3 Construction Requirements
620.3.01 Personnel
General Provisions 101 through 150.

620.3.02 Equipment
General Provisions 101 through 150.

620.3.03 Preparation
General Provisions 101 through 150

620.3.04 Fabrication
A. Method 1
Perform barrier fabrication as detailed on Ga. Std. 4961 or in accordance with the manufacturer’s recommendations.
B. Method 2
Perform barrier fabrication in accordance with the manufacturer’s recommendations.

620.3.05 Construction
A. General
Handle and transport units to prevent damage and/or as recommended by the manufacturer. When required, use units at one or more sites on the same project.
Ensure that the units are complete and in acceptable condition and located where designated on the Plans or directed by the Engineer before acceptance by the Department.
Use the Plan quantity of barrier effectively to complete The Work within the Contract time. If scheduling The Work requires additional barrier, furnish it at no additional expense to the Department.
Use only one section shape, length, and connection type in a single run of interconnected barrier.
Section 620 – Temporary Barrier

B. Method 2

Rigidly attach the barrier to the bridge deck and extend it off the bridge a transition distance indicated in the Standard Plans.

Interconnect all barrier sections within each single run of barrier.

Use non-shrink grout to fill all holes remaining in permanent bridge decks after barrier is removed.

620.3.06 Quality Assurance

General Provisions 101 through 150.

620.3.07 Contractor Warranty and Maintenance

General Provisions 101 through 150.

620.4 Measurement

No measurement to be included for this item.

620.4.01 Limits

General Provisions 101 through 150.

620.5 Payment

Payment will be made under CONSTRUCTION COMPLETE. Includes fabrication, use, moving, reuse, and removal of the units.

No separate payment will be made for moving and/or reusing units during the work or for using additional units beyond the Plan quantity to facilitate the construction schedule.

No separate payment will be made for filling holes used to bolt Method 2 barrier to bridge decks.
620.5.01 Adjustments

General Provisions 101 through 150.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

Project Number:  CSNHS-0006-00 (043)
P.I. Number:  0006043
Cherokee County

SPECIAL PROVISION

Section 624—Sound Barriers

Delete Sub-section 624.2.B and substitute the following:

B. Type C

Use precast concrete panels that meet these requirements:

<table>
<thead>
<tr>
<th>Class AA Concrete</th>
<th>Section 500</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reinforcing</td>
<td>AASHTO M 31/M 31M and M 32/M 32M</td>
</tr>
<tr>
<td>Piling-Galvanized Steel</td>
<td>Section 520 and AASHTO M 111/M 111M</td>
</tr>
<tr>
<td>Elastomeric Bearing Pads</td>
<td>Section 885</td>
</tr>
</tbody>
</table>

Use piling, bolts, and fittings that are hot-dip galvanized when the barrier rests on another concrete structure.

Delete Sub-section 624.3.F.2.e and substitute the following:

e. Give the panels a Type III—Rubbed Finish on the upper surface (as cast) according to Subsection 500.3.05.AB, “Finish Concrete.” When an architectural finish is specified for one side of the barrier, provide a similar finish to the opposite side unless noted otherwise in the plans.

Delete Sub-section 624.3.F.5.c and substitute the following:

c. Cast the panels on a steel surface with steel side forms. When an architectural finish is specified for one side of the barrier, provide a similar finish to the opposite side unless noted otherwise in the plans.
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  

PROJECT NO.: CSNHS-0006-00 (043)  
P. I. NO. 0006043  
CHEROKEE COUNTY  

Section 627—Mechanically Stabilized Embankment Retaining Wall-Contractor Design  

Delete Section 627.2 and substitute the following:  

627.2 Materials  
Meet the requirements of Section 626.2, “Materials” of the Specifications.  

Delete Subsection 627.3.03.B and substitute the following:  

B. Wall Design  

Use the following design criteria for a Contractor designed wall:  

1. Provide one of the following wall systems:  
   - ARES (Tensar Earth Technologies)  
   - Reinforced Earth Wall (The Reinforced Earth Company)  
   - Tricon Retained Soil Wall (Tricon Precast)  
   - Stabilized Earth Wall (T&B Structural Systems)  

2. Design the MSE Wall according to the current AASHTO Standard Specifications for Highway Bridges including interims. (Mechanically Stabilized Earth Wall Design – Section 5.8)  

3. Design the MSE wall to account for all live load, dead load and wind load from all traffic barrier, lights, overhead signs, sound barriers and other appurtenances located on top and adjacent to the wall. Design MSE walls to account for all external forces. Also, design abutment walls for all horizontal and vertical loads applied by the bridge.  

4. Assume responsibility for all temporary shoring that may be necessary for wall construction. Design the shoring using sound engineering principles.  

5. Use permanent concrete wall facing panels that are at least 7 in (175 mm) thick.  

6. Provide a minimum length of soil reinforcement of 10 feet (3 m) or seven-tenths (0.7) of the wall height, whichever is greater.  

7. Ensure that the special wall backfill extends a minimum of 12 in (300 mm) past the end of the soil reinforcement.
8. Use the Architectural treatment of facing panels as indicated on the Department’s drawings.
9. Provide internal walls to allow for future widening if shown on the wall envelope. Ensure the internal walls have galvanized wire or concrete facing. Ensure as a minimum that the facing of the internal walls extend to the back limit of the MSE Wall Backfill for the permanent wall.
10. Ensure the maximum panel area does not exceed 35 square feet (3.25 square meters).
11. Design the barrier for a 500 lbs. per linear foot (744 kilograms per linear meter) loading applied horizontally along the top of the barrier. The barrier shall be continuous or have a counterweight slab continuous over not less than four panels.
12. A Foundation Investigation Report may be available from the Geotechnical Engineering Bureau of the Department. The information contained in this report may be used by the Contractor to assist in evaluating existing conditions for design as well as construction. However, the accuracy of the information is not guaranteed and no requests for additional monies or time extensions will be considered as a result of the Contractor relying on the information in this report.
13. Ensure the following requirements are met:
   The gutterline grade on the proposed top of wall submitted matches the gutter elevations required by the plans.
   The top of coping is at or above the top of coping shown on the envelope.
   The leveling pad is at or below the elevation shown on the wall envelope.
   Any changes in wall pay quantities due to changes in the wall envelope are noted in the contractor’s plans.
   All changes in quantities due to the proposed walls being outside the wall envelope (step locations, ending wall at full panel, etc.) are shown as separate quantities.
14. Ensure the minimum embedment of the wall (top of leveling pad) is at least 2 feet (600 mm). If the soil slopes away from the bottom of the wall, lower the bottom of the wall to provide a minimum horizontal distance of 10 ft (3 m) to the slope. [i.e. a 2:1 slope in front of the wall requires 5 ft (1.5 m) of embedment; a 4:1 slope in front of the wall requires 2.5 ft (750 mm) of embedment]
15. If the Department’s review of the submitted plans and calculations results in more than two submittals to the Department by the Contractor, the Contractor will be assessed for all reviews in excess of two submittals. The assessment for these additional reviews will be at the rate of $60.00 per hour of engineering time expended.

Delete Subsection 627.3.04 and substitute the following:

627.3.04 Fabrication
Meet the requirements of Section 626.3.04 of the Specifications.

Delete Subsection 627.3.05 and substitute the following:

627.3.05 Construction
Meet the requirements of Section 626.3.05 of the Specifications.
Delete Subsection 632.3 and substitute the following:

632.3 Construction Requirements

632.3.01 Personnel
General Provisions 101 through 150.

632.3.02 Equipment
Use Portable Changeable Message Sign (PCMS) meeting the requirements of MUTCD, Section 6F.55 Portable Changeable Message Signs and the following:

A. Completed a full evaluation cycle (1-year) by National Transportation Product Evaluation Program (NTPEP).
B. Passed NTPEP durability test.
C. Has a control system with a keyboard to allow programming of user defined messages.
D. Have primary and backup power sources.
E. Capable of adjusting its brightness from daylight to night time conditions.
F. Capable of displaying 3 lines of legend.
G. Has a minimum reliability from its primary power supply for a minimum of 14 days for solar units (5 days for diesel units). Gasoline powered units not allowed.
H. Message displayed on the sign is visible for 3000 feet (915 m) and legible for not less than 650 ft (198 m) during both daytime and nighttime operation.
I. Is a self-contained unit including a control system with keyboard, primary and backup power source, mounting and transporting equipment (trailer mounted with all applicable lights and hardware).
J. Bottom of message sign panel is capable of being raised a minimum of 7 feet above the roadway.
K. Listed on QPL 82 as maintained by the Office of Materials and Research or have a letter of approval from the Office of Materials and Research before the sign is used on any portion of the worksite.
L. PCMS that remain the property of the Contractor may be either new or used provided the PCMS meets the requirements of this Subsection.
M. In addition to the alphanumeric combinations, the signs should include the capability to display directional arrow messages. A PCMS may be used as an arrow board display panel provided the PCMS meets the size and display requirement of a Type C panel as defined by the MUTCD, Section 6F.55 Portable Changeable Message Signs.
N. The PCMS has the following programmed as permanent messages:
   1) /KEEP/RIGHT/ / 
   2) /KEEP/LEFT/ / 
   3) /TWO WAY/ TRAFFIC/AHEAD/ 
   4) /ONE LANE/BRIDGE/AHEAD/
5) /MERGING/TRAFFIC/AHEAD/
6) /HEAVY/TRAFFIC/AHEAD/
7) /BUMP/AHEAD/ /
8) /PAINT/CREW/AHEAD/
9) /LOOSE/GRAVEL/AHEAD/
10) /SURVEY/PARTY/AHEAD/
11) /ICY/BRIDGE/AHEAD/
12) /ROUGH/ROAD/AHEAD/
13) /DO/NOT/PASS/
14) /LOW/SOFT/SOULDER/ /
15) /SHOULDER/DROPOFF/ /
16) VEHICLES/CROSSING/ROADWAY/
17) /DETOUR/AHEAD/ /
18) /MERGE/RIGHT/AHEAD/
19) /MERGE/LEFT/AHEAD/
20) /TRAFFIC/ACCIDENT/AHEAD/
21) /TRAFFIC/SLOWS/AHEAD/
22) /ROAD/NARROWS/AHEAD/
23) /LEFT/LANE/NARROWS/
24) /RIGHT/LANE/NARROWS/
25) /LANE/NARROWS/AHEAD/
26) /LEFT/LANE/ /
27) /RIGHT/LANE/ /
28) /LEFT/SOULDER/ /
29) /RIGHT/SOULDER/ /
30) /CLOSED/AHEAD/ /

O. The PCMS is entirely mounted on a trailer that meets all of the requirements of the Georgia Vehicle Code. Additional trailer requirements:

1. The trailer and the components of the sign is designed to allow one person to perform all transporting and operating functions without assistance.
2. The trailer is designed for unlimited on-highway travel at 70 mph (110 kph).
3. The trailer has a minimum of four outrigger type leveling jacks, one at each corner of the trailer deck.
4. The jacks are mounted to allow them to swivel into a locked position for secure storage during travel.
5. The trailer and all mounted equipment are structurally adequate for unlimited normal operation in wind velocities up to 80 mph (130 kph).

632.3.03 Preparation
General Provisions 101 through 150.

632.3.04 Fabrication
General Provisions 101 through 150.

632.3.05 Construction
A. Utilization Requirements

1. When set up as a Pay Item in the Contract, utilize PCMS whenever any condition(s) exists that would require extra emphasis in warning motorists of a situation or at any location as directed by the Engineer. Furnish PCMS and have them available on a continuous basis.
2. Use PCMS on Interstate, limited access and multi-lane divided highways when any of these conditions exist:
   a. Workers or equipment operating with in 2 ft. (600 mm) of a travel lane without appropriate traffic control devices for positive barrier protection.
b. Excavation or other construction creates drop-offs adjacent to the edge of a travel lane and channelization devices are placed within the travel lane that is adjacent to the drop-off.

c. Material hauling in or out of a travel lane by hauling vehicles requires traffic to slow in the temporary traffic control zone.

d. Traffic is delayed by pacing all lanes for short periods of time for placing bridge beams, overhead sign structures, blasting, etc.

e. Any time that divided highway traffic is required to operate as two-way traffic condition and traffic is not separated by a positive barrier system.

f. One mile in advance of lane closure, place PCMS on outside shoulder denoting appropriate lane closure one mile ahead.

3. Use PCMS on all other types of roadways according to the traffic control plan or as directed by the Engineer.

4. Locate the PCMS near the construction activity and display a message that is both concise and meaningful. Obtain the Engineer’s approval for messages used on the PCMS.

5. Include the location of the PCMS and any message to be displayed on the PCMS in the approved traffic control plan required in Section 150-Traffic Control.

6. For emergency situations, PCMS that are smaller in size and do not have all of the capabilities outlined in this Specification, may be used until a PCMS that meets these requirements can be located and placed in operation at the site.

The Engineer will determine when conditions and situations are to be considered emergencies and will regulate the length of time that non-specification PCMS may be used.

Provide the Engineer written notification when non-specification PCMS signs are in use on the work.

B. PCMS Phase Messages

1. Messages are displayed in preferably one phase but no more than two phases.

2. The first phase directs the motorist to take a specific action, such as MERGE/RIGHT, KEEP/RIGHT, OR REDUCE/SPEED.

3. The second phase, if necessary, is used to inform the motorist of road conditions such as LEFT/LANE/CLOSED; LANE/NARROWS/AHEAD; WATER/IN/ROAD; SHOULDER/DROP OFF; TRUCKS/IN AND/OUT.

4. Do not use messages such as USE/CAUTION; HAZARD/AHEAD; or DANGER which are confusing and give no guidance to the motorist. Also, do not use messages such as BUCKLE/UP or DRIVE/SAFELY which diminish the impact of important and relevant messages.

632.3.06 Quality Acceptance

General Provisions 101 through 150.

632.3.07 Contractor Warranty and Maintenance

Keep the units in good repair and neat and clean in appearance. If the unit fails, malfunctions, or is damaged, immediately repair the unit and furnish flaggers or other approved means to safely control the traffic until the units are back in service. Make repairs or replace the unit within 24 hours. Maintenance also includes periodically cleaning the units.

Office of Materials & Research
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

Special Provision  

Project Number:  CSNHS-0006-00(043)  
P.I. Number:  0006043  
Cherokee County  

Section 636—Highway Signs  

Delete Subsection 636.3.05.E and substitute the following:  

E. Legends and Borders  

Place legends and borders according to Subsection 917.2.01, “Demountable Characters”, with Type IX reflective sheeting.  

Delete Subsection 636.4 and substitute the following:  

636.4 Measurement  
No separate measurement will be included for this item.  

A. Type-1 or Type-2 Highway Signs  
Type 1 or Type 2 highway signs with reflective sheeting of Type III, IV, or IX includes providing the message and furnishing and placing signs complete and accepted.  

B. Extruded Aluminum Panels  
Includes legend components, border material, fittings, nuts, washers, clamps, molding, etc., furnished, erected, completed, and accepted.  

C. Galvanized Steel Posts  
Includes number of pounds (kilograms) furnished, erected, and accepted as well as base plates, connections, anchors, stub post, etc.  

D. Delineators  
Includes posts, rivets, and spacers, that are furnished, placed, and completed and accepted.  

E. Mast Arm Assemblies  
Includes actual number furnished and erected and concrete footing, sign, and post, completed and accepted.  

F. Special Roadside Signs  
No deductions are made for the volume of concrete displaced by steel piling, anchor bolts, or posts.  

G. Portland Cement  
Portland cement stabilized material used for backfilling holes is not measured for payment.
Section 636—Highway Signs

H. Steel H—Piling

No separate measurement will be included for this item.

636.4.01 Limits

General Provisions 101 through 150.

_Delete Subsection 636.5 and substitute the following:_

636.5 Payment

Payment will be made under CONSTRUCTION COMPLETE.

Office of Materials and Research
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

SECTION 639 – STRAIN POLES FOR OVERHEAD SIGN AND SIGNAL ASSEMBLIES

Retain Sub-Section 639.2 as written and add the following sub-section:

Section 639 - Strain Poles for Overhead Sign and Signal Assemblies

D. Strain Poles for ATMS Applications

Provide poles for supporting CCTV, VDS, and microwave radar detection devices that meet the following design specifications:

• Designed to 80 mph AASHTO wind load requirements
• Limited to a live horizontal deflection at the top equal to or less than 1% of pole height in a 50 mph wind, with a design load of four static cameras and one movable camera.
• Torsional deflection limited to a 1 degree, maximum.

Install mounting brackets, as illustrated on the plans, that are galvanized steel and are compatible with the mounting design of the specified cameras and pan/tilt devices, and are affixed to the pole to prohibit rotation.

Install all wiring internal up to the camera mounting bracket with no external conduit on the pole.

Provide a weatherproof wiring access point or handhole on the pole.
Delete Subsection 647.3.07.A and substitute the following:

647.3.07 Contractor Warranty and Maintenance

A. Traffic Signal Equipment Maintenance

Perform an inspection with the Engineer to determine the operational status of existing field equipment and finalize materials and equipment that is to be removed due to the project.

Prepare written report identifying what equipment was operational and non-operational and responsibility for repair.

Functional responsibility for new traffic signal equipment installed will become the responsibility of the Contractor until acceptance of the project. Contractor responsibility for operation, maintenance and response to reports of operational or equipment malfunction for existing or newly installed signal material at the intersection begins from the issuance of the Notice to Proceed (NTP) until Final Acceptance of the project.

Measure and document existing vertical signal head clearance during the inspection. Maintain existing vertical clearances until Final Acceptance.

Failure to measure and document vertical clearances as part of the inspection will require that all signals be maintained with a vertical clearance of 17 feet (5.1 m) until Final Acceptance. Maintain newly installed signals continuously as detailed in following sections, until Final Acceptance.

Provide a telephone number where the Worksite Traffic Control Supervisor (WTCS) or responsible representative of the Contractor can be reached twenty four (24) hours a day seven (7) days a week in the event of an emergency.

If a signal is not functioning properly:

1. Non-Emergency

Commence work on this signal within three (3) days of the delivery of written notice or e-mail from the Engineer. Failure to respond within three (3) days will result in liquidated damages in the amount of $1,000.00 per day, or portion of, until the work is complete.
In addition, the cost of labor and materials will be charged by the Department if the Department takes corrective action after the three (3) days from written notice using its own forces or local municipality forces.

The department or local municipality will not be held responsible or liable for any alleged damage to the signal or as a result of the signal malfunction due to problems that may occur after the Department or local municipality forces make repairs.
2. Emergency

If the Engineer determines that the signal malfunction or failure is an operational hazard, the Contractor is to take corrective action within three (3) hours of the first attempt of verbal notification. Response shall be considered only when qualified personnel and equipment are provided.

Failure to respond within three (3) hours will result in a non-refundable deduction of money of $1,000.00 with an additional charge of $500.00 per hour after the first three (3) hours until qualified personnel and equipment arrives on site and begins corrective action.

In addition, the cost of labor and material will be charged by the Department if the Department takes corrective action after the three (3) hours notification using its own forces or local municipality forces.

Total charges will not exceed $10,000.00 (per emergency call) in addition to the material cost and labor incurred to make repairs by the Department or local municipality forces respond to the malfunction.

The Department or local municipality will not be held responsible or liable for any alleged damage to the signal or as a result of the signal malfunction due to problems that may occur after Department or local municipality forces make emergency repairs.

The Contractor shall be responsible for all materials and equipment necessary to correct signal malfunction or repair.

Final Acceptance will not be given until payment for such work is received.
Delete Section 648 and substitute the following:

**648.1 General Description**

This work includes furnishing and installing impact attenuator units/arrays to conform with Plan locations and details and/or as directed by the Engineer. All impact attenuator units/arrays shall be tested and approved at the specified NCHRP 350 Test Level.

**648.1.01 Definitions**

General Provisions 101 through 150.

Gating - A gating end treatment allows a vehicle impacting the nose or the side of the unit at an angle near the nose to pass through the device.

Non-Gating - A non-gating end treatment is capable of redirecting a vehicle impacting the nose or the side of the unit along the unit’s entire length.

**648.1.02 Related References**

A. **Standard Specifications**

   General Provisions 101 through 150

B. **Referenced Documents**

   - ASTM A 123/A 123M
   - QPL 64
   - Roadside Design Guide

**648.1.03 Submittals**

A. **Installation Drawings**

Submit all required certifications, test reports and drawings of details for completing the installation. Obtain Engineer’s approval of these documents before beginning work on attenuator installation. Portable Impact Attenuator arrays shall meet the requirements of Ga. Std. 4960 and manufacturer specifications.
Section 648 – Traffic Impact Attenuator

B. Manufacturer’s Information

Submit certification from the manufacturer that the attenuator unit/array and its interconnecting hardware replicates an NCHRP-350 approved attenuator in an accepted letter from the FHWA. Furnish items such as manufacturer’s brochures or specifications that completely outline the manufacturer’s recommendations for materials and installation methods. All workmanship and materials are subject to the Engineer’s approval.

648.2 Materials

A. Attenuator

1. Ensure that materials are in accordance with the manufacturer’s recommendations, specifications and details.

2. Use attenuators that have been classified as “accepted” by the Department’s Office of Materials and Research and approved by the Federal Highway Administration (FHWA) as meeting NCHRP-350 for the test level specified.

3. Where restoration and/or repair cannot be accomplished without the necessity of removing the unit/array from the original location, ensure replacement unit/array installation upon removal of the damaged unit/array. Furnishing, installing and maintaining the replacement will be at no additional cost.

4. Where required, ensure the approach end of the attenuator is equipped with a reflectorized object marker in accordance with Plan Details. The object marker may be furnished by the manufacturer of the attenuator or by others. Ensure that the front most section of the unit (the “nose”) is yellow in color unless specified otherwise.

5. Where required, use an approved back-up system as specified in the Plans.

6. For non-gating attenuators, anchor the attenuator to the pavement according to a system recommended by the manufacturer for the type pavement encountered.

7. Use Class “A” concrete for reinforced concrete pads, concrete back up if used, and concrete transition where required.

8. Use metal components and hardware galvanized according to ASTM A 123/A 123M unless otherwise specified. Ensure all metal components and hardware of permanent attenuators are free of corrosion when shipped.

9. In freezing conditions, water filled attenuators shall be treated according to the manufacturer’s recommendations.

648.2.01 Delivery, Storage, and Handling

A. General

General Provisions 101 through 150.

648.3 Construction Requirements

648.3.01 Personnel

General Provisions 101 through 150.

648.3.02 Equipment

General Provisions 101 through 150.

648.3.03 Preparation

General Provisions 101 through 150.

648.3.04 Fabrication

A. Design Criteria and Type Selection

The Impact Attenuator Unit/array Type will be shown on the plans and designated by four characters.

- **First character**
  Indicates the type of permanent installation.
  The letter “P” designates a permanent (non-gating) installation.
  The letter “S” designates a permanent self restoring (non-gating) installation which is capable of withstanding multiple hits without requiring repair or adjustment.

- **Second character**
  Designates the required NCHRP test level.
Section 648 – Traffic Impact Attenuator

- **Third character**
  Indicates the traffic flow direction(s).
  The letter "B" indicates bi-directional traffic typical for median applications or when the unit is installed on the shoulder of a two-lane, two-way traffic facility. Bi-directional means traffic flows in opposite directions at the site of the attenuator installation.
  The letter "U" indicates uni-directional traffic flow typical for gore areas. Uni-directional means traffic on both sides traveling the same direction, from the nose to the rear of the unit.
  The letter "S" indicates traffic flow in one direction on a single side only, typical for a unit located on the outside shoulder of a roadway with one-way traffic and the other side of the attenuator not being exposed to traffic.

- **Fourth character**
  Indicates the numerical value of the width, in inches (millimeters), of the base of the rigid object that the attenuator will be shielding.
  At bridge columns, this character is typically the width of the column plus the barrier base widths on the column sides at the pavement surface.

**B. Example**

A Type P-3-U-60 attenuator designates
- a permanent installation
- tested and approved at NCHRP test level 3
- Uni-directional traffic flow
- a 60" (1500 mm) wide base for the rigid object being shielded.

Temporary portable units/arrays may be either gating or non-gating based on construction sequencing and/or field conditions, See Specification Section 150. Unless otherwise specified, all permanent attenuators shall be non-gating.

648.3.05 Construction

Field locate the position of the attenuator nose as shown on the plans prior to beginning the installation. Have any variations approved by the Engineer.

If the length of the attenuator unit/array is less than that indicated in the plan details for the specified conditions, the length of the concrete transition section or the length of the longitudinal barrier shall be increased as needed to provide a proper beginning point for the attenuator nose as shown in the plans.

The length of the system will be the combined length of the attenuator unit/array, the back-up system and any required transition. The length of the system shall not be excessive to the extent that it intrudes appreciably within the clear offset distance as shown on the plans.

The increased length of transition or barrier is considered as an incidental part of the system and will not be itemized separately.

Temporary portable units/arrays shall be installed, moved, reinstalled and maintained as required.

648.3.06 Quality Assurance

Obtain certification from the manufacturer that the impact attenuator unit/array installed meets all required approvals and specifications and furnish these to the Engineer.

Furnish any mill test/galvanizing test reports and heat numbers for all metal components of the unit per current requirements of the Department's Office of Materials and Research.

648.3.07 Contractor Warranty and Maintenance

General Provisions 101 through 150.

648.4 Measurement

No measurement to be included for this item. Includes components, hardware, anchors, incidentals, freeze treated water or sand, and labor for each installation shown on the plans or as directed by the Engineer.
648.4.01 Limits

General Provisions 101 through 150.

648.5 Payment

Payment will be made under CONSTRUCTION COMPLETE. Includes materials, labor, and incidentals necessary to complete the Item including installing, moving, reinstalling and maintaining Units/Arrays as required and the back-up system and transitions where required.

648.5.01 Adjustments

General Provisions 101 through 150.
Section 652—Painting Traffic Stripe

Delete Subsection 652.1 and replace with the following:

652.1 General Description
This work includes furnishing and applying reflectorized traffic line paint according to the Plans and these Specifications.
This Item also includes applying words and symbols according to Plan details, Specifications, and the current Manual on Uniform Traffic Control Devices.

652.1.01 Definitions
Painted Stripes: Solid or broken (skip) lines. The location and color are designated on the Plans.
Skip Traffic Stripes: Painted segments between unpainted gaps as specified on the Plans. The location and color are designated on the Plans.

652.1.02 Related References
A. Standard Specifications
   Section 656—Removal of Pavement Markings
   Section 870—Paint
   EPA Method 3052
   EPA Method 601B
B. Referenced Documents
   QPL 46
   AASHTO M 247

652.1.03 Submittals
General Provisions 101 through 150.

Delete Subsection 652.2 and replace with the following:

652.2 Materials
Ensure that materials for painting traffic stripe, words, and symbols meet the following requirements:
Section 652—Painting Traffic Stripe

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Traffic Line Paint 5A and 5B</td>
<td>870.2.02.A.2 and</td>
</tr>
<tr>
<td></td>
<td>870.2.02.A.3</td>
</tr>
<tr>
<td>Glass Beads for Use in Luminous Traffic Lines</td>
<td>*AASHTO M 247 Type 1, 2, or 3</td>
</tr>
</tbody>
</table>

*In addition, meet the following requirements for glass beads:

- Maximum quantity of angular particles is less than 1% by weight
- Maximum quantity of particles with milkiness, scoring, or scratching is less than 2% by weight
- Glass beads do not impart any noticeable hue to the paint film
- Glass beads do not contain greater than 200 ppm total arsenic, antimony, or lead when tested according to US EPA Methods 3052 and 601B.
- Glass beads conforming to the following alternate gradation may be used provided that all other requirements of AASHTO M 247 and this Specification are met.

<table>
<thead>
<tr>
<th>Alternate Gradation</th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Sieve Size</td>
<td>Percent Passing</td>
</tr>
<tr>
<td>No. 16 (1.190 mm)</td>
<td>99 - 100</td>
</tr>
<tr>
<td>No. 20 (0.850 mm)</td>
<td>75 - 95</td>
</tr>
<tr>
<td>No. 30 (0.600 mm)</td>
<td>55 – 85</td>
</tr>
<tr>
<td>No. 50 (0.300 mm)</td>
<td>10 – 35</td>
</tr>
<tr>
<td>No. 100 (0.150 mm)</td>
<td>0 - 5</td>
</tr>
</tbody>
</table>

652.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

Office of Materials and Research

Section 652—Polyurea Traffic Stripe

Add the following:

652.1 General Description
This work includes furnishing and applying reflectorized traffic line paint according to the Plans and these Specifications.
This Item also includes applying words and symbols according to Plan details, Specifications, and the current Manual on Uniform Traffic Control Devices.

652.1.01 Definitions
Painted Stripes: Solid or broken (skip) lines. The location and color are designated on the Plans.

Skip Traffic Stripes: Painted segments between unpainted gaps on a designated sequence with a ratio of 1:3 [10 ft (3 m) segment and 30 ft (9 m) gap] as specified on the Plans. The location and color are designated on the Plans.

652.1.02 Related References
A. Standard Specifications
   General Provisions 101 through 150.
   Section 656—Removal of Pavement Markings
Section 652—Painting Traffic Stripe

Section 870—Paint

B. Referenced Documents

QPL 46
AASHTO M 247
ACI Method 503
ASTM C 4060
ASTM D 711
ASTM D 1155
ASTM D 1213
ASTM D 4061
ASTM D 6359
ASTM E 303
ASTM E 1710
ASTM G 154
Federal Standard No. 595A-17778

652.2 Materials

A. Polyurea Composition Requirements

- Use a polyurea composition that is specifically formulated for use as a durable pavement marking material and for application at elevated temperatures not exceeding 170 °F (77 °C).
- Ensure the liquid markings consist of a two-component (Part A and Part B), 100% solids polyurea film formulated and designed to provide a simple volumetric mixing ratio as recommended by the manufacturer.
- Use white or yellow films for the markings. Ensure that these films are manufactured without the use of lead chromate pigments or other similar, lead-containing chemicals.
- Ensure that the white polyurea contains not less than 13% by weight rutile titanium dioxide pigment to insure adequate opacity, hiding power, and reflective properties.

B. Glass Beads and Ceramic Reflective Elements

Use glass beads and/or ceramic reflective elements for the reflective media system that ensures the polyurea pavement markings meet the reflectance performance requirements in Section 652.2.C.2.

C. Finished Product Requirements:

1. Composition

   Ensure that the retroreflective pavement markings consist of a mixture of high-quality resins, curing agent and pigments, with a reflective layer bonded to the top surface consisting of glass beads.

2. Reflectance

   When applied according to the manufacturer’s recommendations, ensure that the white and yellow markings have the average initial and 12 months retroreflectance values shown in the tables below, as measured in accordance with the testing procedures of ASTM D4061 or ASTM E 1710.

   An observation angle of 1.05° and an entrance angle of 88.8° corresponds to 30 meter geometry. The photometric quantity to be measured is the coefficient of retroreflected luminance \((R_1)\) and is expressed as millicandelas per square foot per foot-candle \([\text{mcd}\cdot\text{ft}^{-2}\cdot\text{fc}^{-1}]\). The metric equivalent is expressed as millicandelas per square meter per lux \([\text{mcd}\cdot\text{m}^{-2}\cdot\text{lx}^{-1}]\).

   Determine the initial and 12 months retroreflectance of a single installation according to the measurement and sampling procedures outlined in ASTM D 6359, using a 30-meter retroreflectometer.
3. Color

Meet these color requirements:
- White markings are pure white and free from dirt or tint.
- Yellow markings are “Federal Yellow” in color.
- The material does not change its color and brightness characteristics after prolonged exposure to sunlight.

4. Skid Resistance

Ensure the surface of the retroreflective marking provides an initial average skid resistance value of 45 BPN when tested according to ASTM E303.

5. Color and Weathering Resistance

Ensure that the mixed polyurea compound, both white and yellow, when applied to 3 in (75 mm) x 6in (150 mm) aluminum panels at 15 ± 1 mils (0.381 mm ± 0.025 mm) wet thickness without glass beads and exposed in a Q.U.V. Environmental Testing Chamber, as described in ASTM G-53-77, conforms to the following minimum requirements:
- The color of the white polyurea compound is not darker than Federal Standard No. 595A-17778.
- The color of the yellow polyurea compound meets the requirements of the “Federal Yellow” color chart.

6. Drying Time (Laboratory)

When tested in accordance with ASTM D-711 the polyurea marking material shall reach a no-pick-up condition in 10 minutes or less. Perform this test with ASHTO M247 Type 1 beads applied at a rate of 0.099 pounds per square foot (0.483 kg/m²). Ensure that the drying time does not increase substantially with decreasing temperature.

7. Drying Time (Field)

When installed at 77 °F (25 °C), at a wet film thickness of 20 ± 2 mils (0.508 mm ± 0.051 mm) and reflectorized with glass beads and/or ceramic reflective elements, ensure that the polyurea markings reaches a no-track condition in less than 10 minutes. Dry to “no-tracking” will be considered as the condition where no visual deposition of the polyurea marking to the pavement surface is observed when viewed from a distance of 50 feet (15 m), after a traveling vehicle’s tires have passed over the marking.

8. Abrasion Resistance

Ensure that the wear index of the polyurea compound does not exceed 0.00026 lbs (120 mg) when tested in accordance with ASTM C4060 using a CS-17 wheel and under a load of 2.2 lbs (1000 g) for 1000 cycles.

9. Adhesion to Concrete
Section 652—Painting Traffic Stripe

Ensure that the polyurea pavement marking materials, when tested according to ACI Method 503, have such a high degree of adhesion to the specified concrete surface that there is a 100% concrete failure in the performance of this test. Condition the prepared specimens at room temperature 75 ° ± 2 °F (24 °C) for a minimum of 24 hours and maximum of 72 hours prior to the performance of this test.

10. Adhesion to Asphalt

Ensure that the polyurea pavement marking materials, when tested according to ACI Method 503, have such a high degree of adhesion to the specified asphalt surface that there is a 100% asphalt failure in the performance of this test. Condition the prepared specimens at room temperature 75 ° ± 2 °F (24 °C) for a minimum of 24 hours and maximum of 72 hours prior to the performance of this test.

652.3 Construction Requirements

652.3.01 Equipment

A. Traveling Traffic Stripe Painter

To apply the traffic marking material, use a mobile, truck mounted and self contained pavement marking machine, specifically designed to apply two-component liquid materials, and glass beads, in a continuous and skip-line pattern.

Apply the two-component liquid materials through airless impingement mixing guns. The guns must accommodate a plural component material system at the manufacturer’s recommended volumetric mixing ratio. The guns must have the capacity to deliver materials from approximately 1.5 (5.7 L) to 3 gal (11.4 L) per minute to compensate for a typical range of application speeds of 3 mph (5 km/h) to 6 mph (10 km/h). Ensure that the machine travels at a uniform rate of speed both uphill and downhill.

Select the necessary accessories such as spray tip, mix chamber or static tube, and rod diameter to ensure proper mixing.

Ensure that the machine meets the following:

- The machine is capable of applying three separate stripes, either solid or skip, in any specified pattern by utilizing three adjacent spray nozzles at the same time.
- Each nozzle is equipped with satisfactory cutoff valves that will apply skip lines automatically.
- The application equipment is maneuverable to the extent that straight lines can be followed and normal curves can be made in a true arc.
- The truck-mounted unit is provided with accessories to allow for the marking of symbols and legends.

Ensure that the mobile applicator also includes the following features:

- The mobile applicator provides individual material reservoirs for the storage of Part A and Part B of the resin composition.
- The applicator is equipped with heating equipment of sufficient capacity to maintain the individual resin components at the manufacturer’s recommended temperature for spray application.
- The applicator is equipped with separate temperature controls for each component.
- The applicator is equipped with glass bead dispensing equipment and capable of applying the glass beads at a uniform rate.
- The application equipment is equipped with metering devices or pressure gauges on the proportioning pumps as well as stroke counters to monitor volumetric usage. Ensure that the metering devices or pressure gauges and stroke counters are visible.
- The applicator is equipped with all the necessary spray equipment, mixers, compressors, and other appurtenances to allow for the placement of reflectorized pavement markings in a simultaneous sequence of operations.

B. Cleaning Equipment

Use brushes, brooms, scrapers, grinders, high-pressure water jets, or air blasters to remove dirt, dust, grease, oil, and other foreign matter from painting surfaces without damaging the underlying pavement.

652.3.02 Preparation

Before painting, thoroughly clean pavement surfaces of dust, dirt, grease, oil, and all other foreign matter.
Section 652—Painting Traffic Stripe

Remove concrete curing compounds on new Portland cement concrete surfaces and existing pavement markings on both concrete and asphalt surfaces.

652.3.03 Construction

A. Atmospheric Conditions

Apply pavement markings only during conditions of dry weather and subsequently dry pavement surfaces. Ensure that the pavement surface temperature and the ambient temperature at the time of installation are both greater than 40 °F (4 °C) and that the relative humidity is not greater than 85%.

B. Alignment

Ensure that the traffic stripe is the specified length, width, and placement. On sections where no previously applied markings are present, ensure accurate stripe location by establishing control points at spaced intervals. The Engineer will approve control points.

C. Application

Apply the pavement markings as follows:

1. Apply the liquid marking material by spray method and according to the manufacturer’s installation instructions.

2. Ensure marking configurations are in accordance with the “Manual on Uniform Traffic Control Devices.”

3. Place the reflectorized pavement markings only on properly prepared surfaces and at the widths and patterns designated on the Plans. Do not begin marking operations until applicable surface preparation work is completed and approved by the Engineer.

4. Air-blast the surface first, to remove any dirt and residues from the pavement. Then apply the pavement markings as a continuous operation.

5. Heat Component A and Component B to the manufacturer’s recommended temperatures.

6. Ensure that mixing of the two components occurs in a static tube or impingement chamber prior to reaching the application spray nozzle.

7. Spray the mixed resin onto the pavement surface at a minimum uniform wet thickness of 20 mils (0.51 mm).

8. Immediately following application, drop the glass beads and/or ceramic reflective elements onto the liquid marking at the application rates recommended by the binder manufacture.

9. The work will be subject to application rate checks for both paint and beads.

Following an application of glass beads, and upon curing, ensure that the resulting marking is an adherent reflectorized stripe of the specified thickness and width that is capable of resisting deformation by traffic.

D. Protective Measures

Protect newly applied paint as follows:

1. Traffic

   Control and protect traffic with warning and directional signs during painting. Set up warning signs before beginning each operation and place signs well ahead of the painting equipment. When necessary, use a pilot car to protect both the traffic and the painting operation.

2. Fresh Paint

   Protect the freshly painted stripe using cones or other satisfactory devices. Repair stripe damage or pavement smudges caused by traffic according to Subsection 652.3.06.

E. Appearance and Tolerance of Variance

Continually deviating from stated dimensions is cause for stopping the work and removing the nonconforming stripe. (See Section 656.) Adhere to the following measurements:

1. Width

   Do not lay stripe less than the specified width. Do not lay stripe more than 1/2 in (13 mm) over the specified width.

2. Length
Section 652—Painting Traffic Stripe

Ensure that the 10 ft (3 m) painted skip stripe and the 30 ft (10 m) gap between painted segments vary no more than ± 1 ft (300 mm) each.

3. Alignment
   a. Ensure that the stripe does not deviate from the intended alignment by more than 1 in (25 m) on tangents or curves of 1 degree or less.
   b. Ensure that the stripe does not deviate by more than 2 in (50 mm) on curves exceeding 1 degree.

652.3.04 Quality Acceptance

Ensure that stripes and segments of stripes are clean-cut and uniform. Markings that do not appear uniform or satisfactory, either during the day or night, or do not meet Specifications or become marred or damaged by traffic or from other causes, will be corrected at the Contractor’s expense.

Sections of painted stripe, words, and symbols placed according to the Plans and Specifications and have dried so that paint will not be picked up or marred by vehicle tires will be accepted. The Contractor will be relieved of responsibility for maintenance on accepted sections.

A. Correction of Alignment

When correcting a deviation that exceeds the permissible tolerance in alignment, do the following:

1. Remove the affected portion of stripe, plus an additional 25 ft (8 m) in each direction.
2. Paint a new stripe according to these Specifications.

Remove the stripe according to Section 656.

B. Removal of Excess Paint

Remove misted, dripped, or spattered paint to the Engineer’s satisfaction. Do not damage the underlying pavement during removal.

Refer to the applicable portions of Section 656.

652.4 Measurement

No separate measurement will be made for this item.

652.5 Payment

Payment will be made under CONSTRUCTION COMPLETE. It includes:

- Cleaning and preparing surfaces
- Furnishing materials, including paints, beads, and thinners
- Applying, curing, and protecting paints
- Protecting traffic, including providing and placing necessary warning signs
- Furnishing tools, machines, and other equipment necessary to complete the Item
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  

Project Number: CSNHS-0006-00(043)  
P.I. Number: 0006043  
Cherokee County  

Section 653—Thermoplastic Traffic Stripe

Delete Section 653 and substitute the following:

653.1 General Description

This work includes furnishing and applying thermoplastic reflectorized pavement marking compound. Ensure that markings conform to Plan details and locations, these Specifications, and the Manual on Uniform Traffic Control Devices.

Thermoplastic traffic stripe consists of solid or broken (skip) lines, words, and symbols according to Plan color, type, and location.

653.1.01 Definitions

Thermoplastic Marking Compound: A heated compound extruded or mechanically sprayed on the pavement that cools to pavement temperature. When combined with glass spheres it produces a reflectorized pavement marking.

Short Lines: Crosswalks, stop bars, arrows, symbols, and crosshatching. Extrude short lines rather than spraying them on. Unless otherwise specified, spray all other lines.

653.1.02 Related References

A. Standard Specifications

   Section 652—Painting Traffic Stripe

B. Referenced Documents

   QPL 46
   QPL 71
   Federal Test Standard Number 595B
   AASHTO M 249
   ASTM D 92
   ASTM D 476
   ASTM D 762
   ASTM D 2240
   ASTM D 4960
   ASTM E 1710
   40 CFR 261.24
Section 653—Thermoplastic Traffic Stripe

EPA Method 3050
EPA Method 6010
EPA Method 7000
Federal Test Standard Number 595B

653.1.03 Submittals
Ensure that the producers of the thermoplastic compound and glass spheres furnish to the Department copies of certified test reports showing results of all tests specified in this Section. Also ensure that producers certify that the materials meet the other requirements of this Section by submitting copies of certification at the time of sampling.

653.2 Materials
A. Requirements

Ensure the resin of the thermoplastic material is an alkyd binder. Use alkyd binder consisting of a mixture of synthetic resins and a high boiling point plasticizer. Use at least one synthetic resin that is a solid at room temperature. Ensure at least 50% of the binder composition is 100% maleic-modified glycerol ester resin. Ensure at least 15% by weight of the entire material formulation consists of binder. Do not use alkyd binder that contains petroleum based hydrocarbon resins. Ensure the finished thermoplastic material is not adversely affected by contact with pavement materials or by petroleum droppings from traffic. Use thermoplastic material that has been evaluated (2 year field evaluation) by the National Transportation Product Evaluation Panel (NTPEP) test facility or other approved test facility. Use thermoplastic material produced from an approved source listed on QPL 46. Use thermoplastic material that meets the requirements of AASHTO M 249 with the following exceptions:

1. Color

   Confirm the color of thermoplastic as follows:

   a. White – Use titanium dioxide that meets the requirements of ASTM D 476, Type II, Rutile, as the pigment for white thermoplastic material. Do not use anatase titanium dioxide pigment. Use thermoplastic material free from dirt or tint. Ensure white thermoplastic material heated for 240 ± 5 minutes at 425 ± 3 °F (218 ± 3 °C) and cooled to 77 ± 3 °F (25 ± 2 °C) matches Federal Test Standard Number 595B-Color 17925. Use material, when compared to the magnesium oxide standard using a standard color spectrophotometer according to ASTM D 4960, meets the following:

<table>
<thead>
<tr>
<th>Scale</th>
<th>Definition</th>
<th>Magnesium Oxide Standard</th>
<th>Sample</th>
</tr>
</thead>
<tbody>
<tr>
<td>Rd</td>
<td>Reflectance</td>
<td>100</td>
<td>75 min.</td>
</tr>
<tr>
<td>a</td>
<td>Redness-Greenness</td>
<td>0</td>
<td>-5 to + 5</td>
</tr>
<tr>
<td>b</td>
<td>Yellowness-Blueness</td>
<td>0</td>
<td>-10 to + 10</td>
</tr>
</tbody>
</table>

   b. Yellow – Use only non-hazardous pigments as defined by the Resource Conservation and Recovery Act (RCRA) Subarticle C rules, table 1 of 40 CFR 261.24 “Toxicity Characteristic”. Do not use yellow thermoplastic containing more than 3.0 ppm lead by weight when tested in accordance with the most up to date EPA Methods 3050 and 6010 or 7000. Ensure yellow thermoplastic material heated for 240 ± 5 minutes at 425 ± 3 °F (218 ± 2 °C) and cooled to 77 ± 3 °F (25 ± 2 °C) matches Federal Test Standard Number 595B-Color 13538. Use material, when compared to PR#1 Chart using a standard color spectrophotometer according to ASTM D 4960, plots within the following chromaticity coordinates:

<table>
<thead>
<tr>
<th></th>
<th>1</th>
<th>2</th>
<th>3</th>
<th>4</th>
</tr>
</thead>
<tbody>
<tr>
<td>X</td>
<td>0.455</td>
<td>0.510</td>
<td>0.472</td>
<td>0.530</td>
</tr>
<tr>
<td>Y</td>
<td>0.444</td>
<td>0.485</td>
<td>0.400</td>
<td>0.456</td>
</tr>
</tbody>
</table>

   Initial Reflectance (CIE Y): 45 minimum

   Ensure the in-service daytime chromaticity for yellow material plots within the following coordinates after a period of 30 days:
Section 653—Thermoplastic Traffic Stripe

### 2. Indentation Resistance

Measure the hardness by a Shore Durometer, Type A2, as described in ASTM D 2240. Maintain the temperature of the Durometer, 4.4 lb. (2 kg) load and the specimen for 2 hours at 115 °F (45 °C). Apply the Durometer and 4.4 lb. (2 kg) load to the specimen. The reading must fall between 50 to 75 units, after 15 seconds.

### 3. Reheating

Ensure that the compound does not break down, deteriorate, scorch, or discolor if held at application temperature of 425 °F (218 °C) for 6 hours and if reheated up to 4 times to the application temperature. Ensure that the color of white and yellow thermoplastic comply with Subsection 653.2.A.1.a and Subsection 653.2.A.1.b after prolonged heating or reheating.

### 4. Drop-On Glass Spheres

Use spheres that meet the requirements of Subsection 652.2. Also, use spheres produced from an approved source listed on QPL-71.

### 5. Sealing Primer

Place the particular type of two-part epoxy binder-sealer at the application rate as recommended in writing by the thermoplastic material manufacturer.

### 6. Flashpoint

Ensure the thermoplastic flashpoint is not less than 500 ºF (260 ºC) as determined by ASTM D 92.

### 7. Specific Gravity

Ensure the specific gravity of the thermoplastic is between 2.00 to 2.20 as determined by ASTM D 762.

### B. Performance Requirements

#### 1. General

For a minimum of 30 days from the time of placement, ensure the thermoplastic pavement marking material shows no signs of failure due to blistering, excessive cracking, chipping, bleeding, staining, discoloration, oil content of the pavement materials, smearing or spreading under heat, deterioration due to contact with grease deposits, oil, diesel fuel, or gasoline drippings, spilling, poor adhesion to the pavement material, vehicular damage, and normal wear. In the event that failures mentioned above occur, ensure corrective work is completed at no additional cost to the Department.

#### 2. Retroreflectivity

At the time of installation, ensure the in-place markings when tested according to ASTM E 1710 meet the following minimum reflectance values:

- **White**: 375 mcd/lux/m²
- **Yellow**: 250 mcd/lux/m²

Retest the in-place markings 30 days after installation to ensure these minimum retroreflectance values are maintained.

**NOTE: The Contractor is responsible for retroreflectivity testing. Furnish all test reports to the Department.**

In the event failures occur, ensure corrective work is completed at no additional cost to the Department. Perform testing according to ASTM E 1710 at above described intervals. Any retest due to failures will be performed at no additional cost to the Department. Furnish all test reports to the Department.

### 653.2.01 Delivery, Storage, and Handling

Use material delivered in 50 lb (22.7 kg) unit cardboard containers or bags strong enough for normal handling during shipment and on-the-job transportation without loss of material.
Section 653—Thermoplastic Traffic Stripe

Ensure that each unit container is clearly marked to indicate the following:

- Color of the material
- Process batch number or similar manufacturer’s identification
- Manufacturer’s name
- Address of the plant
- Date of manufacture

653.3 Construction Requirements

653.3.01 Personnel
General Provisions 101 through 150.

653.3.02 Equipment
Depending on the marking required, use hand equipment or truck-mounted application units on roadway installations.

A. Spray Application Machine

Ensure that each spray application machine is equipped with the following features:

- Parts continuously mix and agitate the material.
- Truck-mounted units for lane, edge, and center lines can operate at a minimum of 5 mph (8 kph) while installing striping.
- Conveying parts between the main material reservoir and the shaping die or gun prevent accumulation and clogging.
- Parts that contact the material are easily accessible and exposable for cleaning and maintenance.
- Mixing and conveying parts, including the shaping die or gun, maintain the material at the plastic temperature with heat transfer oil or electrical element controlled heat. Do not use an external source of direct heat.
- Parts provide continuously uniform stripe dimensions.
- Applicator cleanly and squarely cuts off stripe ends and applies skip lines. Do not use pans, aprons, or similar appliances that the die overruns.
- Parts produce varying widths of traffic markings.
- Applicator is mobile and maneuverable enough to follow straight lines and make normal curves in a true arc.

B. Automatic Bead Dispenser

Apply glass spheres to the surface of the completed stripe using a dispenser attached to the striping machine to automatically dispense the beads instantaneously upon the installed line. Synchronize the glass sphere dispenser cutoff with the automatic cutoff of the thermoplastic material.

C. Special Kettles

Use special kettles for melting and heating the thermoplastic material. Kettles equipped with automatic thermostatic control devices provide positive temperature control and prevent overheating. Ensure that the applicator and kettles are equipped and arranged according to the requirements of the National Fire Underwriters.

D. Hand Equipment

Use hand equipment for projects with small quantities of lane lines, edge lines, and center lines, or for conditions that require the equipment. Use hand equipment approved by the Engineer.

Ensure that hand equipment can hold 150 lbs (68 kg) of molten material and is maneuverable to install crosswalks, arrows, legends, lane, edge, and center lines.

E. Auxiliary Vehicles

Supply the necessary auxiliary vehicles for the operation.

653.03 Preparation
General Provisions 101 through 150.
653.3.04 Fabrication

General Provisions 101 through 150.

653.3.05 Construction

A. General Application

Thoroughly clean pavement areas to be striped. Use hand brooms, rotary brooms, air blasts, scrapers, or other approved methods that leave the pavement surface clean and undamaged. Take care to remove all vegetation and road film from the striping area. All new Portland Cement Concrete pavement surfaces shall be mechanically wire brushed or abrasive cleaned to remove all laitance and curing compound before being striped.

Lay stripe with continuous uniform dimensions.

Apply the type of stripe at each location according to the Plans, using one of the following methods:

- Spray techniques
- Extrusion methods wherein one side of the shaping die is the pavement, and the other three sides are contained by or are part of the suitable equipment to heat and control the flow of material.

1. Temperature

Apply thermoplastic traffic stripe only when the pavement temperature in the shade is above 40 °F (4 °C).

To ensure optimum adhesion, install the thermoplastic material in a melted state at the manufacturer’s recommended temperature but not at less than 375 °F (190 °C).

2. Moisture

Do not apply when the surface is moist. When directed by the Engineer, perform a moisture test on the Portland cement concrete pavement surface. Perform the test as follows:

a. Place approximately 1 yd² (1 m²) of roofing felt on the pavement surface.

b. Pour approximately 1/2 gallon (2 L) of molten thermoplastic onto the roofing felt.

c. After 2 minutes, lift the roofing felt and inspect to see if moisture is present on the pavement surface or underside of the roofing felt.

d. If moisture is present, do not proceed with the striping operation until the surface has dried sufficiently to be moisture free.

3. Binder-Sealer

To ensure optimum adhesion, apply a binder-sealer material before installing the thermoplastic in each of the following cases:

- Extruded thermoplastic
- Where directed by the Engineer for sprayed thermoplastic
- Old asphaltic concrete pavements with exposed aggregates
- Portland cement concrete pavements as directed by the Engineer

Ensure that the binder-sealer material forms a continuous film that mechanically adheres to the pavement and dries rapidly. Use a binder-sealer currently in use and recommended by the thermoplastic material manufacturer according to QPL-46.

To ensure optimum adhesion, apply a two-part epoxy binder-sealer on all Portland cement concrete pavements for either sprayed or extruded thermoplastic material.

Apply the epoxy binder-sealer immediately in advance of, but concurrent with, the application of the thermoplastic material. Apply in a continuous film over the pavement surface.

4. Bonding to Old Stripe

The old stripe may be renewed by overlaying with new material. Ensure the new material bonds to the old line without splitting or cracking.

5. Offset from Construction Joints

Off-set longitudinal lines at least 2 in (50 mm) from construction joints of Portland cement concrete pavements.

6. Crosswalks, Stop Bars, and Symbols
Make crosswalks, stop bars, and symbols at least 3/32 in (2.4 mm) thick at the edges and no more than 3/16 in (4.8 mm) thick at the center.

7. Film Thickness
   a. Maintain the following minimum average film thicknesses on all open graded asphalt concrete friction courses:
      - 0.120 in (3.0 mm)* for lane lines
      - 0.090 in (2.3 mm)* for edge lines
      - 0.150 in (3.8 mm)* for gore area lines
   b. Maintain the following minimum average film thicknesses on all other pavement types:
      - 0.090 in (2.3 mm)* for lane lines
      - 0.060 in (1.5 mm)* for edge lines
      - 0.120 in (3.0 mm)* for gore area lines

   (See below for ‘*’ reference.)

Compute the minimums by the amount of material used each day, as follows:

| (For 5 in wide stripe) | * Average Film Thickness (in) = \([\text{lbs used}} \div \text{total linear feet}] \times 0.236
| (For 125 mm wide stripe) | *Average Film Thickness (mm) = \([\text{kg used}} \div \text{total linear meters}] \times 4.0
| (For 10 in wide stripe) | * Average Film Thickness (in) = \([\text{lbs used}} \div \text{total linear feet}] \times 0.118
| (For 250 mm wide stripe) | * Average Film Thickness (mm) = \([\text{kg used}} \div \text{total linear meters}] \times 2.0

8. Glass Spheres
   a. Apply glass spheres to installed stripe surface at a minimum rate of 14 lbs of spheres to each 100 square feet ((700 g/m^2) of thermoplastic material.
   b. Apply the glass sphere top-coating with a pressure-type gun specifically designed for applying glass spheres that will embed at least one-half of the sphere’s diameter into the thermoplastic immediately after the material has been applied to the pavement.

B. Removing Existing Stripe

Remove existing stripe according to Section 656.

Remove 100 percent of existing traffic stripe from:
   - Portland cement concrete pavement where the new stripe will be placed at the same location as the existing marking
   - Pavement where the new stripe will be placed at a different location from the existing markings

C. Tolerance and Appearance

No traffic stripe shall be less than the specified width and shall not exceed the specified width by more than 1/2 in (13 mm). The length of the 10 ft (3 m) segment for skip stripe and the 30 ft (9 m) gap between segments may vary plus or minus 1 ft (300 mm). The alignment of the stripe shall not deviate from the intended alignment by more than 1 in (25 mm) on tangents and on curves up to and including 1 degree (radius of 1745 m or greater). On curves exceeding 1 degree (radius less than 1745 m), the alignment of the stripe shall not deviate from the intended alignment by more than 2 in (50 mm).

Stop work when deviation exceeds the above dimensions, and remove the nonconforming stripe.

653.3.06 Quality Acceptance

Segments of the thermoplastic traffic stripe placed according to the Plans and Specifications may be accepted 30 days after the required work is complete in that segment.
If thermoplastic traffic stripe fails to meet Plan details or Specifications or deviates from stated dimensions, correct it at no additional cost to the Department. If removal of pavement markings is necessary, perform it according to Section 656 and place it according to this Specification. No additional payment will be made for removal and replacement of unsatisfactory striping.

653.4 Measurement
No separate measurement will be included for this item.

653.4.01 Limits
General Provisions 101 through 150.

653.5 Payment
Payment will be made under CONSTRUCTION COMPLETE. Includes:

- Cleaning and preparing surfaces
- Furnishing all materials
- Applying, curing, and protecting stripe
- Protecting traffic, including providing necessary warning signs
- Furnishing tools, machines, and other equipment necessary to complete the Item

653.5.01 Adjustments
General Provisions 101 through 150.

Office of Materials and Research
DELETE SUBSECTION 657.1 AND SUBSTITUTE THE FOLLOWING:

657.1 General Description
This work includes placing plastic pavement markings or legends according to the Plans and Specifications or as otherwise directed.

657.1.01 Definitions
General Provisions 101 through 150.

657.1.02 Related References
A. Standard Specifications
   General Provisions 101 through 150.
B. Referenced Documents
   ASTM D 638
   ASTM D 4061
   ASTM E 303
   ASTM E 1710
   Manual on Uniform Traffic Control Devices for Streets and Highways
   QPL 74

657.1.03 Submittals
Transfer to the Department manufacturer warranties or guarantees for heat-applied and wet reflective preformed plastic marking materials. Ensure that warranties or guarantees state that they are subject to transfer.

DELETE SUBSECTION 657.2 AND SUBSTITUTE WITH THE FOLLOWING:

657.2 Materials
Select one of the following types of preformed marking material according to the Plans and Proposal:

- Type TR – Temporary Removable Plastic Marking
Section 657—Preformed Plastic Pavement Markings

- Type TN – Temporary Non-Removable Plastic Marking
- Type PA – Permanent Plastic Marking
- Type PB – Permanent Patterned Plastic Marking
- Type PW – Permanent Wet Reflective Plastic Marking

For a list of sources, see QPL-74.

A. General Requirements for Preformed Pavement Markings

1. Shapes and Sizes
   Use markings that conform to the shapes and sizes outlined in the Manual on Uniform Traffic Control Devices for Streets and Highways.

2. Pigmentation
   Use white or yellow pigmented plastic according to each marking type.

3. Adhesion
   Use markings that can be affixed to bituminous or Portland cement concrete pavements by pressure-sensitive precoated adhesive or a liquid contact cement.
   Ensure that marking adhesive adheres to the roadway under normal climactic and traffic conditions.

4. Conformability
   Use markings that will mold to pavement contours, breaks, faults, and the like, by normal action of traffic at normal pavement temperatures.

5. Resealability
   Use markings containing resealing characteristics that allow the material to fuse to itself or to similar previously applied material under normal use.

6. Glass or Ceramic Beads
   Use markings with a layer of glass or ceramic beads bonded to the surface according to the marking type. Type PB contains ceramic beads and glass beads. Types TR, TN, and PA contain only glass beads.
   Use glass beads with less than 2% by weight showing any milkiness, scoring or scratching. Use clear, transparent beads that are free from air inclusions and conform to the following:

<table>
<thead>
<tr>
<th></th>
<th>Glass Beads</th>
<th>Ceramic Beads</th>
</tr>
</thead>
<tbody>
<tr>
<td>Refractive Index, (tested by oil immersion)</td>
<td>1.50 minimum</td>
<td>1.70 minimum</td>
</tr>
<tr>
<td>Uniform Distribution of Spheres</td>
<td>0.75 minimum</td>
<td>0.75 minimum</td>
</tr>
</tbody>
</table>

7. Reflective Intensity
   Ensure that marking types TR, TN, and PA use white or yellow film with the initial reflective intensity indicated in the table below, when measured at the angles shown. See Subsection 657.2.C.2.k for reflective intensity of Type PB.

<table>
<thead>
<tr>
<th></th>
<th>White</th>
<th>Yellow</th>
</tr>
</thead>
<tbody>
<tr>
<td>Divergence Angle</td>
<td>0.2°</td>
<td>0.5°</td>
</tr>
<tr>
<td>Incidence Angle</td>
<td>86°</td>
<td>86°</td>
</tr>
<tr>
<td>Reflective Intensity --candle power per foot-candle per square foot (Candelas per Lux per square meter)</td>
<td>1.00</td>
<td>0.75</td>
</tr>
</tbody>
</table>

8. Composition
   Use markings made of high-quality polymeric materials and pigments. Ensure types TR, PA, and PB contain the following composition of materials:
B. Requirements for Temporary Markings (Types TR and TN)

1. Temporary Removable Markings (Type TR)

Use temporary, removable markings that meet the following requirements:

a. Removability

   Ensure the marking material can be removed from asphaltic and Portland cement as follows:

   • Lifted intact or in large pieces.
   • Lifted either manually or with a roll-up device.
   • Lifted at temperatures above 40 °F (5 °C) without using heat, solvents, sand blasting, or grinding.

   Ensure the pavement shows no objectionable staining or damage after removing the marking.

b. Elongation and Tensile Strength

   Provide temporary markings with the following elongation and tensile strength when tested according to ASTM D 638:

<table>
<thead>
<tr>
<th>Property</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Elongation</td>
<td>0.75 minimum</td>
</tr>
<tr>
<td>Tensile Strength</td>
<td>40 lbs/in² (275 kPa) minimum</td>
</tr>
</tbody>
</table>

   Test as follows:

   1) Cut a 1 in by 6 in (25 mm by 150 mm) specimen.
   2) Test at a temperature between 70 °F and 80 °F (21 °C and 27 °C).
   3) Test at a jaw speed of 12 in/min (300 m/min).

c. Adhesion

   Ensure that at least 10 lbs (20 N) of force is required to lift stuck-on marking material from the pavement.

d. Glass Bead Retention

   Confirm the glass bead retention quality of marking material in both of the following ways:

   1) Laboratory Test

      • Take a 2 in by 6 in (50 mm by 150 mm) sample.
      • Bend the sample over a ½ in (13 mm) diameter mandrel, leaving the 2 in (50 m) side perpendicular to the mandrel axis.
      • Ensure that the area on the mandrel shows no more than 10 percent of the beads entrapped by the binder less than 40 percent.

   2) Field test

      Ensure the beads cannot be easily removed by scratching the material firmly with the thumbnail.

e. Skid Resistance

   Ensure that the material surface provides a 35 BPN minimum skid resistance value when tested according to ASTM E 303.

f. Thickness

   Ensure that the removable marking material is at least 20 mils (0.50 mm) thick not including the backing adhesive.

2. Temporary Non-Removable Markings (Type TN)

This type of pavement marking may use a conformable metallic foil backing with a precoated pressure-sensitive adhesive.

a. Abrasion Resistance

---

Material | Min% By Weight
---------|-----------------|
Resins and Plasticizers | 20
Pigments | 30
Graded Glass Beads | 33
Use marking material that does not wear through to the backing surface in less than 125 cycles. Test according to Federal Test Standard 141, Method 6192, using an H-22 wheel and a 250 gram load.

b. Skid Resistance

Ensure the retroreflective pliant polymer surface provides a skid resistance value of at least 35 BPN. Test according to ASTM E 303.

c. Elongation and Tensile Strength

No test for elongation and tensile strength is required for type TN marking.

d. Glass Bead Retention

Refer to Subsection 657.2.B.1.d, “Glass Bead Retention” for types TR and TN.

e. Thickness

Ensure the nonremovable marking material is at least 20 mils (0.50 mm) not including the adhesive backing.

C. Requirements for Permanent Markings (Types PA, PB and PW)

1. Permanent Plastic Marking (Type PA)

Provide permanent plastic markings with these features:

a. Adhesive and Backing

Use markings supplied with the following:

- A precoated adhesive
- An easily removable backing to protect the adhesive
- An adhesive backing that allows repositioning of the marking on the surface before permanently sticking with greater pressure

In addition, supply rolls of lane lines with a precoated adhesive but without the protective backing material.

b. Pigments

1) White

Use white marking material with at least 20 percent of the total pigment consisting of titanium dioxide that meets Federal Specification TT-P442 for a dense opaque marking.

2) Yellow

Use yellow marking material with sufficient yellow pigment for a durable finished color.

In addition, match the yellow to the Highway Yellow Color Tolerance Chart and Chip 33538 of Federal Standard 595.

3) Appearance

Ensure that each marking meets the following appearance standards:

- Markings are extruded to a uniform thickness.
- Edges are smoothly cut and true.
- Glass spheres are retained on all sides by the plastic base material.
- The wearing surface is free of indentations, displaced spheres, or other irregularities that retain dirt, dust, or other foreign materials.

c. Thickness

Ensure the permanent material is at least 60 mils (1.52 mm) thick, without the pre-coated adhesive.

d. Glass Bead Retention

Confirm that the surface glass beads are strongly bonded and are not easily removed by traffic. Test them as follows:

1) Use a Taber Abraser with an H-18 wheel and 125 gram load.
2) Inspect the sample at 200 cycles under the microscope to observe the extent and type of bead failure.
3) Ensure that no more than 15 percent of the beads have popped-out.
4) Verify that the predominant mode of failure is “wear-down” of the beads.
e. Reseal Test

Test the plastic to confirm that it reseals to itself. Test as follows:
1) Cut two samples, 1 in by 3 in (25 mm by 75 mm) each, keeping the adhesive backing material in place.
2) Overlap these pieces face-to-face on a flat steel plate. The overlap area should be 1 in² (625 mm²).
3) Center a 1000 gram weight over the overlap area.
4) Place the sample in an oven for 2 hours at 190 °F ± 10 °F (88 °C ± 5 °C).
5) Cool the sample to room temperature.
6) Ensure the sample pieces cannot be separated except by tearing. Reject material that separates without tearing.

f. Tensile Strength and Elongation

Ensure that the permanent markings have the following elongation and tensile strength when tested according to ASTM D 638:

<table>
<thead>
<tr>
<th>Elongation</th>
<th>75% minimum</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile Strength</td>
<td>150 psi (1035 kPa) minimum</td>
</tr>
</tbody>
</table>

Test as follows:

**NOTE: Run this test 3 times and base the result on an average of the 3 tests.**

1) Cut 3 specimens, 1 in by 6 in (25 mm by 150 mm) each.
2) Place 1 in² (625 mm²) of carborundum extra-coarse emery cloth or its equivalent at each end of the test specimens to prevent the adhesive from sticking to test equipment.
3) Test at a temperature between 70 ° and 80 °F (21° and 27 °C).
4) Test at a jaw speed of 10 to 12 in/min (250 mm to 300 mm/min).

4. Skid Resistance

Test the plastic surface to verify that it provides a skid resistance value of at least 45 BPN. Test according to ASTM E 303.

h. Abrasion Resistance

Ensure that plastic loses no more than 0.25 grams of weight in 500 revolutions when abraded according to Federal Test Method Standard No. 141 (Method 6192).

Test the material with calibrade H-18 wheels with a 1000 gram load on each wheel.

i. Adhesive Shear Strength

Ensure that the load required to break the adhesive bond is strong enough to resist a load at least 10 lbs (4.54 kg).

Test as follows:

**NOTE: Run this test 3 times and base the result on an average of the 3 tests**

1) Cut 3 specimens, 1 in by 6 in (25 mm by 150 mm) each.
2) Apply a 1 in by 3 in (25 mm by 75 mm) piece of carborundum extra coarse emery cloth or its equivalent to the adhesive face of each test strip. Overlap the area by 1 in² (625 mm²).
3) Apply 60 psi (415 kPa) of pressure over the overlapped area for 120 seconds.
   Apply the load by gripping the ends of each laminated piece in a tensile test machine, such as a Dillon or Scott tester.
4) Run the test at 77 °F (25 °C).
5) Run the test at 0.25 in/min (64 mm/min).

2. Permanent Patterned Plastic Marking (Type PB)

Use patterned plastic markings with these features:
a. Patterned Surface
Ensure that the patterned surface has the following characteristics:
- A reflective layer of ceramic beads bonded to a durable polyurethane topcoat.
- The raised area comprises between 35 and 65 percent of the total marking face.
- The surface presents a near vertical face to traffic from any direction.
- The Office of Materials and Research approves the pattern configuration.
- The channels between raised areas are free of exposed beads or particles.

b. Adhesive and Backing
Refer to Subsection 657.2.C.1.a, “Adhesive and Backing” for Type PA.

c. Pigments
Refer to Subsection 657.2.C.1.b, “Pigments” for Type PA.

d. Ceramic Beads
Ensure that the top layer of ceramic beads is bonded to a durable polyurethane surface.

e. Ceramic Bead Retention
Refer to Subsection 657.2.C.1.d, “Glass Bead Retention” for Type PA.

f. Thickness
Ensure the materials are at least 60 mils (1.52 mm) thick, not including the pre-coated adhesive backing.

g. Reseal Test
Refer to Subsection 657.2.C.1.e, “Reseal Test” for Type PA.

h. Tensile Strength and Elongation
Refer to Subsection 657.2.C.1.f, “Tensile Strength and Elongation” for Type PA.

i. Skid Resistance
Refer to Subsection 657.2.C.1.g, “Skid Resistance” for Type PA.

j. Abrasion Resistance
Refer to Subsection 657.2.C.1.h, “Abrasion Resistance” for Type PA.

k. Reflective Intensity
Determine reflective intensity using photometric testing procedures of Federal Specification L-S-300 A, Paragraph 4.4.7. Reflective values are as follows:

<table>
<thead>
<tr>
<th></th>
<th>White</th>
<th></th>
<th>Yellow</th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Observation Angle</td>
<td>0.2°</td>
<td>1.0°</td>
<td>1.05°</td>
<td>0.2°</td>
</tr>
<tr>
<td>Entrance Angle</td>
<td>86°</td>
<td>86.5°</td>
<td>88.8°</td>
<td>86°</td>
</tr>
<tr>
<td>Reflective Intensity</td>
<td>1.10</td>
<td>0.70</td>
<td>0.50</td>
<td>0.80</td>
</tr>
</tbody>
</table>

3. Wet Reflective Preformed Pavement Markings (Type PW)

a. Reflective Intensity
Determine reflective intensity using photometric testing procedures of ASTM D 4061 under dry conditions and ASTM E 1710 under wet conditions.

Create the wet test condition by pouring clean water from a bucket of approximately 3 gallon (11 Liter) capacity from a height of approximately 20 in (500 mm) above the surface. Pour the water evenly along the test surface so that a crest of water momentarily floods the measuring field and its surrounding area.

Ensure that markings use white or yellow film with the initial reflective intensity indicated in the table below, when measured at the angles shown.

<table>
<thead>
<tr>
<th></th>
<th>White</th>
<th></th>
<th>Yellow</th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Divergence Angle</td>
<td>0.2°</td>
<td>1.0°</td>
<td>1.05°</td>
<td>0.2°</td>
</tr>
</tbody>
</table>
Section 657—Preformed Plastic Pavement Markings

<table>
<thead>
<tr>
<th>Incidence Angle</th>
<th>86.0°</th>
<th>86.5°</th>
<th>88.8°</th>
<th>86.0°</th>
<th>86.5°</th>
<th>88.8°</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reflective Intensity --candle power per foot-candle per square foot (Candelas per Lux per square meter)</td>
<td>1.50</td>
<td>1.00</td>
<td>0.75</td>
<td>1.10</td>
<td>0.70</td>
<td>0.45</td>
</tr>
</tbody>
</table>

b. Composition
Use markings made of high-quality polymeric materials, pigments and reflective glass beads enclosed in a polymeric layer.

c. Adhesive and Backing
Use markings supplied with the following:
- A precoated adhesive.
- An easily removable backing to protect the adhesive.
- An adhesive backing that allows repositioning of the marking on the surface before permanently sticking with greater pressure.
In addition, supply rolls of lane lines with a precoated adhesive but without the protective backing material.

d. Appearance
Ensure that each marking meets the following appearance standards:
- Markings are extruded to a uniform thickness.
- Edges are smoothly cut and true.
- Glass spheres are retained on all sides by the plastic base material.
- The wearing surface is free of indentations or other irregularities that retain dirt, dust, or other foreign materials.
- The color conforms to standard white or yellow highway colors.

e. Thickness
Ensure the permanent material is at least 30 mils (0.76 mm) thick, without the pre-coated adhesive.

f. Elongation and Tensile Strength
Provide temporary markings with the following elongation and tensile strength when tested according to ASTM D 638:

<table>
<thead>
<tr>
<th>Elongation</th>
<th>0.75 minimum</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile Strength</td>
<td>150 psi (1035 kPa) minimum</td>
</tr>
</tbody>
</table>

Test as follows:

**NOTE: Run this test 3 times and base the result on an average of the 3 tests.**

1). Cut 3 specimens, 1 in by 6 in (25 mm by 150 mm) each.
2). Place 1 in² (625 mm²) of carborundum extra-coarse emery cloth or its equivalent at each end of the test specimens to prevent the adhesive from sticking to test equipment.
3). Test at a temperature between 70 °F and 80 °F (21 °C and 27 °C).
4). Test at a jaw speed of 10 to 12 in/min (250 mm to 300 mm/min).

-g. Skid Resistance
Test the plastic surface to verify that it provides a skid resistance value of at least 50 BPN. Test according to ASTM E 303.

h. Plastic Pull Test
1). Cut a test specimen to 1 in by 6 in (25 mm by 150 mm).
2). Ensure that the specimen can support a dead load weight of 6 lbs (27 N) for at least 30 minutes.
3). Test at a temperature between 70 °F and 80 °F (21 °C and 27 °C)
Delete Subsection 657.4 and Substitute with the following:

**657.4 Measurement**

No measurement to be included for this item. Removal of existing pavement markings will not be paid for separately, but will be included in the payment for other Work under this Section.

**657.4.01 Limits**

General Provisions 101 through 150.

Delete Subsection 657.5 and Substitute with the following:

**657.5 Payment**

Payment will be made under CONSTRUCTION COMPLETE. Includes applying markings, including adhesives, cleaning, application, and traffic control necessary to complete the Item.

**657.5.01 Adjustments**

General Provisions 101 through 150.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043

Cherokee County

Section 658—Polyurea Traffic Stripe

658.1 General Description
This work includes furnishing and applying reflectorized traffic line paint according to the Plans and these Specifications.

This Item also includes applying words and symbols according to Plan details, Specifications, and the current Manual on Uniform Traffic Control Devices.

658.1.01 Definitions
Painted Stripes: Solid or broken (skip) lines. The location and color are designated on the Plans.

Skip Traffic Stripes: Painted segments between unpainted gaps on a designated sequence with a ratio of 1:3 [10 ft (3 m) segment and 30 ft (9 m) gap] as specified on the Plans. The location and color are designated on the Plans.

658.1.02 Related References
A. Standard Specifications
   General Provisions 101 through 150.
   Section 656—Removal of Pavement Markings
   Section 870—Paint

B. Referenced Documents
   QPL 46
   AASHTO M 247
   ACI Method 503
   ASTM C 4060
   ASTM D 711
   ASTM D 1155
   ASTM D 1213
   ASTM D 4061
658.2 Materials

A. Polyurea Composition Requirements

- Use a polyurea composition that is specifically formulated for use as a durable pavement marking material and for application at elevated temperatures not exceeding 170 °F (77 °C).
- Ensure the liquid markings consist of a two-component (Part A and Part B), 100% solids polyurea film formulated and designed to provide a simple volumetric mixing ratio as recommended by the manufacturer.
- Use white or yellow films for the markings. Ensure that these films are manufactured without the use of lead chromate pigments or other similar, lead-containing chemicals.
- Ensure that the white polyurea contains not less than 13% by weight rutile titanium dioxide pigment to insure adequate opacity, hiding power, and reflective properties.

B. Glass Beads and Ceramic Reflective Elements

Use glass beads and/or ceramic reflective elements for the reflective media system that ensures the polyurea pavement markings meet the reflectance performance requirements in Section 658.2.C.2.

C. Finished Product Requirements:

1. Composition

   Ensure that the retroreflective pavement markings consist of a mixture of high-quality resins, curing agent and pigments, with a reflective layer bonded to the top surface consisting of glass beads.

2. Reflectance

   When applied according to the manufacturer’s recommendations, ensure that the white and yellow markings have the average initial and 12 months retroreflectance values shown in the tables below, as measured in accordance with the testing procedures of ASTM D4061 or ASTM E 1710.

   An observation angle of 1.05° and an entrance angle of 88.8° corresponds to 30 meter geometry. The photometric quantity to be measured is the coefficient of retroreflected luminance (R_L) and is expressed as millicandelas per square foot per foot-candle [(mcd•ft⁻²)•fc⁻¹]. The metric equivalent is expressed as millicandelas per square meter per lux [(mcd•m⁻²)•lx⁻¹].

   Determine the initial and 12 months retroreflectance of a single installation according to the measurement and sampling procedures outlined in ASTM D 6359, using a 30-meter retroreflectometer.

<table>
<thead>
<tr>
<th>Entrance Angle</th>
<th>White</th>
<th>Yellow</th>
</tr>
</thead>
<tbody>
<tr>
<td>Observation Angle</td>
<td>88.8°</td>
<td>88.8°</td>
</tr>
<tr>
<td>Retroreflective Luminance</td>
<td>1.05°</td>
<td>1.05°</td>
</tr>
<tr>
<td>R_L ([mcd•ft²)•fc⁻¹]</td>
<td>600</td>
<td>400</td>
</tr>
</tbody>
</table>

Measure initial performance of pavement markings within 7 days after application.

<table>
<thead>
<tr>
<th>Entrance Angle</th>
<th>White</th>
<th>Yellow</th>
</tr>
</thead>
<tbody>
<tr>
<td>Observation Angle</td>
<td>88.8°</td>
<td>88.8°</td>
</tr>
<tr>
<td>Average Minimum Reflectance at 12 Months</td>
<td>1.05°</td>
<td>1.05°</td>
</tr>
</tbody>
</table>
3. Color
Meet these color requirements:
- White markings are pure white and free from dirt or tint.
- Yellow markings are “Federal Yellow” in color.
- The material does not change its color and brightness characteristics after prolonged exposure to sunlight.

4. Skid Resistance
Ensure the surface of the retroreflective marking provides an initial average skid resistance value of 45 BPN when tested according to ASTM E303.

5. Color and Weathering Resistance
Ensure that the mixed polyurea compound, both white and yellow, when applied to 3 in (75 mm) x 6in (150 mm) aluminum panels at 15 ± 1 mils (0.381 mm ± 0.025 mm) wet thickness without glass beads and exposed in a Q.U.V. Environmental Testing Chamber, as described in ASTM G-53-77, conforms to the following minimum requirements:
- The color of the white polyurea compound is not darker than Federal Standard No. 595A-17778.
- The color of the yellow polyurea compound meets the requirements of the “Federal Yellow” color chart.

6. Drying Time (Laboratory)
When tested in accordance with ASTM D-711 the polyurea marking material shall reach a no-pick-up condition in 10 minutes or less. Perform this test with ASHTO M247 Type 1 beads applied at a rate of 0.099 pounds per square foot (0.483 kg/m²). Ensure that the drying time does not increase substantially with decreasing temperature.

7. Drying Time (Field)
When installed at 77 ºF (25 ºC), at a wet film thickness of 20 ± 2 mils (0.508 mm ± 0.051 mm) and reflectorized with glass beads and/or ceramic reflective elements, ensure that the polyurea markings reaches a no-track condition in less than 10 minutes. Dry to “no-tracking” will be considered as the condition where no visual deposition of the polyurea marking to the pavement surface is observed when viewed from a distance of 50 feet (15 m), after a traveling vehicle’s tires have passed over the marking.

8. Abrasion Resistance
Ensure that the wear index of the polyurea compound does not exceed 0.00026 lbs (120 mg) when tested in accordance with ASTM C4060 using a CS-17 wheel and under a load of 2.2 lbs (1000 g) for 1000 cycles.

9. Adhesion to Concrete
Ensure that the polyurea pavement marking materials, when tested according to ACI Method 503, have such a high degree of adhesion to the specified concrete surface that there is a 100% concrete failure in the performance of this test. Condition the prepared specimens at room temperature 75 °C ± 2 °F (24 °C) for a minimum of 24 hours and maximum of 72 hours prior to the performance of this test.

10. Adhesion to Asphalt
Ensure that the polyurea pavement marking materials, when tested according to ACI Method 503, have such a high degree of adhesion to the specified asphalt surface that there is a 100% asphalt failure in the performance of this test. Condition the prepared specimens at room temperature 75 °C ± 2 °F (24 °C) for a minimum of 24 hours and maximum of 72 hours prior to the performance of this test.

658.3 Construction Requirements
658.3.01 Equipment
A. Traveling Traffic Stripe Painter

To apply the traffic marking material, use a mobile, truck mounted and self contained pavement marking machine, specifically designed to apply two-component liquid materials, and glass beads, in a continuous and skip-line pattern.

Apply the two-component liquid materials through airless impingement mixing guns. The guns must accommodate a plural component material system at the manufacturer’s recommended volumetric mixing ratio. The guns must have the capacity to deliver materials from approximately 1.5 (5.7 L) to 3 gal (11.4 L) per minute to compensate for a typical range of application speeds of 3 mph (5 km/h) to 6 mph (10 km/h). Ensure that the machine travels at a uniform rate of speed both uphill and downhill.

Select the necessary accessories such as spray tip, mix chamber or static tube, and rod diameter to ensure proper mixing.

Ensure that the machine meets the following:

- The machine is capable of applying three separate stripes, either solid or skip, in any specified pattern by utilizing three adjacent spray nozzles at the same time.
- Each nozzle is equipped with satisfactory cutoff valves that will apply skip lines automatically.
- The application equipment is maneuverable to the extent that straight lines can be followed and normal curves can be made in a true arc.
- The truck-mounted unit is provided with accessories to allow for the marking of symbols and legends.

Ensure that the mobile applicator also includes the following features:

- The mobile applicator provides individual material reservoirs for the storage of Part A and Part B of the resin composition.
- The applicator is equipped with heating equipment of sufficient capacity to maintain the individual resin components at the manufacturer’s recommended temperature for spray application.
- The applicator is equipped with separate temperature controls for each component.
- The applicator is equipped with glass bead dispensing equipment and capable of applying the glass beads at a uniform rate.
- The application equipment is equipped with metering devices or pressure gauges on the proportioning pumps as well as stroke counters to monitor volumetric usage. Ensure that the metering devices or pressure gauges and stroke counters are visible.
- The applicator is equipped with all the necessary spray equipment, mixers, compressors, and other appurtenances to allow for the placement of reflectorized pavement markings in a simultaneous sequence of operations.

B. Cleaning Equipment

Use brushes, brooms, scrapers, grinders, high-pressure water jets, or air blasters to remove dirt, dust, grease, oil, and other foreign matter from painting surfaces without damaging the underlying pavement.

658.3.02 Preparation

Before painting, thoroughly clean pavement surfaces of dust, dirt, grease, oil, and all other foreign matter from painting surfaces without damaging the underlying pavement.

Remove concrete curing compounds on new Portland cement concrete surfaces and existing pavement markings on both concrete and asphalt surfaces.

658.3.03 Construction

A. Atmospheric Conditions

Apply pavement markings only during conditions of dry weather and subsequently dry pavement surfaces. Ensure that the pavement surface temperature and the ambient temperature at the time of installation are both greater than 40 °F (4 °C) and that the relative humidity is not greater than 85%.

B. Alignment

Ensure that the traffic stripe is the specified length, width, and placement. On sections where no previously applied markings are present, ensure accurate stripe location by establishing control points at spaced intervals. The Engineer will approve control points.
C. Application

Apply the pavement markings as follows:

1. Apply the liquid marking material by spray method and according to the manufacturer’s installation instructions.

2. Ensure marking configurations are in accordance with the “Manual on Uniform Traffic Control Devices.”

3. Place the reflectorized pavement markings only on properly prepared surfaces and at the widths and patterns designated on the Plans. Do not begin marking operations until applicable surface preparation work is completed and approved by the Engineer.

4. Air-blast the surface first, to remove any dirt and residues from the pavement. Then apply the pavement markings as a continuous operation.

5. Heat Component A and Component B to the manufacturer’s recommended temperatures.

6. Ensure that mixing of the two components occurs in a static tube or impingement chamber prior to reaching the application spray nozzle.

7. Spray the mixed resin onto the pavement surface at a minimum uniform wet thickness of 20 mils (0.51 mm).

8. Immediately following application, drop the glass beads and/or ceramic reflective elements onto the liquid marking at the application rates recommended by the binder manufacture.

9. The work will be subject to application rate checks for both paint and beads.

Following an application of glass beads, and upon curing, ensure that the resulting marking is an adherent reflectorized stripe of the specified thickness and width that is capable of resisting deformation by traffic.

D. Protective Measures

Protect newly applied paint as follows:

1. Traffic
   Control and protect traffic with warning and directional signs during painting. Set up warning signs before beginning each operation and place signs well ahead of the painting equipment. When necessary, use a pilot car to protect both the traffic and the painting operation.

2. Fresh Paint
   Protect the freshly painted stripe using cones or other satisfactory devices. Repair stripe damage or pavement smudges caused by traffic according to Subsection 658.3.06.

E. Appearance and Tolerance of Variance

Continually deviating from stated dimensions is cause for stopping the work and removing the nonconforming stripe. (See Section 656.) Adhere to the following measurements:

1. Width
   Do not lay stripe less than the specified width. Do not lay stripe more than 1/2 in (13 mm) over the specified width.

2. Length
   Ensure that the 10 ft (3 m) painted skip stripe and the 30 ft (10 m) gap between painted segments vary no more than ± 1 ft (300 mm) each.

3. Alignment
   a. Ensure that the stripe does not deviate from the intended alignment by more than 1 in (25 m) on tangents or curves of 1 degree or less.
   b. Ensure that the stripe does not deviate by more than 2 in (50 mm) on curves exceeding 1 degree.

658.3.04 Quality Acceptance

Ensure that stripes and segments of stripes are clean-cut and uniform. Markings that do not appear uniform or satisfactory, either during the day or night, or do not meet Specifications or become marred or damaged by traffic or from other causes, will be corrected at the Contractor’s expense.
Sections of painted stripe, words, and symbols placed according to the Plans and Specifications and have dried so that paint will not be picked up or marred by vehicle tires will be accepted. The Contractor will be relieved of responsibility for maintenance on accepted sections.

A. Correction of Alignment

When correcting a deviation that exceeds the permissible tolerance in alignment, do the following:

1. Remove the affected portion of stripe, plus an additional 25 ft (8 m) in each direction.
2. Paint a new stripe according to these Specifications.

Remove the stripe according to Section 656.

B. Removal of Excess Paint

Remove misted, dripped, or spattered paint to the Engineer’s satisfaction. Do not damage the underlying pavement during removal.

Refer to the applicable portions of Section 656.

658.4 Measurement

When traffic stripe is paid for by the square yard (meter), the number of square yards (meters) painted is measured and the space between stripes is included in the overall measurement.

Linear measurements are made on the painted surface by an electronic measuring device attached to a vehicle. On curves, chord measurements, not exceeding 100 linear feet (30 linear meters), are used.

Traffic stripe and markings, complete in place, are measured and accepted for payment as follows:

A. Solid Traffic Stripe

Solid traffic stripe is measured by the linear foot (meter), linear mile (kilometer), or square yard (meter). Breaks or omissions in solid lines or stripes at street or road intersections are not measured.

B. Skip Traffic Stripe

Skip traffic stripe is measured by the gross linear foot (meter) or gross linear mile (kilometer). Unpainted spaces between the stripes are included in the overall measurements if the Plan ratio of 1 to 3 remains uninterrupted. Measurement begins and ends on a stripe.

C. Pavement Markings

Pavement markings, words and symbols completed according to Plan dimensions are measured by the unit.

658.5 Payment

Payment will be full compensation for the work under this Section, including the following:

- Cleaning and preparing surfaces
- Furnishing materials, including paints, beads, and thinners
- Applying, curing, and protecting paints
- Protecting traffic, including providing and placing necessary warning signs
- Furnishing tools, machines, and other equipment necessary to complete the Item

Payment will be made under: CONSTRUCTION COMPLETE

Office of Materials & Research
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  

Section 700—Grassing

Delete Section 700 and substitute the following:

700.1 General Description
This work includes preparing the ground, furnishing, planting, seeding, fertilizing, sodding, and mulching disturbed areas within the Right-of-Way limits and easement areas adjacent to the right-of-way as shown on the Plans except as designated by the Engineer to remain natural.

700.1.01 Definitions
General Provisions 101 through 150.

700.1.02 Related References
A. Standard Specifications
   Section 160—Reclamation of Material Pits and Waste Areas
   Section 163—Miscellaneous Erosion Control Items
   Section 718—Wood Fiber
   Section 822—Emulsified Asphalt
   Section 882—Lime
   Section 890—Seed and Sod
   Section 891—Fertilizers
   Section 893—Miscellaneous Planting Materials
   Section 895—Polyacrylamide

B. Referenced Documents
   QPL 33
   QPL 84

700.1.03 Submittals
Submit manufacturer’s product expiration date along with written instructions to ensure proper application, safety, storage, and handling of Polyacrylamide products used in The Work.
700.2 Materials
Use materials that meet the requirements of the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Wood Fiber Mulch</td>
<td>718.2</td>
</tr>
<tr>
<td>Emulsified Asphalt</td>
<td>822</td>
</tr>
<tr>
<td>Agricultural Lime</td>
<td>882.2.01</td>
</tr>
<tr>
<td>Liquid Lime</td>
<td>882.2.01</td>
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<tr>
<td>Seed</td>
<td>890.2.01</td>
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<tr>
<td>Sod</td>
<td>890.2.02</td>
</tr>
<tr>
<td>Fertilizer</td>
<td>891.2.01</td>
</tr>
<tr>
<td>Plant Topsoil</td>
<td>893.2.01</td>
</tr>
<tr>
<td>Mulch</td>
<td>893.2.02</td>
</tr>
<tr>
<td>Inoculants</td>
<td>893.2.04</td>
</tr>
<tr>
<td>Tackifiers</td>
<td>QPL 33</td>
</tr>
<tr>
<td>Anionic Polyacrylamide</td>
<td>QPL 84 &amp; Section 895</td>
</tr>
</tbody>
</table>

A. Seeds
Whenever seeds are specified by their common names, use the strains indicated by their botanical names.

B. Water
Obtain the water for grassing from an approved source. Use water free of harmful chemicals, acids, alkalis, and other substances that may harm plant growth or emit odors. Do not use salt or brackish water.

C. Asphalt
Secure the mulch with asphalt made of a homogenous emulsification of a refined petroleum. Ensure that the asphalt can be sprayed on with or without diluting with water.

Use suitable asphalt free of petroleum solvents or other diluting agents that may harm plant growth. Use asphalt according to Section 822 or Section 824, “slow setting”. Do not use asphalt that separates after freezing or from any other cause.

D. Fertilizer Mixed Grade
Select fertilizer mixed grade such as 10-10-10, 6-12-12, 5-10-15, or other analysis within the following limits:

- Nitrogen 5 to 10 percent
- Phosphorus 10 to 15 percent
- Potassium 10 to 15 percent

If using mixed grade fertilizer for hydroseeding, ensure that it has the following analysis:

- Nitrogen 5 to 19 percent
- Phosphorus 10 to 19 percent
- Potassium 10 to 19 percent

E. Mulch
Use straw or hay mulch according to Subsection 700.3.05.G.

Use wood fiber mulch in hydroseeding according to Subsection 700.3.05.F.1.
700.2.01 Delivery, Storage, and Handling
General Provisions 101 through 150.

700.3 Construction Requirements

700.3.01 Personnel
General Provisions 101 through 150.

700.3.02 Equipment
Use grassing equipment able to produce the required results.

Never allow the grading (height of cut) to exceed the grassing equipment’s operating range.

A. Blower Equipment
   When using blower equipment to apply bituminous treated mulch in a single operation, place two or more jets or spray nozzles at or near the end of the discharge spout to eject a uniform coat of mulch.

B. Mulch Material Equipment
   Use mulching equipment that uniformly cuts the specified materials into the soil to the required control depth.

C. Rollers
   Use at least 12 in (300 mm) diameter rollers with corrugated or notched surfaces. Do not use smooth surface rollers.

D. Hydroseeding Equipment
   For hydroseeding equipment, see Subsection 700.3.05.F.

700.3.03 Preparation
General Provisions 101 through 150.

700.3.04 Fabrication
General Provisions 101 through 150.

700.3.05 Construction
Follow the planting zones, planting dates, types of seed, seed mixtures, and application rates described throughout this Section. The Engineer has the authority to alter the planting dates as set forth by a period of 2 weeks. This 2-week period may be applied to either the beginning of the specified planting and/or to the end of the end of the specified planting season.

In general:

- Obtain the Engineer’s approval before changing the ground cover type.
- Do not use annual rye grass seeds with permanent grassing.
- Follow the planting zones indicated on the Georgia State Planting Zone Map, below.
- Sod may be installed throughout the year, weather permitting.
- For permanent grassing, apply the combined amounts of all seeds for each time period within each planting zone and roadway location listed in the Seeding Table, below. Do not exceed the amounts of specified seed.
## Section 700—Grassing

### SEEDING TABLE

<table>
<thead>
<tr>
<th>PLANTING ZONES</th>
<th>PLANTING DATES</th>
<th>Rye Grass, Millet Cereal Grass (Oats)</th>
<th>Common Bermuda Grass (Hulled)</th>
<th>Common Bermuda Grass (Unhulled)</th>
<th>Tall Fescue</th>
<th>Pensacola Bahia Grass</th>
<th>Weeping Love Grass</th>
<th>Scarified Interstate Lespedeza</th>
<th>Unscarified Interstate Lespedeza</th>
<th>REQUIRED PERMANENT GRASSING</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>MARCH 1 – MAY 15</td>
<td>10 (11)</td>
<td>10 (11)</td>
<td>50 (56)</td>
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<td>COMMON BERMUDA GRASS</td>
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<td>10 (11)</td>
<td>10 (11)</td>
<td>20 (23)</td>
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</table>

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<thead>
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<th>Rye Grass, Millet Cereal Grass (Oats)</th>
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<th>Unscarified Interstate Lespedeza</th>
<th>REQUIRED PERMANENT GRASSING</th>
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<td>1,2</td>
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</tr>
</tbody>
</table>

### A. Ground Preparation

Prepare the ground by plowing under any temporary grass areas and preparing the soil as follows:

1. **Slopes 3:1 or Flatter**
   - On slopes 3:1 or flatter, plow shoulders and embankment slopes to between 4 in and 6 in (100 mm and 150 mm) deep.
   - Plow front and back slopes in cuts to no less than 6 in (150 mm) deep. After plowing, thoroughly disk the area until pulverized to the plowed depth.

2. **Slopes Steeper Than 3:1**
   - Serrate slopes steeper than 3:1 according to Plan details when required.
   - On embankment slopes and cut slopes not requiring serration (sufficient as determined by the Engineer), prepare the ground to develop an adequate seed bed using any of the following methods as directed by the Engineer:
     - Plow to a depth whatever depth is practicable.
     - Use a spiked chain.
Section 700—Grassing

- Walk with a cleated track dozer.
- Scarify.

Disking cut slopes and fill slopes is not required.

3. All Slopes
   a. Obstructions
      Remove boulders, stumps, large roots, large clods, and other objects that interfere with grassing or may slide into the ditch.
   b. Topsoil
      Spread topsoil stockpiled during grading evenly over cut and fill slopes after preparing the ground.
      Push topsoil from the top over serrated slopes. Do not operate equipment on the face of completed serrated cuts.

B. Grassing Adjacent to Existing Lawns

When grassing areas adjacent to residential or commercial lawns, the Engineer shall change the plant material to match the type of grass growing on the adjacent lawn. The Contract Unit Price will not be modified for this substitution.

If the Engineer believes bituminous treated mulch would harm other portions of the work, bituminous treated mulch may be substituted with 1,500 lbs/acre (1680 kg/ha) of wood fiber mulch with tackifier.

C. Temporary Grassing

Apply temporary grassing according to Subsection 163.3.05.F. Determine lime requirements by a laboratory soil test.

In March or April of the year following planting and as soon as the weather is suitable, replace all areas of temporary grass with permanent grass by plowing or overseeding using the no-till method. If the no-till method is used, ensure that temporary grass is less than 3 inches in height (this may be achieved by mowing). Additional mulch will be required only if the temporary grass does not provide adequate mulch to meet the requirements of Subsection 700.3.05.G, “Mulching.”

Temporary grass, when required, will be paid for according to Section 163.

Projects that consist of asphalt resurfacing with shoulder reconstruction and/or shoulder widening: Type II Wood Fiber Blanket is used to stabilize disturbed areas, no till seeding will be used when permanent grassing is applied and the areas will not be re-disturbed.

D. Applying Agricultural Lime and Fertilizer Mixed Grade

Apply and mix lime and fertilizer as follows:

1. Agricultural Lime
   Uniformly spread agricultural lime on the ground at the approximate rate determined by the laboratory soil test.
   A. Liquid Lime (Flowable Dolomitic Lime) may be applied during the hydroseeding operation at the rate of 2.5 gallons (of Liquid Lime concentrate) per acre (23.75 liters per hectare). This provides the equivalent of 1 ton per acre (2.25 mg per hectare) of agricultural lime. The remainder of lime specified by the soil test is applied as agricultural lime and uniformly spread over the surface of the ground.
   B. Agricultural Lime may be used as filler material in mixed grade fertilizer in lieu of inert material. The use of agricultural lime as filler material is to be shown on the fertilizer bag or invoice from the supplier. Do not deduct any amount of fertilizer when lime is used as filler.

2. Fertilizer Mixed Grade
   Uniformly spread the fertilizer selected according to Subsection 700.2.D over the ground at approximately 1,200 lbs/acre (1350 kg/ha).
   If using a higher analysis fertilizer with hydroseeding, apply it at the same rate per acre (hectare) as the standard fertilizer.
3. **Mixing**

Before proceeding, uniformly work the lime and fertilizer into the top 4 in (100 mm) of soil using harrows, rotary tillers, or other equipment acceptable to the Engineer.

On cut slopes steeper than 3:1, other than serrated slopes, reduce the mixing depth to the maximum practical depth as determined by the Engineer.

Omit mixing on serrated slopes.

**E. Seeding**

Following is a list of both common names and botanical names for approved seed types. Whenever seeds are specified by the common names, the strains indicated by their botanical name apply.

<table>
<thead>
<tr>
<th>Common Name</th>
<th>Botanical Name</th>
</tr>
</thead>
<tbody>
<tr>
<td>Annual Ryegrass</td>
<td>Lolium multiflorum</td>
</tr>
<tr>
<td>*Bermuda Grass, Common Hulled and Unhulled</td>
<td>Cynodon dactylon</td>
</tr>
<tr>
<td><strong>Lespedeza Virgata</strong></td>
<td>Lespedeza Ambro Virgata</td>
</tr>
<tr>
<td><strong>Lespedeza Sericea</strong></td>
<td>Lespedeza cuneta, Var. Sericea</td>
</tr>
<tr>
<td><strong>Lespedeza Seralia</strong></td>
<td>Lespedeza cuneta, Var. Seralia</td>
</tr>
<tr>
<td><strong>Lespedeza Interstate</strong></td>
<td>Lespedeza cuneta, Var. Interstate</td>
</tr>
<tr>
<td><strong>Lespedeza Korean</strong></td>
<td>Lespedeza stipulacea Maxim</td>
</tr>
<tr>
<td>Pensacola Bahiagrass</td>
<td>Paspalum notatum, var. Pensacola</td>
</tr>
<tr>
<td>Tall Fescue</td>
<td>Festuca arundinacea</td>
</tr>
<tr>
<td>Weeping Love Grass</td>
<td>Eragrostis curvula</td>
</tr>
</tbody>
</table>

*Do not use Giant Bermuda Seed (Cynodon species) including NK-37.

**Requires inoculation.

Prepare seed and sow as follows:

1. **Inoculation of Seed**

   Inoculate each kind of leguminous seed separately with the appropriate commercial culture according to the manufacturer's instructions for the culture.

   When hydroseeding, double the inoculation rate.

   Protect inoculated seed from the sun and plant it the same day it is inoculated.

2. **Sowing**

   Weather permitting, sow seed within 24 hours after preparing the seed bed and applying the fertilizer and lime.

   Sow seed uniformly at the rates specified in the Seeding Table. Use approved mechanical seed drills, rotary hand seeders, hydraulic equipment, or other equipment to uniformly apply the seed. Do not distribute by hand.

   To distribute the seeds evenly sow seed types separately, except for similarly sized and weighted seeds. They may be mixed and sown together.

3. **Rolling**

   Roll seeded areas before applying mulch, except on steep slopes where rollers cannot operate satisfactorily. On slopes inaccessible to compaction equipment, cover the seeds by dragging spiked chains over them or by using other methods.

   Do not sow during windy weather, when the prepared surface is crusted, or when the ground is frozen, wet, or otherwise non-tillable.
4. **Overseeding**

Temporary grass areas that were prepared in accordance with Subsection 700.3.05.A, may be overseeded using the no-till method. The no-till method is defined by planting permanent grass seeds using a drill-type seeder over existing temporary grass without plowing or tilling soil and in accordance with Subsection 700.3.05.C.

**F. Hydroseeding**

Hydroseeding may be used on any grassing area. Under this method, spread the seed, fertilizer, and wood fiber mulch in the form of a slurry. Seeds of all sizes may be mixed together. Apply hydroseeding as follows:

1. Use wood fiber mulch as a metering agent and seed bed regardless of which mulching method is chosen. Apply wood fiber mulch at approximately 500 lbs/acre (560 kg/ha).
2. Prepare the ground for hydroseeding as for conventional seeding in Subsection 700.3.05.A.
3. Use specially designed equipment to mix and apply the slurry uniformly over the entire seeding area.
4. Agitate the slurry mixture during application.
5. Discharge slurry within one hour after being combined in the hydrosedeer. Do not hydroseed when winds prevent an even application.
6. Closely follow the equipment manufacturer’s directions unless the Engineer modifies the application methods.
7. Mulch the entire hydroseeded area according to Subsection 700.3.05.F.1, above, and Subsection 700.3.05.G, below.

**G. Mulching**

Except as noted in Subsection 700.3.05.B and Subsection 700.3.05.C, apply mulch immediately after seeding areas as follows:

Areas with permanent grass seed and covered with slope mats or blankets will not require mulch.

Evenly apply straw or hay mulch between 3/4 in and 1-1/2 in (20 mm and 40 mm) deep, according to the texture and moisture content of the mulch material.

Mulch shall allow sunlight to penetrate and air to circulate as well as shade the ground, reduce erosion, and conserve soil moisture. If the type of mulch is not specified on the Plans or in the Proposal, use any of the following as specified.

1. **Mulch with Binder**

   Apply mulch with binder regardless of whether using ground or hydroseeding equipment for seeding.
   a. Mulch uniformly applied manually or with special blower equipment designed for the purpose. When using a blower, thoroughly loosen baled material before feeding it into the machine so that it is uniformly coated with binder and broken up.
   b. After distributing the mulch initially, redistribute it to bare or inadequately covered areas in clumps dense enough to prevent new grass from emerging.
   
   Do not apply mulch on windy days.
   c. Apply enough binder to the mulch to hold it in place. Immediately replace mulch that blows away.
   
   When using a power blower to distribute the mulch, spray the binder onto the mulch as the mulch is ejected from the machine. If distributing the mulch by hand, immediately apply the binder uniformly over the mulched areas.
   
   Use one of the following binders:
   - Emulsified asphalt, SS-1h or SS-1 (Section 822): The public, adjacent property, bridges, pavements, curbs, sidewalks, and other existing structures shall be protected from discoloration by the asphalt. Correct discoloration damage at no expense to the Department.
   - Tackifier: Use a tackifier listed in the Laboratory Qualified Products Manual may be used at the manufacturer’s recommended rates.

2. **Walked-in-Mulch**

Apply walked-in-mulch on slopes ranging in steepness from 5:1 to 2:1 and treat as follows:

a. Immediately walk it into the soil with a cleated track dozer. Make dozer passes vertically up and down the slope.

b. Where walked-in-mulch is used, do not roll or cover the seeds as specified in Subsection 700.3.05.E.3.
Section 700—Grassing

H. Sod

Furnish and install sod in all areas shown on the Plans or designated by the Engineer.

1. Kinds of Sod

   Use only Common Bermudagrass (Cyndon dactylon) or one of the following Bermudagrass varieties:
   
   Tifway 419
   Tifway II
   Tift 94
   Tifton 10
   Midlawn
   Midiron
   GN-1
   Vamont

   No dwarf Bermuda types shall be used. Sod shall be nursery-grown and be accompanied with a Georgia Department of Agriculture Live Plant License Certificate or Stamp. Sod shall consist of live, dense, well-rooted material free of weeds and insects as described by the Georgia Live Plant Act.

2. Type And Size Of Sod:

   Furnish either big roll or block sod. Ensure that big roll sod is a minimum of 21 inches wide by 52 feet long. Minimum dimensions for block sod are 12 inches wide by 22 inches long. Ensure all sod consists of a uniform soil thickness of not less than 1 inch.

3. Ground Preparation

   Excavate the ground deep enough and prepare it according to Subsection 700.3.05.A to allow placing of sod. Spread soil, meeting the requirements of Subsection 893.2.01, on prepared area to a depth of 4 inches.

4. Application Of Lime And Fertilizer

   Apply lime and fertilizer according to Subsection 700.3.05.D within 24 hours prior to installing sod.

5. Weather Limitation

   Do not place sod on frozen ground or where snow may hinder establishment.

6. Install Sod

   Install Sod as follows:
   
   • Place sod by hand or by mechanical means so that joints are tightly abutted with no overlaps or gaps. Use soil to fill cracks between sod pieces, but do not smother the grass.
   • Stake sod placed in ditches or slopes steeper than 2:1 or any other areas where sod slipping can occur.
   • Use wood stakes that are at least 8 in (200 mm) in length and not more than 1 in (25 mm) wide.
   • Drive the stakes flush with the top of the sod. Use a minimum of 8 stakes per square yard (meter) to hold sod in place.
   • Once sod is placed and staked as necessary, tamp or roll it using adequate equipment to provide good contact with soil.
   • Use caution to prevent tearing or displacement of sod during this process. Leave the finished surface of sodded areas smooth and uniform.

7. Watering Sod

   After the sod has been placed and rolled or tamped, water it to promote satisfactory growth. Additional watering will be needed in the absence of rainfall and during the hot dry summer months. Water may be applied by Hydro Seeder, Water Truck or by other means approved by the Engineer.

8. Dormant Sod

   Dormant Bermuda grass sod can be installed. However, assume responsibility for all sod through establishment and until final acceptance.
9. Establishment
   Sod will be inspected by the Engineer at the end of the first spring after installation and at the time of Final
   Inspection. Replace any sod that is not live and growing. Any cost for replacing any unacceptable sod will be at the
   Contractor’s expense.

I. Application of Nitrogen
   Apply nitrogen at approximately 50 lbs/acre (56 kg/ha) when specified by the Engineer after plants have grown to 2 in
   (50 mm) high.

   One application is mandatory and must be applied before Final Acceptance.

   Apply nitrogen with mechanical hand spreaders or other approved spreaders capable of uniformly covering the grassed
   areas. Do not apply nitrogen on windy days or when the foliage is damp.

   Do not apply nitrogen between October 15 and March 15 except in Zone 4. In planting zones 3 and 4 apply an additional
   application of nitrogen.

J. Application of Polyacrylamide (PAM)
   1. Prepare soil according to project Plans and Specifications prior to applying PAM.
   2. Apply PAM according to manufacturer’s recommendations and the requirements listed herein.
   3. Apply Polyacrylamide (PAM) to all areas that receive permanent grassing.
   4. Apply PAM (powder) before grassing or PAM (emulsion) to the hydroseeding operation.
   5. Use only anionic PAM.
   6. Ensure that the application method provides uniform coverage to the target and avoids drift to non-target areas
      including waters of the state.
   7. Achieve > 80% reduction in soil loss as measured by a rainfall simulator test performed by a certified laboratory (1
      hour storm duration, 3 inches (75 mm) rainfall per hour).
   8. Ensure uniform coverage to the target area and minimize drift to non-target areas. Apply anionic PAM to all cut and
      fill slopes, permanently grassed or temporarily grassed, either prior to grassing or in conjunction with hydroseeding
      operations. Mulch will not be eliminated.
   9. Use application rates in accordance with manufacturer’s instructions.
   10. Do not exceed 200 lbs/acre/year (224 kg/ha/year).

700.3.06 Quality Acceptance
   The Engineer may require replanting of an area that shows unsatisfactory growth for any reason at any time.

   Except as otherwise specified or permitted by the Engineer, prepare replanting areas according to the Specifications as if they
   were the initial planting areas. Use a soil test or the Engineer’s guidance to determine the fertilizer type and application rate,
   then furnish and apply the fertilizer.

700.3.07 Contractor Warranty and Maintenance
A. Plant Establishment
   Before Final Acceptance, provide plant establishment of the specified vegetation as follows:
   1. Plant Establishment
      Preserve, protect, water, reseed or replant, and perform other work as necessary to keep the grassed areas in
      satisfactory condition.
   2. Watering
      Water the areas during this period as necessary to promote maximum growth.
   3. Mowing
      Mow seeded areas of medians, shoulders, and front slopes at least every 6 months. Avoid damaging desirable
      vegetation.
      In addition, mow as necessary to prevent tall grass from obstructing signs, delineation, traffic movements, sight
      distance, or otherwise becoming a hazard to motorists.
Section 700—Grassing

Do not mow lespedezas or tall fescue until after the plants have gone to seed.

B. Additional Fertilizer Mixed Grade

Apply fertilizer at approximately 600 lbs/acre (675 kg/ha) each spring after initial plant establishment. Continue annual applications until Final Acceptance. This additional fertilizer will be measured and paid for at the Contract Unit Price for fertilizer mixed grade.

C. Growth and Coverage

Provide satisfactory growth and coverage, ensuring that vegetation growth is satisfactory with no bare spots larger than 1 ft² (0.1 m²). Bare spots shall comprise no more than 1 percent of any given area. An exception is given for seed not expected to have germinated and shown growth at that time.

D. Permissible Modifications

When all Items of the work are ready for Final Acceptance except for newly planted repaired areas or other areas with insufficient grass, the Contractor may fill the eroded areas or treat bare areas with sod obtained, placed, and handled according to Subsection 700.3.05.H.

Carefully maintain the line and grade established for shoulders, front slopes, medians, and other critical areas.

Sod as described above will not be paid for separately, but will be an acceptable substitute for the satisfactory growth and coverage required under this Specification. These areas treated with sod are measured for payment under the Item for which the sod is substituted.

700.4 Measurement

No Measurement to be included for this item.

700.4.01 Limits

General Provisions 101 through 150.

700.5 Payment

Payment will be made under CONSTRUCTION COMPLETE.

A. Permanent Grassing

Includes preparing the ground, seeding, wood fiber mulch, polyacrylamide, and providing plant establishment and other incidentals.

B. Straw or Hay Mulch

Includes straw or hay mulch required for Permanent Grassing.

C. Fertilizer Mixed Grade

Includes furnishing and applying the material.

D. Lime

Includes furnishing and applying the material.

E. Nitrogen

Includes furnishing and applying the material.

F. Sod

Includes ground preparation, including addition of topsoil, furnishing and installing live sod, and for Plant Establishment.

G. Temporary Grass

Includes temporary grass.

700.5.01 Adjustments

General Provisions 101 through 150.
Delete Section 702 and substitute the following:

702.1 General Description
This Work includes furnishing and planting vines, shrubs, trees and plants, as well as treating regenerated areas according to the Specifications, Plans, and the Engineer.

702.1.01 Definitions
General Provisions 101 through 150.

702.1.02 Related References
A. Standard Specifications
   Section 108—Prosecution and Progress
   Section 700—Grassing
   Section 882—Lime
   Section 891—Fertilizers
   Section 893—Miscellaneous Planting Materials
B. Referenced Documents
   Standardized Plant Names

702.1.03 Submittals
A. Certificates of Inspection
   Submit certificates of inspection with the invoice for each shipment of plants as required by law for transportation.
   File certificates with the Engineer before the material is accepted. Plants may be rejected at the site regardless of Federal or State government inspections at the place of growth.

B. Substitutions
   When both primary and alternate plants are specified, use the alternate only after providing written proof that the primary plants specified are not available. In this case a Supplemental Agreement is not required to use the alternate plants.
When a primary or an alternate plant cannot be furnished, provide the Engineer written proof that neither is available. A Supplemental Agreement is required for substitute plants in this case.

702.2 Materials

Ensure that materials meet the requirements of the following Specifications:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Water</td>
<td>700.2.B</td>
</tr>
<tr>
<td>Agricultural Lime</td>
<td>882.01</td>
</tr>
<tr>
<td>Fertilizers</td>
<td>891.01</td>
</tr>
<tr>
<td>Plant Topsoil</td>
<td>893.01</td>
</tr>
<tr>
<td>Landscape Mulch</td>
<td>893.02</td>
</tr>
<tr>
<td>Vines, Shrubs, Trees, and Miscellaneous Plants</td>
<td>893.03</td>
</tr>
<tr>
<td>Tree Paint</td>
<td>893.06</td>
</tr>
<tr>
<td>Prepared Plant Topsoil</td>
<td>893.07</td>
</tr>
<tr>
<td>Stakes</td>
<td>893.08</td>
</tr>
<tr>
<td>Organic Soil Additives</td>
<td>893.09</td>
</tr>
</tbody>
</table>

A. Plant Specifications

Furnish plants according to the plant name and Specifications included on the Plans titled, “Plant Specifications.”

1. Plant Names

   Ensure that the botanical and common names of plants specified conform with the most current edition of Standardized Plant Names, as adopted by the American Joint Committee on Horticultural Nomenclature.

2. Grades

   Ensure that plants meet the grade requirements of the most current American Standard for Nursery Stock of the American Association of Nurserymen and any other requirements.

   Caliper used for establishing plant grades or trunk sizes is measured according to the American Standard for Nursery Stock. Plant trees with straight stems and symmetrical branches according to their natural growth. Trees with broken or damaged terminal or main stems will be rejected.

3. Substitutions

   Use approved substitute plants, as designated by the Engineer, equal in value to specified plants. Request substitutions at least 30 days before the end of the planting season in the area.

B. Nursery Plants

Unless otherwise specified, use plants stock-grown in a licensed nursery under intensive care and cultivation for at least one year. The branch system shall be normally developed and free of disease, injurious insects, disfiguring knots, sunscald, injuries, bark abrasions, dead or dry wood, broken terminal growth, or other disfigurements. Ensure that proper certificates of inspection and a complete list of the nursery growers accompany nursery grown plants. See Subsection 893.03.

C. Collected Plants

Collected plants grow in the wild and are uncultivated and untransplanted. Do not take collected plants from areas infested with insects under quarantine. See Subsection 893.03.

D. Approval and Selection of Materials and Work

Select materials and execute operations required under the Specifications and drawings with the approval of the Engineer. Remove rejected materials from the site promptly.
Section 702—Vine, Shrub, and Tree Planting

702.2.01 Delivery, Storage, and Handling

A. Bare-Rooted Plants

Tie bare-rooted plants in bundles and place moist sphagnum moss, shingletoe, or other moisture-retaining material around the roots to keep the plants moist for up to 10 days. Over-wrap the bundle with a heavy weight, waterproof, flexible material, covering the roots and one-half of the tops. Keep the plants wrapped until they are planted or heeled-in.

Wrapped plants may be held in the package for up to 10 days from shipment if protected from the sun and wind. If unable to plant plants within 10 days from shipment, unwrap, spread the roots, heel-in using moist soil, and water well.

Protect roots of plants that have been heeled-in from drying out. Cover soil and roots with wet canvas, burlap, or straw while transporting and distributing them for planting. The type of protection depends on weather conditions and the length of time the plants remain unplanted. Use protection methods satisfactory to the Engineer.

B. Balled and Burlapped Plants (B&B)

Ensure that the soil in the ball is the original and undisturbed soil in which the plant has grown.

1. Dig, burlap, transport, and handle the plant carefully to avoid loosening the soil (stripping or exposing the roots). Burlap shall be a natural biodegradable material. Do not use synthetic burlap.
2. Replace plants rejected because of broken or loose balls, or balls of less diameter than that specified.
3. Adequately protect the roots of balled and burlapped plants, unless they are planted immediately after they are delivered. Completely cover them with damp soil, sawdust, or other moist material until removing them for planting.
4. Keep plants moist while awaiting planting.
   a. Do not saturate the ball, causing it to pull off in handling.
   b. Handle B&B plants by the ball and not by the top growth.
   c. Never leave the balls of plants unprotected overnight.

C. Container-Grown Plants

Keep container-grown plants moist until planted. Handle them by the container or soil ball and not by the top growth.

D. Collected Plants

Do not collect plants more than 24 hours before planting.

1. Select plants with good shape and form. Do not select poorly shaped, weak plants taken from dense shade and crowded conditions.
2. Dig collected plants with a wide root system equal to at least the spread of the top of the plant.
3. Protect the roots with a moist packing material.
4. Load them onto a covered truck, protected from the sun and wind and transfer them directly to the final planting site.
5. Prune collected plants by removing from one-third to one-half of the side branches as directed by the Engineer.

E. Heeled-in Plants

Properly maintain heeled-in plants until they are planted. Do not allow plants to remain heeled-in over the summer or for over 30 days without the Engineer’s consent.

F. Injury Prevention

In digging, loading, unloading, planting, or otherwise handling plants, avoid injuring the trunk, branches, and roots of the plants. Injured plants will be rejected. Protect tops of shrubs and trees while in transit to prevent windburn.

702.3 Construction Requirements

702.3.01 Personnel

General Provisions 101 through 150.

702.3.02 Equipment

General Provisions 101 through 150.
Section 702—Vine, Shrub, and Tree Planting

702.3.03 Preparation
A. Inspect Plants Before Digging

The Engineer will inspect trees or plants from the bidder’s source for acceptability. When rejecting the trees or plants, the Engineer reserves the right to pursue and examine other sources of plants to find acceptable specimens. This change will not constitute an increase in cost to the State.

B. Clear and Grub

Clear and grub before planting or beginning to prepare the plant bed. See Section 201

C. Prepare Plant Bed

Prepare for planting as follows:

1. Planting Limits
   Stake planting limits according to Plan details and the Engineer. Have the Engineer approve the method of plant identification before planting.

2. Applications of Soil Additives
   a. Apply fertilizer approximately 3 lbs/100 ft² (1.5 kg/10 m²) of bed surface. Fertilizer for plant bed may be 6-12-12 if 4-12-12 is not available.
   b. Apply agricultural lime for plant bed approximately 5 lbs/100 ft² (2.5 kg/10 m²) of bed surface.
   c. Spread an organic soil additive, (See Subsection 893.2.09), evenly throughout the designated area to at least 2 in (50 mm) deep. Thoroughly dig it into the soil to at least 6 in (150 mm) deep using a rotary hoe type tiller or other equipment that evenly mixes the soil, lime, fertilizer, and organic soil additive.
   d. Till the area until the surface is smooth and free of weeds, roots, rocks, and other debris, to the satisfaction of the Engineer.

702.3.04 Fabrication

General Provisions 101 through 150.

702.3.05 Construction
A. Seasonal Limitations for Planting

For geographic seasonal limitations, refer to the Planting Zones Map found in Subsection 700.3.05. Plant in Zones 1 and 2 between October 15 and March 15. Plant in Zones 3 and 4 between November 1 and March 1.

B. Planting Operations

Plant using either the pit method or the dibble method as called for on the Plant Specification sheet. Before beginning planting of each area, have available the necessary materials including prepared plant topsoil (see Subsection 893.2.07), water, stakes, and mulch.

When seasonal limitations and weather conditions permit, continuously water, mulch, guy, and stake, until completing the last operation.

After completing planting, provide a method for retaining water adjacent to the plant according to the details shown on the Plans or as directed by the Engineer.

1. Planting By the Pit Method
   a. Placing Bare-Rooted Plants
      Plant bare-rooted plants delivered to the pit area. Protect roots from drying out until placing them in the pit.
      • Center plants in pits and spread roots as they originally grew.
      • Cover and prepare the topsoil according to details shown on the Plans.
   b. Placing Balled and Burlapped Plants
      Immediately plant these plants after they are delivered to the pit site. Never allow the balls to remain unprotected overnight.
• The pit diameter shall be a minimum of 3 times the diameter of the rootball. Center the ball in the prepared pit, leaving the top of the ball 1 in (25 mm) above the top of the ground for settlement.
• Cut away and remove the top 1/3 of burlap from the rootball. Cut all ropes and twine, pull the nails, and drop the remaining burlap to the bottom of the hole. Cut away and remove any wire from the top 1/3 of the rootball.
• Partially fill the pit with prepared plant topsoil and compact the soil enough to hold the ball firmly.

c. Placing Container-Grown Plants

When the container is delivered to the pit site, split the container from top to bottom and carefully remove the plant.
• The pit diameter shall be a minimum of 3 times the diameter of the rootball. Spread into the hole any major roots growing around the container or prune them to remove any circular growth.
• Place the ball in the center of the prepared pit, leaving the top of the ball 1 in (25 mm) above the top of the ground for settlement.
• Partially fill the pit with prepared plant topsoil and compact the soil enough to hold the ball firmly.

d. Completing Pit Plantings

After placing pit plantings, water plants thoroughly the same day regardless of weather or soil moisture conditions.
• After the water has soaked in, add prepared plant topsoil and compact firmly up to 2 in (50 mm) below the adjacent ground.
• Stop compacting when the compacted prepared topsoil is 2 in (50 mm) below the adjacent ground.
• Fill the remainder of each pit with loose, prepared plant topsoil according to the details shown on the Plans.
• Prepare the loose topsoil to retain water adjacent to the plant according to the Plans or as directed by the Engineer.

2. Planting By the Dibble Method

If the Plans require the dibble method, perform the Work as outlined. Standard dibble blades are made in 10 in (250 mm) and 12 in (300 mm) heights. Use the 12 in (300 mm) blade on all plants except those with a root system of 8 in (200 mm) or less.

Locate plants as shown on the Plans or as approved by the Engineer. Only plant when there is adequate moisture in the ground and when the ground is not frozen.

Follow these steps when grass or other vegetation is present:

a. Mow an area at least 2 ft (600 mm) on all sides of the proposed location of the individual dibbled plants to a height of 1 in (25 mm).

b. Apply landscape mulch of the specified type and amount to the mowed area before planting.

c. Dibble the seedling into the soil.

d. Dibble the plant within 48 hours after mowing.

e. Complete each planting according to the Plan details to retain water adjacent to the plant.

C. Landscape Mulching

1. For Pit Plantings

Follow these requirements when mulching for pit plantings:

a. Where the distance between plants is 8 ft (2.4 m) or less, spread mulch throughout and 3 ft (900 mm) beyond the outermost plants. Where plants are more than 8 ft (2.4 m) apart, apply mulch in a circular fashion around each plant, forming a ring 5 ft (1.5 m) in the outside diameter.

If plant pits are greater than 5 ft (1.5 m) in diameter, ensure that the mulch extends out to cover the berm as shown in the planting details on the Plans.

b. Apply mulch within 3 days of planting at least 4 in (100 mm) in depth to obtain a compacted depth of at least 3 in (75 mm).
Compaction occurs naturally. Check compaction at least two months after spreading and exposing the mulch to the elements.

If the compacted depth is less than 3 in (75 mm), apply additional mulch to deficient areas within 1 month following notification.

c. Apply mulch to a uniform depth and remove lumps for a neat appearance. Tuck mulch neatly against all paving edges, drainage structures, and where planting beds meet grassed areas.

d. Leave a 1 in (25 mm) to 2 in (50 mm) ring of non-mulched area directly around all tree trunks.

e. Do not mulch with Cypress Mulch.

2. For Plantings by the Dibble Method

Apply landscape mulch according to Subsection 702.3.05.C.1 with the following exceptions:

a. Apply mulch before planting.

b. Ensure that the minimum compacted height after 2 months exposure is 2 in (50 mm).

D. Wrapping

Do not wrap the trucks of tree unless specified in the plans. When wrapping is specified, tightly wrap the trunks of deciduous trees over 1.25 in (32 mm) in caliper. Wrap in strip burlap or waterproof crepe tree wrapping paper or other approved materials.

1. Begin wrapping at the ground and extend spirally up and beyond the first rosette of branches with an overlap of one half the width of the wrapping material.

2. Tie the wrapping material securely with binder twine spaced every 12 in (300 mm) for the full length of the wrapping. Wrap immediately after planting.

E. Staking and Guying

1. Perimeter Staking

Place perimeter stakes 2 in x 2 in x 36 in (50 mm x 50 mm x 900 mm). Stake the perimeter of indicated regenerated areas within specified planting dates according to the Plans or as directed by the Engineer.

2. Vine, Shrub, and Miscellaneous Plant Staking

Use stakes to identify isolated vines, shrubs, and miscellaneous plants outside of solid mulched beds according to Plan details.

3. Tree Staking and Guying

Stake trees with an identification stake and guy according to the details and dimensions shown on the Plans. Each guy wire shall consist of 18-gauge (1.2 mm) malleable galvanized iron wires twisted into a single strand and enclosed loosely into a rubber hose (or other approved covering or guying materials) extending around the trunk. Replace at no additional expense to the Department, any staking and guying materials that break or loosen.

**Nylon guying straps of accepted size and quality may be substituted for the hose and wire specified above.**

a. After fastening the wire to the stake by tying or twisting it into a figure-8, nail or staple the wire to the stake to prevent slippage using a 4d nail or a 0.5 in (13 mm) staple.

b. Tighten the wire so that twisting the wire causes a slight strain between the tree and the stake.

c. Place guy wires above the first rosette of lower branches and fasten wire to the stake approximately 6 in (150 mm) above the ground.

F. Pruning

1. Prune plants on the site before planting and after initial inspection by the Engineer. Never prune severely to get plants to meet Specifications.

a. Follow modern horticultural practices and use approved tools designed for pruning.

Lopping, topping, or shearing trees or shrubs will result in rejection.

b. Prune back damaged, scarred, frayed, split, and skinned branches, limbs, and roots to live wood nearest to the next sound, outside lateral bud, branch, limb, or root.

c. Leave the terminal leaders or buds in trees intact.
d. Remove approximately one-third of the smaller branches on nursery grown vines, shrubs, and trees for root-top balance.

e. Prune roots, when necessary, as directed by the Engineer.

f. Prune Crape Myrtles to maintain natural form only. Severely cutting back crape myrtles is not permitted. Remove sucker growth from Crape Myrtles.

G. Watering

1. Apply water in a manner to prevent erosion. Water plants at the time of planting. Water after applying fertilizer called for in Subsection 702.3.05.H and as necessary to maintain enough moisture to promote plant growth.

a. Apply enough water to wet the soil to a depth slightly below the roots. Direct the water to the ground around the plant, not the tops.

b. Do not allow plant foliage to dry out or plants to defoliate from lack of water. Remove plants in such condition from the site immediately.

c. Apply water once per week throughout the planting season in which the plants are installed. Follow Subsection 702.3.07.B and 702.3.07.C for shrub and tree watering requirements throughout the life of the project.

H. Spring Application of Fertilizer

1. Method and Rate of Application

Follow these requirements when applying fertilizer in the spring:

a. Trees

Deep-root feed trees each spring by using a 8-12-12 slow release fertilizer. Bore a 1.5 in (38 mm) diameter hole between 18 in to 24 in (450 mm to 600 mm) deep at the rate of 8 to 10 holes per tree.

Use 1 cup (0.25 L) of fertilizer per 1 in (25 mm) in caliper of tree measured 6 in (150 mm) off the ground. Fill the holes with soil upon completing each hole.

b. Shrubs

Fertilize shrubs with a 6-12-12 slow release 60 percent organic fertilizer by spreading fertilizer around the base of the plant and working it into the soil by hand. Use 0.5 cup (0.12 L) of fertilizer per foot (300 mm) of shrub height.

c. Bed Areas

Spread fertilizer on bed areas (defined by method of planting in Subsection 702.3.05.B), over the mulch at the rate of 3 lbs/100 ft² (1.5 kg/10 m²) using 6-12-12 or 8-12-12. Thoroughly water in the plants.

d. Vines

Fertilize vines when not planted in a bed at the rate of 1/4 cup (60 ml) per vine using 6-12-12 or 8-12-12. Thoroughly water in the plants.

e. Regenerated Areas

Spread fertilizer on regenerated areas evenly at a rate of 3 lbs/100 ft² (1.5 kg/10 m²) and thoroughly water in using 6-12-12.

NOTE: 2 cups (1 L) of 6-12-12 or 8-12-12 equals 1 lb (1 kg).

2. Time of Application

Apply fertilizer in the spring in Zones 1 and 2 (with reference to the Planting Zones specified in Subsection 702.3.05.A) between April 1 and April 15. Apply between March 15 and April 1 for Zones 3 and 4. For late plantings, do not apply fertilizer less than 30 days after the plantings.

3. Additional Fertilizer Grades 8-12-12 or 6-12-12

Approximately one month after the spring fertilizer is applied, the Engineer will inspect planted areas and determine if an additional application of fertilizer is needed for any plant or group of plants.

If the Engineer determines additional fertilizer is required, apply fertilizer at the rate specified in Subsection 702.3.05.H. Make the additional application between June 15 and July 15th.
Section 702—Vine, Shrub, and Tree Planting

I. Treatment of Regenerated Areas

Treating regenerated areas includes staking the perimeter and applying fertilizer in the spring.

Pruning, mulching, staking (except perimeter staking), guying, mowing, weeding, and watering (except watering following fertilization) are not required.

Perform perimeter staking as specified in Subsection 702.3.03.C.1. Apply fertilizer in the spring as specified in Subsection 702.3.03.C.2.

J. Restoration and Cleanup

Restore areas where existing grass has been damaged or scarred during planting operations at no expense to the Department. Restore the disturbed areas to their original conditions as directed by the Engineer. Clean up debris, spoil piles, and containers and leave the Project area clean.

702.3.06 Quality Acceptance

Preserve the plants in a healthy growing condition. The acceptability of the plant material planted and maintained as specified will be determined at the end of an establishment period.

The plant establishment period is the period from the last planting specified in Subsection 702.3.05.B until the following October 1. Plant all plants in one planting season unless otherwise approved by Engineer.

A. First Establishment Period

At the end of the first planting season, the first establishment period begins. The Department will make the first semi-final inspection 30 days before the end of the first establishment period. Replace dead, dying, diseased, unsatisfactory, and missing plants, by January 20 of the next (second) planting season.

B. Second Establishment Period

At the end of the second planting season, the second plant establishment period begins. The Department will make the second semi-final inspection 30 days before the end of the second establishment period. Again, replace dead, dying, diseased, unsatisfactory, and missing plants, by January 20 of the next (third) planting season.

C. Final Inspection

The Department will make the final inspection of the plants during May, following any needed replacements during the previous planting season. Assume responsibility for the plants until the Final Acceptance of the Project or a portion of the Project.

702.3.07 Contractor Warranty and Maintenance

Project maintenance includes, but is not limited to, watering, cultivating, weeding, pruning, repairing, adjusting guys and stakes, and performing other work as ordered by the Engineer until final acceptance.

Promptly remove from the Project area dead plants or those that no longer conform to the requirements of Subsection 702.2.A.2.

Mow the entire right-of-way within the limits of the Project up to a maximum of four times per calendar year.

A. Leaning Trees

Straighten leaning trees as directed by the Engineer. Follow Staking and Guying requirements for replacements or repairs as per Subsection 702.3.05.E.

B. Shrub Maintenance

1. Pruning

Prune or thin shrubs, as directed by the Engineer, at least two times per year, once before spring and once during mid-summer. Maintain an attractive shape and fullness with respect to the intended character of the planting. See Subsection 702.3.05.F.

2. Landscape Mulching
Continuously maintain shrub and tree beds with a clean, freshly mulched appearance using the mulch originally specified. See Subsection 702.3.05.C.

a. Apply a 2 in (50 mm) loose layer of specified mulch (top-dressing) on top of all areas, including tree pits, initially mulched, at the following times:
   - In August, during the first plant establishment period.
   - In April, during the second plant establishment period.
   - In August, during the second plant establishment period.
   - In April, prior to the final inspection.

3. Applying Fertilizer
   See Subsection 702.3.05.H.

4. Applying Pesticides
   a. Inspect all planted or seeded vegetation for insects, grubs, mites, diseases, etc., once every two weeks. Apply insecticides, fungicides, and herbicides according to the manufacturer’s recommendations to effectively control or eradicate the problem.
   b. Perform all pesticide applications under the direct supervision of a trained licensed pesticide operator. Carry the pesticide license/certification on the work site during applications. Carry all labeling associated with the chemical being applied at the work site.
   c. Submit all product information data sheets and EPA approval numbers on all pesticides proposed to be used prior to application for approval.
   d. Notify the Engineer a minimum of 48 hours prior to any and all pesticide applications.
   e. Add a blue dye to all spray applications unless approved otherwise by the Engineer.
   f. Monitor the weather and spray under proper weather conditions. Spraying shall not occur when the weather is greater than 10 miles per hour.
   g. Wear the proper safety attire. Wear long sleeve shirts, long pants, gloves, and safety glasses. Wear or use any additional protective safety attire or gear as recommended by the product’s manufacturer.
   h. Repair any damage that is a result of mishandling or misuse of materials, at no expense to the Department, to the satisfaction of the Engineer.

5. Edging
   a. Edge all shrub pits, shrub beds, and tree pits twice a month throughout the life of the project such that the vee-cut edging detail specified on the plans is maintained. Prevent grass and weeds from growing over or into the shrub beds and tree pits.
   b. Use equipment specifically designed for edging. Line trimming equipment shall not be used.

6. Watering
   a. Check all planted material once a week throughout the contract for dryness by removing the mulch from their base and “sampling the soil” approximately 4 in (100mm) deep. Water if the soil is not moist.
   b. Water all planted material if a drought (no rain for two weeks) occurs. Provide the water required to meet the watering requirements.
   c. Water each plant thoroughly until the ground is saturated to a depth slightly below the root ball. Apply water in a manner to prevent erosion.

7. Weed Control
   Perform weed control throughout the project, a minimum of once every two weeks, in all areas within the project limits to maintain tree pits, shrub beds, sidewalks, curb and gutter, walkways, ditch paving, concrete medians, and other pavement weed free. Meet the following conditions:
   a. Perform weed control to prevent weeds from becoming established, setting seed, or from becoming visible in the planting beds.
   b. Completely remove all undesirable plants (weeds) by hand pulling. Removal of weeds may be accomplished using herbicides if approved by the Engineer.
c. Apply an approved pre-emergent herbicide twice each year, once in the spring and once in the fall, throughout the contract. Apply pre-emergent to all shrub beds and tree pits. Notify the Engineer 48 hours prior to spraying. Use a blue dye in all applications unless approved otherwise by the Engineer.

d. If noted on plans, eradicate all invasive exotic pest plants found within the project limits throughout the life of the project.

e. Dispose off site on a daily basis all weed, exotic plants, clippings, litter, and debris generated.

8. Policing
   Remove debris such as paper, broken limbs, bottles, cans, etc., a minimum of the first and third week of each month from all areas within the project limits while maintaining the site.

C. Tree Maintenance

1. Watering
   See Subsection 702.3.07.B.6

2. Landscape Mulch
   See Subsection 702.3.07.B.2

3. Fertilizer
   See Subsection 702.3.05.H.

4. Abnormal Conditions
   Periodically (once every two weeks) observe trees and shrubs for abnormal conditions such as insects, borers, web worms, red spiders, etc., and immediately treat.

5. Sucker Growth
   Remove sucker growth once a month. Sucker growth is the shoots that sprout out around the base of the tree trunk.

6. Pruning and Deadwood
   Remove deadwood at least two times a year. Prune dead branches. Paint cuts, and wounds or scars with tree paint only when specified in the plans. Do not top Crape Myrtles. See Subsection 702.3.05.F.

7. Pesticide Control

   Apply pesticides as necessary to control bores, aphids, mealy bugs, mites, and tent worms, and diseases. Follow the manufacturer’s instructions. See Subsection 702.3.07.B.4. NOTE: Use chemicals according to Federal, State and county directives on environmental control that carry an EPA approval number.

8. Weed Control
   See Subsection 702.3.07.B

9. Staking and Guying
   Remove all guy wires/nylon strapping and stakes from plants which have gone through one complete growing season.

702.4 Measurement

A. Plants
   Plants of the name and size specified are measured for payment according to the number planted that are still living and in an acceptable condition at the time of Final Acceptance.

B. Fertilizer
   Spring application fertilizer applied to planted and regenerated areas will be the actual number of pounds (kilograms) placed and accepted. Fertilizer, lime, and plant topsoil used in prepared plant topsoil or plant bed preparation are not measured for separate payment.

C. Perimeter Stakes
   Perimeter stakes is not measured for payment unless such item is shown as a separate Pay Item in the Proposal.
Section 702—Vine, Shrub, and Tree Planting

D. Clearing and Grubbing

Clearing and grubbing is not measured for payment unless the Item is shown as a separate Pay Item in the Proposal.

E. Landscape Mulch

The quantity of landscape mulch and top-dressing measured for payment will be the actual number of square yards (meters) completed as specified and accepted. The presence of weeds or other growth, or foreign material, will be cause for rejection. The addition of landscape mulch in deficient areas will not be measured for payment.

702.4.01 Limits
General Provisions 101 through 150.

702.5 Payment

A. Plants

Plants - paid for under: CONSTRUCTION COMPLETE

Includes furnishing, planting, replanting as required, pruning, staking, guyng, soil conditioning, and preparing plant beds, including applying additives, digging plant pits, preparing plant topsoil and mulch, disposing of waste material, and maintaining the plants during the plant-establishment period.

B. Fertilizer – Paid for under: CONSTRUCTION COMPLETE

Includes furnishing and applying and for watering regenerated areas.

C. Perimeter Stakes – Paid for under: CONSTRUCTION COMPLETE

Perimeter stakes will not be measured for payment. The cost will be included in Construction Complete.

D. Landscape Mulch – Paid for under: CONSTRUCTION COMPLETE

Includes furnishing, installing, topdressing, and maintaining mulch as required.

702.5.01 Adjustments
General Provisions 101 through 150.
Section 814—Soil Base Materials

Delete 814.2.02 and substitute the following:

814.2.02 Soil-Cement Material

A. Requirements

1. Ensure that the material for soil-cement base will:
   a. Meet the requirements of Subsection 810.2.01 for Classes IA1, IA2, IA3, or IIB1 with the following modifications:

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Clay content</td>
<td>5 to 25%</td>
</tr>
<tr>
<td>Volume change</td>
<td>18% maximum</td>
</tr>
<tr>
<td>Liquid Limit</td>
<td>25% maximum</td>
</tr>
<tr>
<td>Plasticity Index</td>
<td>10% maximum</td>
</tr>
<tr>
<td>Maximum dry density</td>
<td></td>
</tr>
<tr>
<td>Sulfates</td>
<td>4000 ppm maximum</td>
</tr>
<tr>
<td>pH</td>
<td>4.0 minimum</td>
</tr>
</tbody>
</table>

   b. Be friable and not contain large amounts of heavy or plastic clay lumps, organic material, roots, or other substances that would interfere with how the Portland cement sets, plant production, or the finished surface of the base and meet the requirements of Subsection 301.3.05.A.1. “Pulverization” or Subsection 301.3.05.B.1. “Soil”.

   c. Produce a laboratory unconfined compressive strength of at least 450 psi (3.1 MPa). To make the sample, mix in a minimum of 5 percent to a maximum of 9 percent Type I Portland cement, moist-cure for 7 days, and test with GDT 65.

2. Analyze the soil-cement design and create a Job Mix Formula for each Project where soil-cement base or subbase is specified. Have the Job Mix Formula approved by the Engineer before starting base or subbase construction.

3. You may use fly ash or slag that meets the requirements of Subsection 831.2.03 as admixtures for poorly reacting soils when the blend of soil and fly ash, or slag, meets the design requirements in this Subsection.
4. Ensure that subgrade material used underneath the soil-cement base meets the sulfate and pH requirements of this subsection (See Subsection 209.3.05.A.7).

B. Fabrication
General Provisions 101 through 150.

C. Acceptance
Test as follows:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Soil gradation</td>
<td>GDT 4</td>
</tr>
<tr>
<td>Volume Change</td>
<td>GDT 6</td>
</tr>
<tr>
<td>Maximum density</td>
<td>GDT 7 or GDT 67</td>
</tr>
<tr>
<td>Soil-Cement Design</td>
<td>GDT 65</td>
</tr>
<tr>
<td>pH</td>
<td>GDT 98</td>
</tr>
<tr>
<td>Sulfates</td>
<td>ASTM D 516</td>
</tr>
<tr>
<td>Liquid Limit</td>
<td>AASHTO T 89</td>
</tr>
<tr>
<td>Plastic Limit and Plasticity Index</td>
<td>AASHTO T 90</td>
</tr>
</tbody>
</table>

D. Materials Warranty
General Provisions 101 through 150.

Office of Materials and Research
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Section 820—Asphalt Cement

Delete Section 820 and substitute the following:

820.1 General Description
This Section includes the requirements for asphalt cements prepared from crude petroleum.

820.1.01 Related References
A. Standard Specifications
   General Provisions 101 through 150.
B. Referenced Documents
   Standard Operating Procedure (SOP 4)
   AASHTO R 28
   AASHTO T 48
   AASHTO T 179
   AASHTO T 240
   AASHTO T 313
   AASHTO T 314
   AASHTO T 315
   AASHTO T 316
   AASHTO TP70 (proposed) / ASTM D7405

820.2 Materials
820.2.01 Asphalt Cement
A. Requirements
   1. Type
      Use a material that is homogenous and water-free and that does not foam when heated to 347 °F (175 °C). Ensure that a blend used to produce a specified performance grade meets the following requirements:
      • Is uniform and homogeneous without separation
      • Uses PG 64-22 or PG 67-22 described below for the base asphalt
      • Consists of production materials that have not been “air-blown or acid modified” to achieve the performance grade
   2. Grade
      Use the various grades of asphalt cement that meet the requirements shown in the test requirements for Petroleum Asphalt Cements
      Add only Styrene-Butadiene-Styrene (SBS) or Styrene-Butadiene (SB) to neat asphalt to produce a binder that meets requirements for PG 76-22.
For non Stone Matrix Asphalt Mixtures (SMA), SBR or crumb rubber modified PG 76-22 is an acceptable alternative to SBS or SB modified asphalt cement at contractor’s discretion, provided the SBR and crumb rubber modified asphalt cement meets the tests’ requirements of PG 76-22. For SBR modified PG 67-22 to meet PG 76-22, use only SBR currently approved on QPL-65 “Georgia’s List of Approved Latex Suppliers”. For crumb rubber modified PG 67-22 to meet PG 76-22, use 30 mesh size ambient or cryogenic ground tire rubber at 10% of weight of total asphalt cement content. Trans-Polycotenamer shall be added at 4.5% of the weight of the crumb rubber to achieve better particle distribution. The maximum Phase Angle requirement is not applicable to the crumb rubber modified PG 76-22 (see note f).

### Test Requirements for Petroleum Asphalt Cements

<table>
<thead>
<tr>
<th>Test And Method</th>
<th>Test Temperature</th>
<th>Original Binder</th>
<th>Residue Of Binder After:</th>
</tr>
</thead>
<tbody>
<tr>
<td>Flash Point, Min., AASHTO T 48</td>
<td>PG 58-22 (Note e)</td>
<td>PG 64-22</td>
<td>PG 67-22</td>
</tr>
<tr>
<td>Viscosity, Max., AASHTO T 316, (Note a)</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Mass Loss (%), Max., AASHTO T 240, (Note b)</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Dynamic Shear, $G^*\sin \delta$, AASHTO T 315, 10 Rad/Sec</td>
<td>136 °F (58 °C)</td>
<td>147 °F (64 °C)</td>
<td>153 °F (67 °C)</td>
</tr>
<tr>
<td>Dissipated Energy, Dynamic Shear, $G^*\sin \delta$, AASHTO T 315, 10 Rad/Sec</td>
<td>72 °F (22 °C)</td>
<td>77 °F (25 °C)</td>
<td>80 °F (26.5 °C)</td>
</tr>
<tr>
<td>Creep Stiffness, 60 sec., AASHTO T 313, (Note c)</td>
<td></td>
<td>10 °F (-12 °C)</td>
<td></td>
</tr>
<tr>
<td>Direct Tension, 1.0 mm/min., AASHTO T314, Failure Strain</td>
<td></td>
<td>10 °F (-12 °C)</td>
<td></td>
</tr>
<tr>
<td>Multiple Stress Creep &amp; Recovery (MSCR) test, ASTM D7405, AASHTO TP70 (proposed), $J_{mr}$ 3.2 kPa, (Note f)</td>
<td></td>
<td>64 °C</td>
<td></td>
</tr>
</tbody>
</table>

Notes:

a. The Department may waive this requirement if the supplier warrants that the asphalt binder can be adequately pumped and mixed at temperatures that meet all applicable safety standards.

b. Heat loss by AASHTO: T 179 may be accepted in lieu of mass loss by AASHTO: T 240.

c. If the creep stiffness is below 300 000 kPa, the direct tension test is not required. If the creep stiffness is ≥300 000 kPa, report the Direct Tension Failure Strain value. Satisfy the m-value requirement in either case.

d. The maximum Phase Angle measured by DSR shall be ≤ 75 degrees.

e. The maximum Mass Loss shall be ≤ 1%, when used in conjunction with Bituminous Surface Treatment (Section 424).

f. MSCR requirement is applicable to the SBR, Crumb Rubber & TOR combination modified PG PG76-22 asphalt cement. Additionally, they shall meet all PG 76-22 requirements except for phase angle.
If modification is required, thoroughly blend the composite materials at the supply facility prior to being loaded into the transport vehicle. Ensure all blending procedures, formulation, and operations are approved by the Office of Materials and Research.

3. Certification:

Provide certified test results from an approved, certified laboratory of blends for proposed PG asphalt for each specification characteristic of the asphalt cement proposed for shipment. Provide the certified results to the State Materials and Research Engineer as required in Standard Operating Procedure (SOP 4).

In the event there is reason to suspect a sample will be outside specification limits, the State Materials and Research Engineer may interrupt production until test results are known.

B. Materials Warranty

General Provisions 101 through 150.

Office of Materials and Research
Section 824 — Cationic Asphalt Emulsion

Delete Subsection 824.1.01.B and substitute the following:

B. Referenced Documents
   AASHTO T 49
   AASHTO T 51
   AASHTO T 53
   AASHTO T 59
   AASHTO T 72
   AASHTO T 301
   AASHTO T 302
   ASTM D 5546 - 01
   QPL 65
   GDT 44
   GDT 91
   GDT 135

Add the following:

824.2.02 Latex-Modified Cationic Asphalt Emulsion

A. Requirements
   1. Latex Rubber Additive (LRA)
      a. Ensure the LRA is a natural latex or an unvulcanized styrene-butadine rubber in an emulsified latex form.
      b. Ensure that the LRA comes from an approved source listed in the Department’s current QPL 65 for use in cationic asphalt emulsion.
   2. Latex-Modified Cationic Asphalt Emulsion
      a. Use PG58-22 as the base asphalt.
      b. Add the LRA in the necessary proportions to result in a minimum of 3% polymer by weight of the asphalt residue.
c. Co-mill the LRA and asphalt cement while manufacturing the emulsified asphalt to produce a homogeneous mixture.

d. Ensure the latex-modified cationic asphalt emulsion, when undisturbed for 24 hours, shows no separation of emulsion and LRA and no color striations, but has a uniform color throughout.

e. Use a latex-modified cationic asphalt emulsion that meets the requirements in Table 2.

Table 2 – Requirements for Latex-Modified Cationic Asphalt Emulsion

<table>
<thead>
<tr>
<th>Tests on Emulsion</th>
<th>Tests on Emulsion Residue</th>
</tr>
</thead>
<tbody>
<tr>
<td>Viscosity, Saybolt Furol @ 122 °F (50 °C), sec.</td>
<td>Penetration @ 77 °F (25 °C), 100g, 5 sec., (dmm)</td>
</tr>
<tr>
<td>Storage stability, 24 hours, percent</td>
<td>Ductility, @ 77 °F (25 °C), 5 cm/min., (cm)</td>
</tr>
<tr>
<td>Settlement, 5 days, percent</td>
<td>Elastic recovery @ 50°F (10 °C), percent</td>
</tr>
<tr>
<td>Demulsibility, 35 ml, 0.8% dioctyl sodium sulfosuccinate, percent</td>
<td>Ring &amp; ball softening point, °F</td>
</tr>
<tr>
<td>Particle charge test</td>
<td>Solubility in toluene by centrifuge, percent</td>
</tr>
<tr>
<td>Sieve test, percent</td>
<td>Polymer solids content, percent</td>
</tr>
<tr>
<td>Residue by distillation, percent</td>
<td>1. AASHTO T-59 modified to include a maximum temperature of 400°F ± 10°F (204°C ± 5°C) to be held for a period of 15 minutes.</td>
</tr>
</tbody>
</table>

1. GDT-135, Residue by evaporation.

B. Fabrication
General Provisions 101 through 150.

C. Acceptance
Test as follows:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Penetration of bituminous materials</td>
<td>AASHTO T 49</td>
</tr>
<tr>
<td>Ductility</td>
<td>AASHTO T 51</td>
</tr>
<tr>
<td>Softening point of bitumen</td>
<td>AASHTO T 53</td>
</tr>
<tr>
<td>Testing emulsified asphalts</td>
<td>AASHTO T 59</td>
</tr>
<tr>
<td>Viscosity</td>
<td>AASHTO T 72</td>
</tr>
<tr>
<td>Elastic recovery</td>
<td>AASHTO T 301</td>
</tr>
<tr>
<td>Polymer content of polymer-modified emulsions</td>
<td>AASHTO T 302</td>
</tr>
<tr>
<td>Solubility of asphalt binders in toluene by centrifuge</td>
<td>ASTM D 5546 – 01</td>
</tr>
<tr>
<td>Residue by evaporation of latex-modified asphalt emulsions</td>
<td>GDT-135</td>
</tr>
</tbody>
</table>

D. Materials Warranty
General Provisions 101 through 150.
DELETE Subsection 831.2.01.A and substitute the following:

831.2.01 Air-Entraining Admixtures

A. Requirements

1. Use air-entraining admixtures listed in QPL 13.
2. Use air-entraining admixture materials meeting AASHTO M 154, Performance and Uniformity requirements.
3. Test compression and flexure strengths at 7 and 28 days.
4. Use air-entraining admixtures evaluated by the National Transportation Product Evaluation Program (NTPEP) test facility or other approved test facility.

DELETE Subsection 831.2.02.A and substitute the following:

831.2.02 Chemical Admixtures for Concrete

A. Requirements

1. Use chemical admixtures listed in QPL 14.
2. Use chemical admixture materials meeting AASHTO M 194 Physical requirements and Uniformity and Equivalence requirements for Types A, B, C, D, E, F, or G, unless otherwise specified.
   a. Waive the length change requirements.
   b. Ensure that the admixtures contain no more than 0.8 percent chloride, calculated as calcium chloride.
   c. Ensure that the air content does not exceed 4 percent when prepared in a standard batch without an added air-entraining agent.
3. Use chemical admixtures evaluated by the National Transportation Product Evaluation Program (NTPEP) test facility or other approved test facility.

Office of Materials and Research
Delete Subsection 832.2.01.A and substitute the following:

832.2.01 Burlap or Cotton Fabric
A. Requirements
1. Use burlap or cotton fabric meeting these requirements:
   - Burlap that is 10 to 18 oz./yd² (340 to 610 g/m²) or two layers of 6 or 7 oz/yd² (200 or 235 g/m²)
   - Cotton fabric that is white, loosely woven, and not less than 7 oz/yd² (235 g/m²)
   - Strips of burlap or cotton fabric that are between 3 and 6 ft (0.9 and 1.8 m) wide and 3 ft (1 m) longer than the width of the slab to be covered
2. Use burlap and cotton fabrics that do not contain starch or other material that could stain the concrete. If the fabric is new, soak and dry it before use.

Delete Subsection 832.2.03.A and substitute the following:

832.2.03 Membrane Curing Compound
A. Requirements
1. Use membrane curing compounds listed in QPL 16
2. Use liquid membrane-forming compounds meeting AASHTO M 148 requirements.
3. Use membrane curing compounds evaluated by the National Transportation Product Evaluation Program (NTPEP) test facility or other approved test facility.

Office of Materials and Research
Delete Subsection 833.2.06 and substitute the following:

A. Requirements

1. Silicone

   Furnish silicone sealant in a one-part or two part silicone formulation. Use sealant that is compatible with the surface to which it is applied. Do not use acid-cure sealants on Portland cement concrete.

   a. Use silicone that meets the physical requirements in Table 1. For a list of silicone joint sealant sources, please see QPL 66. Identify silicones as the following types:

      1) Type A—A one part, low modulus, non-sag silicone. Used to seal horizontal and vertical joints in Portland cement concrete pavements and bridges. Tooling is required.

      2) Type B—A one part, very low modulus, self-leveling silicone. Used to seal horizontal joints in Portland cement concrete pavements and bridges. Tooling is not normally required.

      3) Type C—A one part, ultra-low modulus, self-leveling silicone. Used to seal horizontal joints in Portland cement concrete pavements and bridges and joints between Portland cement concrete pavement and asphaltic concrete shoulders. Tooling is not normally required.

      4) Type D—A two part, ultra low modulus, self-leveling, rapid cure silicone. Used to seal horizontal joints in Portland cement concrete pavements and bridges and joints between Portland cement concrete pavement and asphaltic concrete shoulders. Tooling is not required.

   Use silicone sealant evaluated by the National Transportation Product Evaluation Program (NTPEP).

   c. Use sealant that is compatible with the surface to which it is applied. Do not use acid-cure sealants on Portland cement concrete.

   d. Use silicone that meets the following physical requirements:

<table>
<thead>
<tr>
<th>Physical Requirement</th>
<th>Type</th>
<th>A</th>
<th>B</th>
<th>C</th>
<th>D</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile Stress at 150% Strain, Max. psi (kPa) (Note 1)</td>
<td></td>
<td>45 (310)</td>
<td>40 (275)</td>
<td>15 (105)</td>
<td>25 (175)</td>
</tr>
<tr>
<td>Durometer Hardness, Score [0 °F and 77 °F ± 3 °F (−18 °C and 25 °C ± 2 °C)] (Note 1)</td>
<td></td>
<td>“A” 10-25</td>
<td>“00” 40-80</td>
<td>“00” 20-80</td>
<td>“00” 40-80</td>
</tr>
<tr>
<td>Bond to Concrete Mortar, Min. psi (kPa) (Note 1) (Note 3)</td>
<td></td>
<td>50 (345)</td>
<td>40 (275)</td>
<td>35 (240)</td>
<td>35 (240)</td>
</tr>
<tr>
<td>Tack Free Time (Skin-over) (Max. Minutes) (Note 2)</td>
<td></td>
<td>90</td>
<td>90</td>
<td>90</td>
<td>30</td>
</tr>
</tbody>
</table>
Section 833-Joint Fillers and Sealers

<table>
<thead>
<tr>
<th>Type Silicone</th>
<th>A</th>
<th>B</th>
<th>C</th>
<th>D</th>
</tr>
</thead>
<tbody>
<tr>
<td>Extrusion Rate (Min. Grams/Minute) (Note 4)</td>
<td>75</td>
<td>90</td>
<td>100</td>
<td>200-550</td>
</tr>
<tr>
<td>Non-volatile (Min. %)</td>
<td>90</td>
<td>90</td>
<td>90</td>
<td>90</td>
</tr>
<tr>
<td>Specific Gravity</td>
<td>1.1 - 1.5</td>
<td>1.1 - 1.5</td>
<td>1.1 - 1.5</td>
<td>1.2 - 1.5</td>
</tr>
<tr>
<td>Shelf Life (from date of shipment)</td>
<td>6 Months</td>
<td>6 Months</td>
<td>6 Months</td>
<td>6 Months</td>
</tr>
<tr>
<td>Movement Capability &amp; Adhesion (Note 1)</td>
<td>No adhesive or cohesive failure after 10 cycles at 0 ºF (-18 ºC).</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Ozone and U.V. Resistance (Note 1)</td>
<td>No chalking, cracking or bond loss after 5,000 hours.</td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Note 1: The cure time for these specimens shall be 21 days for Type A and 28 days for Type B, C and D. Specimens shall be cured at 77 ºF ± 3 ºF (25 ºC ± 2 ºC) and 50±5% relative humidity.

Note 2: At conditions of 77 ºF ± 3 ºF (25 ºC ± 2 ºC) and 50±5% relative humidity.

Note 3: Type C and D silicone shall also meet its bond strength requirement to asphalt concrete.

Note 4: Type D extrusion rate shall be within the range specified.

Bond Breakers

Bond breakers shall be chemically inert and resistant to oils, gasoline, solvents, and primer, if one is required. Install silicone sealants over a bond breaker to prevent the sealant from bonding to the bottom of the joint.

a. Use bond breakers that are chemically inert and resistant to oils, gasoline, solvents, and primer, if one is required.

Do not use bond breaker that will stain or adhere to the sealant.

Use either a backer rod or tape bond breaker.

1) Backer Rods

<table>
<thead>
<tr>
<th>Type</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>Type L</td>
<td>Closed-cell, expanded polyethylene foam</td>
</tr>
<tr>
<td>Type M</td>
<td>Closed-cell, polyolefin foam with a closed-cell skin over an open-cell core</td>
</tr>
</tbody>
</table>

Use backer rods that meet the following physical requirements:

<table>
<thead>
<tr>
<th>Physical Property</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Density</td>
<td>2 lb/ft³ (30 kg/m³) min.</td>
</tr>
<tr>
<td>Tensile strength</td>
<td>25 psi (170 kPa) min.</td>
</tr>
<tr>
<td>Water absorption</td>
<td>0.02 g/cm³ max.</td>
</tr>
</tbody>
</table>

2) Bond Breaking Tapes

Type N bond breaking tapes are made from extruded polyethylene with a pressure-sensitive adhesive on one side.

Bond breaking tapes may be used with all three types of silicone, but is suitable for bridge joints only.

Bond breaking tapes shall have a minimum thickness of .005 in (0.13 mm.).

Joint Sealant Certification

Submit, at no cost to the Department, a minimum of 30 gal (100 L) of material and certified test results on each lot of joint sealant furnished to a Project.

Submit a certification that verifies the sealant meets all the test requirements of this Specification, except the Bond to Concrete Mortar and Shore Durometer Hardness at 0 ºF (-18 ºC).

B. Fabrication

Prepare and install silicone and bond breakers according to Section 461.
C. Acceptance

1. Silicone

Test the silicone as follows:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile stress</td>
<td>ASTM D 412 (die C)</td>
</tr>
<tr>
<td>Durometer hardness</td>
<td>ASTM D 2240</td>
</tr>
<tr>
<td>Bond to concrete mortar</td>
<td>GDT 106</td>
</tr>
<tr>
<td>Tack free time (skin-over)</td>
<td>GDT 106*</td>
</tr>
<tr>
<td>Extrusion rate</td>
<td>GDT 106</td>
</tr>
<tr>
<td>Non-volatile</td>
<td>GDT 106</td>
</tr>
<tr>
<td>Specific gravity</td>
<td>ASTM D 792 (Method A)</td>
</tr>
<tr>
<td>Movement capability and adhesion</td>
<td>GDT 106</td>
</tr>
<tr>
<td>Ozone and UV resistance</td>
<td>ASTM C 793</td>
</tr>
</tbody>
</table>

*In cases of dispute, use ASTM C 679 as a referee test.

Bond Breakers

Test the bond breaker backer rods as follows:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Density</td>
<td>ASTM D 1622</td>
</tr>
<tr>
<td>Tensile strength</td>
<td>ASTM D 1623</td>
</tr>
<tr>
<td>Water absorption</td>
<td>ASTM C 1016</td>
</tr>
</tbody>
</table>

Department Responsibility

The Department will:

a. Evaluate the sealant in the field before accepting any silicone sealants that meet the requirements of this Specification.

Install the material submitted by the Contractor in roadway and/or bridge joints. The material shall be in place for two winters without failure before being accepted.

Reject any sealant or bond breaker that is evaluated and approved, yet fails in actual use.

D. Materials Warranty

General Provisions 101 through 150.
Delete Section 845 and substitute the following:

845.1 General Description
This section includes the requirements for smooth-lined, corrugated polyethylene culvert pipe.

845.1.01 Related References
A. Standard Specifications
   General Provisions 101 through 150.

B. Referenced Documents
   AASHTO M 294
   AASHTO Standard Specifications for Highway Bridges, Division II
   QPL 51

845.2 Materials

845.2.01 Smooth-lined, Corrugated Polyethylene (PE) Culvert Pipe
A. Requirements
   1. Use pipe meeting the requirements of AASHTO M 294, Type S.
   2. Use pipe evaluated by the National Transportation Product Evaluation Program (NTPEP) test facility or other approved test facility.
   3. Ensure pipe is produced from an approved source listed on QPL 51.
   4. Use fittings and couplings as recommended by the manufacturer and approved by the Office of Materials and Research. The fittings and couplings shall comply with the joint performance criteria of AASHTO Standard Specifications for Highway Bridges, Division II. Ensure that the joints are “soiltight” per the AASHTO bridge specifications.

B. Fabrication
   General Provisions 101 through 150.

C. Acceptance
   General Provisions 101 through 150.

D. Materials Warranty
   General Provisions 101 through 150.
Delete Section 863 and Substitute the following:

**863.1 General Description**

This section includes the requirements for applying preservatives, conditioning, treating, inspecting, marking, testing, and documenting the necessary information for treated timber used in Department Work.

**863.1.01 Related References**

A. Standard Specifications

   General Provisions 101 through 150.

B. Referenced Documents

   American Wood Preservers Association (AWPA), C14, “Wood for Highway Construction—Preservative Treatment by Pressure Method”
   - AWPA C2
   - AWPA M2
   - AWPA M3
   - AWPA P9
   - AASHTO M 133
   - QPL 50

**863.2 Materials**

**863.2.01 Conditioning and Preservative Treatment**

A. Requirements

   1. Condition and preservative treat all timber products to meet the requirements of American Wood Preservers Association (AWPA) Standard C14, “Wood for Highway Construction—Preservative Treatment by Pressure Method,” except as described in this Section.

   2. Treatment Plants

      Ensure treatment plants comply with quality control procedures in AWPA M3.

      a. To expedite the work, a commercial inspection agency approved by the Department will inspect and test all treated timber products, including any preservative treatment at the treatment plant before it is delivered to the project. The treatment plant shall bear all the cost associated with the inspection and test.
b. Before requesting an inspection, the authorities of the treatment plant shall acquaint themselves with the timber specification requirements and shall segregate the material to be inspected for Department work from other stock.

3. Preservatives

Use preservatives that meet the requirements in the AWPA Standard, unless otherwise specified in the Plans or the Specifications.

a. You may select one of three preservatives (creosote, pentachlorophenol, Chromated Copper Arsenate (CCA)) from the Materials and Usage Table in AWPA C14.

b. Ensure pentachlorophenol solutions have at least 5 percent pentachlorophenol, by weight, dissolved in the petroleum solvents specified or pentachlorophenol in AWPA P9, Type “A.”

B. Fabrication

1. As practicable, cut, frame, and bore timber before treatment.

2. Condition the timber first. For Southern Pine species, use the following treatment:

<table>
<thead>
<tr>
<th>Preservative</th>
<th>Treatment Conditions</th>
</tr>
</thead>
<tbody>
<tr>
<td>Penta-petroleum</td>
<td>Dry in kiln to 30% average moisture content or less or condition in steam</td>
</tr>
<tr>
<td>Chromated Copper Arsenate (CCA)</td>
<td>Dry in kiln to 25% average moisture content or less</td>
</tr>
</tbody>
</table>

 NOTE: Do not heat the wood in the preservative, and do not use Boulton drying.

3. Preservative Penetration

Ensure the preservative penetrates at least 3 in (75 mm) or 90 percent of the sapwood for all lumber, timber, wood fence posts, and ties in contact with the ground.

a. Ensure lumber, timber, and ties that do not contact the ground meet AWPA C2 requirements.

b. Ensure preservative penetrates all other materials, piles, and poles according to applicable AWPA requirements.

4. Preservative Retention

Treat guard rail posts and offset blocks with pentachlorophenol or CCA with a minimum 0.6 lb/ft³ (9.6 kg/m³) retained in the outer 0.6 in (15 mm), as required in AWPA C14 and C2.

5. Retreatment:

You may retreat a charge of material, or a portion of it, if the initial treatment does not meet requirements for retention, penetration, or appearance. The Department will allow only one retreatment.

 NOTE: The Department will reject any damage due to retreatment.

6. Conditioning after Treatment

a. Condition material that is dust-free.

b. For lumber or timber that is treated with water-borne preservative and is to be painted, dry by air, kiln, or some method of artificial conditioning, to a moisture content of not more than 19 percent of the weight of the oven-dry wood.

c. Protect the treated lumber from the elements with a prime coat of paint or other approved means.

d. Ensure the moisture content does not rise above 19 percent before applying the first coat of paint.

e. Dry material treated with water-borne preservative that will not be painted to surface dryness in air or otherwise before installing it.

C. Acceptance

1. Inspection

The Department will sample and test preservatives according to the requirements of AASHTO M 133.

 NOTE: Check OPL 50 for pre-approved manufacturers that supply material compliant with this specification.

a. The Department will determine the level of preservative retention by testing the 0.6 to 1.5 in (15 to 38 mm) assay zone.
b. Unless otherwise provided, an approved commercial inspection agency will inspect treated timber products according to AWPA M2.

c. The Inspector will test before, during, and after treating.

2. Marking
   The Inspector will mark each acceptable piece with a hammer stamp before and after treatment.
   a. Stamp only 25 percent of the offset blocks after treatment.
   b. Ensure that both inspection stamps identify the Inspector. Ensure that the before-treatment stamp is clearly distinguished from the after-treatment stamp.

3. Reporting
   The Inspector from an approved commercial inspection agency shall:
   a. Prepare reports of the treating process and results of the inspection that confirm treatment was completed according to these Specifications.
   b. Furnish these reports to the Office of Materials and Research.
   c. Report according to AWPA M2.
   d. Get a shipping report from the treatment plant showing the project number, purchaser, sizes and amounts of materials, and preservative type for each shipment for Department Work.
   e. Furnish the shipment report and the treatment report to the Office of Materials and Research.

D. Materials Warranty
   1. Retest treated material that has been in stock for two years before using.
   2. The Department will reject any materials that fail to meet specifications unless they are retreated to meet all applicable requirements.

Office of Materials and Research
Delete Subsection 865.1 and substitute the following:

This section includes the following requirements for precast-prestressed concrete bridge members and piling using High Performance Portland cement concrete as shown in the Plans:

1. Manufacturing
2. Inspecting
3. Testing
4. Marking
5. Painting
6. Rubbing as specified
7. Plant handling
8. Storing
9. Shipping

The term “precast-prestressed concrete” is referred to as “prestressed concrete” in the rest of this Section.

Add the following to Subsection 865.2:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section</th>
</tr>
</thead>
<tbody>
<tr>
<td>Concrete, Class AAA HPC</td>
<td>500</td>
</tr>
</tbody>
</table>

Add the following to the end of Subsection 865.2.01.B.7.a.6:

Optional Method of Curing for Release Strengths with HPC: Temperature match curing (“Sure Cure” or equivalent methods) is allowed for specimens used to determine when stress may be transferred to the concrete for High Performance Concrete Units.
Delete Section 881 and substitute the following:

881.1 General Description
This section includes the requirements for the following fabrics:

- Plain cotton duck
- Rubber-impregnated cotton duck
- Burlap and cotton bags
- Plastic filter fabric
- Pavement reinforcement fabric
- Silt fence filter fabric

881.1.01 Related References
A. Standard Specifications
   Section 106—Materials Certification
B. Referenced Documents
   Federal Specification CCC-C 419 Type III
   ASTM D 36
   ASTM D 146
   ASTM D 412
   ASTM D 1777
   ASTM D 3786
   ASTM D 4355
   ASTM D 4632, GRAB
   ASTM D 4751
881.2 Materials

881.2.01 Plain Cotton Duck
A. Requirements
1. Use plain cotton duck meeting the requirements of Federal Specification CCC-C 419 Type III.
2. Ensure the duck weighs at least 8 oz./yd² (270 g/m²).

B. Fabrication
General Provisions 101 through 150.

C. Acceptance
General Provisions 101 through 150.

D. Materials Warranty
General Provisions 101 through 150.

881.2.02 Rubber-Impregnated Cotton Duck
A. Requirements
1. Use preformed rubber-impregnated fabric pads made of multiple layers of 8 oz (270 g) cotton duck, impregnated and bound with high quality natural rubber, or made of equivalent materials compressed into resilient pads of uniform thickness.
2. Use enough plies to reach the specified thickness after compression and vulcanizing.
3. Ensure the finished pad withstands compression loads of not less than 10,000 psi (70 MPa) when applied perpendicular to the plane of the laminations. Ensure the pad does not extrude or harmfully reduce in thickness.

B. Fabrication
General Provisions 101 through 150.

C. Acceptance
General Provisions 101 through 150.

D. Materials Warranty
General Provisions 101 through 150.

881.2.03 Burlap Bags
A. Requirements
Use burlap bags made of at least 95 percent jute and manila fibers.
Use burlap weighing 8 to 18 oz/10 ft² (250 to 550 g/m²).
Use bags with a capacity of 1 to 2 ft³ (0.03 to 0.06 m³).

B. Fabrication
General Provisions 101 through 150.

C. Acceptance
General Provisions 101 through 150.

D. Materials Warranty
General Provisions 101 through 150.

881.2.04 Cotton Bags
A. Requirements
1. Use cotton bags with Osnaburg 40 x 26 thread count and a nominal fabric weight of 6.8 oz/yd² (230 g/m²).
2. Use bags having 1/2 in (13 mm) sewn seams with at least 1 stitch per 1/5 in (5 mm).
3. Use 4 or 5 ply, 12 cotton yarn or equivalent for the stitches.
4. Ensure seam efficiency is at least 80 percent. Ensure the inside measurements tolerance is ± 1/2 in (13 mm).

B. Fabrication
General Provisions 101 through 150.

C. Acceptance
General Provisions 101 through 150.

D. Materials Warranty
General Provisions 101 through 150.

881.2.05 Plastic Filter Fabric
A. Requirements
1. Use pervious sheets of plastic yarn made from a long-chain synthetic polymer. Use polymer composes of at least 85 percent by weight of propylene, ethylene, amide, ester, or vinylidene chloride.
Use a sheet of plastic yarn containing stabilizers and/or inhibitors added to the base plastic to make the filaments resistant to deterioration due to ultra-violet and/or heat exposure.
2. Ensure the fabric is finished so that the filaments will retain their relative position with respect to each other.
3. Use fabric without defects, rips, holes, or flaws.
4. Use fabric meeting the following physical requirements for woven and non-woven fabric:

<p>| | | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Woven Fabrics</strong></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Tensile strength (any direction)</td>
<td>200 lbs (890 N) minimum</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Bursting strength</td>
<td>500 psi (3.5 MPa) minimum</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Elongation before breaking</td>
<td>10% to 35%</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Percent open area</td>
<td>4.0% to 6.5%</td>
<td></td>
<td></td>
</tr>
<tr>
<td><strong>Non-woven Fabrics</strong></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Puncture resistance</td>
<td>30 lbs (135 N) minimum</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Grab tensile strength</td>
<td>65 lbs (290 N) minimum</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Grab elongation</td>
<td>40% minimum</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Flow rate [H from 3 to 1 in (75 to 25 mm)]</td>
<td>50 gal/min/ ft²</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>
5. Use fabric evaluated by the National Transportation Product Evaluation Program (NTPEP).

6. Seams
   a. Get approval on the seams from the Engineer before use on a Project.
   b. Use fabric sewn with thread of the same chemical requirements as the fabric, or use fabric bound with cement or heat. Either have the fabric bound or sewn at the point of manufacture or at a location approved by the Engineer.
   c. Seam Uses: You may use one seam in edge drain and underdrain applications. You may bond or sew fabric together to form sections at least 6 ft (1.8 m) wide for use under rip rap or behind retaining walls.

7. Fabric Use
   a. Use woven fabrics beneath rip rap when dropping stone from 3 ft (1 m) or less.
   b. You may use woven fabrics that meet the flow rate for edge drains.
   c. Use non-woven fabrics to line edge drains, underdrains, or behind retaining walls, where specified.
   d. Do not use non-woven fabrics for filter beneath rip rap.

B. Fabrication
   General Provisions 101 through 150.

C. Acceptance
   Test according to the following:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Puncture resistance</td>
<td>ASTM D 4833</td>
</tr>
<tr>
<td>Tensile strength, elongation, grab strength</td>
<td>ASTM D 4632</td>
</tr>
<tr>
<td>Bursting strength</td>
<td>ASTM D 3786</td>
</tr>
<tr>
<td>Percent open area</td>
<td>GDT 88</td>
</tr>
<tr>
<td>Flow rate</td>
<td>GDT 87</td>
</tr>
</tbody>
</table>

1. See QPL 28 for acceptable woven and non-woven fabrics meeting the requirements of this Specification. See QPL 47 for acceptable Geocomposite wall drains.

2. The Department will reject any fabrics that meet this Specification but fail to perform in actual use.

D. Materials Care and Warranty
   Wrap fabric in burlap or similar heavy duty protection during shipment and storage to protect it from mud, dirt, dust, and debris.

881.2.06 Pavement Reinforcement Fabric

A. Requirements

Type I and Type II Pavement Reinforcement Fabric

1. Use pavement reinforcement fabric that has the following properties:
   - Is non-woven, heat-resistant material composed of polypropylene or polyester fibers
   - Can be saturated with asphalt cement
   - Can be placed smooth with mechanical devices and be without wrinkles
• Can withstand the heat of asphaltic concrete mixes during paving operations
• Can withstand normal field handling and construction operations without damage

For a list of sources, see QPL 40.

• Meets the following physical requirements. The bid item or Plans will indicate which type of fabric is required for a Project.

<table>
<thead>
<tr>
<th></th>
<th>Type I</th>
<th>Type II</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile strength, minimum</td>
<td>90 lbs (400 N)</td>
<td>125 lbs (555 N)</td>
</tr>
<tr>
<td>Elongation at break</td>
<td>40% min., 100% max.</td>
<td>40% min., 100% max.</td>
</tr>
<tr>
<td>Asphalt retention, minimum</td>
<td>0.18 gal/yd² (0.8 L/m²)</td>
<td>0.28 gal/yd² (1.3 L/m²)</td>
</tr>
</tbody>
</table>

2. Submit a certificate from the manufacturer showing the physical properties of the material used and how it meets this Specification. Submit the certificate according to Subsection 106.05, “Materials Certification.”

3. Demonstrate to the Department that fabric meeting the physical properties requirements of this Specification has been used successfully in installations with similar environmental and Project conditions.

**High Strength Pavement Reinforcement Fabric**

1. Use pavement reinforcement fabric with the following properties:
   • Is a flexible, water-resistant, high-density asphaltic membrane laminated between two layers of high strength, heat resistant polypropylene or polyester fabric.
   • Can be placed smooth with mechanical devices and be without wrinkles.
   • Can withstand the heat of asphaltic concrete mixes during paving operations.
   • Can withstand normal field handling and construction operations without damage.
   • Has a self-adhesive backing adhered to a film release liner.

For a list of sources, see QPL 40.

• Meets the following physical requirements. The bid item or Plans will indicate which type of fabric is required for a Project.

<table>
<thead>
<tr>
<th></th>
<th>Type I</th>
<th>Type II</th>
</tr>
</thead>
<tbody>
<tr>
<td>Width, minimum</td>
<td>18 in (450 mm)</td>
<td></td>
</tr>
<tr>
<td>Tensile strength, minimum</td>
<td>1,800 lbs/in² (12 MPa)</td>
<td></td>
</tr>
<tr>
<td>Elongation</td>
<td>20% to 50%</td>
<td></td>
</tr>
<tr>
<td>Softening Point (Asphaltic membrane), minimum</td>
<td>190 °F (87 °C)</td>
<td></td>
</tr>
<tr>
<td>Caliper</td>
<td>0.135 inch (3.43 mm)</td>
<td>95% retained after loading</td>
</tr>
<tr>
<td>Pliability (Cold Flex)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>2&quot; (50 mm) X 5&quot; (125 mm) specimen, condition specimen at 0 °F (-18 °C ) for 1 hour, 180° bend on 2&quot; (50 mm) mandrel</td>
<td>No Separation</td>
<td></td>
</tr>
</tbody>
</table>

2. Submit a certificate from the manufacturer showing the physical properties of the material used and how it meets this Specification. Submit the certificate according to Subsection 106.05, “Materials Certification.”

3. Demonstrate to the Department that fabric meeting the physical properties requirements of this Specification has been used successfully in installations with similar environmental and Project conditions.

**B. Fabrication**

General Provisions 101 through 150.
C. Acceptance

Type I and Type II Pavement Reinforcement Fabric

Test according to the following:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile strength</td>
<td>ASTM D 4632 Grab</td>
</tr>
<tr>
<td>Elongation</td>
<td>ASTM D 4632 Grab</td>
</tr>
<tr>
<td>Asphalt retention</td>
<td>GDT 95</td>
</tr>
</tbody>
</table>

High Strength Pavement Reinforcement Fabric

Test according to the following:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile strength</td>
<td>ASTM D 412</td>
</tr>
<tr>
<td>Elongation</td>
<td>ASTM D 412</td>
</tr>
<tr>
<td>Softening Point</td>
<td>ASTM D 36</td>
</tr>
<tr>
<td>Caliper</td>
<td>ASTM D 1777</td>
</tr>
<tr>
<td>Pliability (Cold Flex)</td>
<td>ASTM D 146</td>
</tr>
</tbody>
</table>

D. Materials Warranty

General Provisions 101 through 150.

881.2.07 Silt Fence Filter Fabric

A. Requirements

1. Use approved silt fence from QPL 36.
   a. Type “A” and “B” Fences: Use either woven or nonwoven filter fabric for Type “A” and “B” fences. If using woven fabric, the fabric may have slit tape yarns in one direction (warp or fill) only.
   b. Type “C” Fences: Use non-calendered woven fabric constructed with monofilament yarns only.

   NOTE: Approved fabrics must consistently exceed the minimum requirements of this Specification as verified by the Office of Materials and Research. If a fabric is removed from the Qualified Products List, do not use it in the work until the Department has reestablished the product’s acceptability.

2. Ensure silt fence filter fabrics have the following characteristics:
   - Has strong rot-proof synthetic fibers formed into either a woven or non-woven fabric
   - Has no treatment or coating that might significantly alter its physical properties after installation
   - Contains stabilizers and/or inhibitors to make the filaments resistant to deterioration resulting from exposure to sunlight or heat
   - Makes a pervious sheet of synthetic fibers oriented into a stable network so that the fibers retain their relative position with respect to each other under normal handling, installation, and service conditions
   - Has finished fabric edges to prevent the outer yarn from pulling away from the fabric
   - Has no defects or flaws that would significantly affect its physical and/or filtering properties
   - Meets the following physical or dimensional requirements:

<p>| Type Fence | A | B | C |</p>
<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Minimum tensile strength, pounds (newtons) (1)</td>
<td>Warp – 120 (530) Fill – 100 (445)</td>
</tr>
<tr>
<td>Elongation (% Max.)</td>
<td>40</td>
</tr>
<tr>
<td>Apparent opening size (max. sieve size)</td>
<td>No. 30 (600 um) No. 30 (600 um) No. 30 (600 um)</td>
</tr>
<tr>
<td>Flow rate, gal/ min./ft² (L/min./m²)</td>
<td>25 (1015) 25 (1015) 70 (2850)</td>
</tr>
<tr>
<td>Ultraviolet stability (2)</td>
<td>80</td>
</tr>
<tr>
<td>Bursting strength, psi (kPa)</td>
<td>175 (1200) 175 (1200) 175 (1200)</td>
</tr>
<tr>
<td>Minimum fabric width</td>
<td>36 in (914 mm) 22 in (559 mm) 36 in (914 mm)</td>
</tr>
</tbody>
</table>

1. Minimum roll average of five specimens.
2. Percent of required initial minimum tensile strength.

3. Use silt fence filter fabrics evaluated by the National Transportation Product Evaluation Program (NTPEP).

B. Fabrication

The fabric may be manufactured with pockets for posts, hems with cord, or with posts pre-attached using staples or button head nails.

Ensure the fabric has the manufacturer’s name and product trade name labeled on the fabric at a minimum of 25 ft (7.6 m) intervals. Ensure the fabric has a color yarn mark in the fabric 14 inches (355 mm) ± 0.5 inch (12 mm) from both top and bottom ends for Type A and C and 8 inches (203 mm) ± 0.5 inch (12 mm) from both top and bottom ends for Type B fabric.

C. Acceptance

Test according to the following:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile strength</td>
<td>ASTM D 4632</td>
</tr>
<tr>
<td>Elongation</td>
<td>ASTM D 4632</td>
</tr>
<tr>
<td>Apparent opening size</td>
<td>ASTM D 4751</td>
</tr>
<tr>
<td>Flow Rate</td>
<td>GDT 87</td>
</tr>
<tr>
<td>Ultraviolet stability</td>
<td>ASTM D 4632 (after 300 hours weathering according to ASTM D 4355)</td>
</tr>
<tr>
<td>Bursting strength</td>
<td>ASTM D 3786, Diaphragm Bursting Strength Tester</td>
</tr>
</tbody>
</table>

D. Materials Care and Warranty

Wrap fabric in a heavy-duty protective covering during shipment and storage to protect it from mud, dirt, dust and debris.

Do not expose fabric to temperatures greater than 140 °F (60 °C).

881.2.08 Filter Fabric for Embankment Stabilization

See Special Provision.
Delete Subsection 894.2.0.6.A and substitute the following:

894.2.06 Silt Fabric Fencing

A. Requirements

1. Fabric
   b. Use a woven wire support fence or a polypropylene support mesh with Type “C” fence.
      1) Woven Wire Support Fence
         a) Ensure the woven wire support fence is at least 26 inches (660 mm) high with at least 6 horizontal wires.
         b) Ensure the vertical wires have a maximum spacing of 12 in (155 mm).
         c) Ensure the top and bottom wires are at least 10 gauge (2.49 mm) and all other wires are at least 12-1/2 gauge (2.03 mm). Use Washburn and Moen Standard requirements for determining wire gauge.
         d) You may use other designs subject to approval by the Office of Materials and Research.
      2) Polypropylene Support Mesh
         a) Ensure the polypropylene support mesh is sewn to the fabric 2 in (50 mm) ± 1 in (25 mm) from top and bottom of fabric and 11 in (279 mm) ± 1 in (25 mm) from top and bottom of fabric. Use a T-90 black polyester thread to sew mesh to fabric with a lock stitch at 5 to 7 stitches per inch.
         b) Ensure the height of the polypropylene support mesh is at least 36 in (914 mm) with a plus tolerance of 1 in (25 mm).
         c) Ensure the polypropylene support mesh minimum tensile strength in the machine direction is 60 lb/3 inches and 72 lb/3 inches in the transverse direction.
         d) Ensure minimum average weight of the polypropylene support mesh is 10.3 lb/1000 ft².
         e) Ensure the average strand count of the polypropylene support mesh in the machine direction is 9.0 ± 1.5 per 10 inches and 14.5 ± 0.7 per 10 inches in the transverse direction.
         f) Ensure the polypropylene support mesh contains stabilizers and/or inhibitors that make the mesh resistant to deterioration from exposure to sunlight or heat.
2. Posts

Use post sizes and types as determined by the type of fence being installed. Generally hardwood posts will be limited to ash, hickory, or oak. Other hardwoods may be acceptable if approved by the Office of Materials and Research.

a. Type “A” Fence: Use either wood or steel posts that are at least 4 ft (1.2 m) long.
   1) If using soft wood, use posts that are at least 3 in (75 mm) in diameter or nominal 2 x 4 in (33 x 89 mm) and straight enough to provide a fence without noticeable misalignment.
   2) If using hardwood, use posts that are 1-1/2 x 1-1/2 in (38 x 38 mm) with a minus tolerance of 3/8 in (9 mm) providing the cross sectional area is at least 2.15 in^2 (1385 mm^2).
   3) If using steel, use posts that are “U,” “T,” or “C” shaped with a minimum weight of 1.15 lb/ft (1.7 kg/m), and have projections for fastening the fence to the posts.

b. Type “B” Fence: Use either wood or steel posts that are at least 3 ft (900 mm) long.
   1) If using soft wood, use posts that are at least 2 in (50 mm) in diameter or nominal 2 x 2 in (33 x 33 mm).
   2) If using hardwood, use posts that are 1 x 1 in (25 x 25 mm) with a minus tolerance of 1/4 in (6 mm) providing the cross sectional area is at least 0.95 in^2 (610 mm^2).
   3) If using steel posts, use types “U,” “T,” or “C” shapes with a minimum weight of 0.75 lb/ft (1.1 kg/m).

c. Type “C” Fence:
   1) Woven Wire Supported: Use only steel posts with a minimum length of 4 ft (1.2 m). Use “U,” “T,” or “C” shaped posts with a minimum weight of 1.15 lb/ft (1.7 kg/m). Use posts that have projections for fastening the woven wire and filter fabric.
   2) Polypropylene Mesh Supported: Use either wood or steel posts that are at least 4 ft (1.2 m) long.
      a) If using soft wood, use posts that are at least 3 in (75 mm) in diameter or nominal 2 x 4 in (33 x 89 mm) and straight enough to provide a fence without noticeable misalignment.
      b) If using hardwood, use posts that are 2 x 2 in (50 x 50 mm) with a minus tolerance of 1/4 in (6 mm) providing the cross sectional area is at least 3.28 in^2 (2120 mm^2).
      c) If using steel posts, use “U,” “T,” or “C” shaped posts with a minimum weight of 1.15 lb/ft (1.7 kg/m). Use posts that have projections for fastening the woven wire and filter fabric.

   NOTE: You must use woven wire or polypropylene mesh to provide extra support for Type “C” fence installations.

3. Fasteners for Wooden Posts

   a. Wire Staples: Use staples that are at least 17 gauge (1.37 mm), legs at least 1/2 in (13 mm) long, and a crown at least 3/4 in (19 mm) wide.

   b. Nails: Use nails that are at least 14 gauge (2.03 mm), 1 in (25 mm) long, with button heads of at least 3/4 in (19 mm).

Office of Materials and Research
895.1 General Description
This section covers the use of anionic Polyacrylamide (PAM) as a floculant on construction projects.

895.1.01 Related References
A. Standard Specifications
   Section 700—Grassing
B. Referenced Documents
   QPL 84

895.2 Materials
A. Requirements
   Use only Polyacrylamide (PAM) products listed on the Qualified Products List (QPL 84).
   Ensure Polyacrylamide (PAM) emulsions and powders are of the anionic type only and meet the following requirements:
   1. Meets the EPA and FDA acrylamide monomer limits of equal to or greater than 0.05% acrylamide monomer.
   2. Has a density of 10% to 55% by weight and a molecular weight of 16 to 24 Mg/mole.
   3. Mixture is non-combustible.
   4. Contains only manufacturer recommended additives.

B. Fabrication
   General Provisions 101 through 150.

C. Acceptance
   1. Polyacrylamide (PAM) products must meet the requirements of Section 895 and be listed on QPL 84.
   2. Provide manufacturer’s data on charge density and molecular weight.

D. Materials Warranty
   General Provisions 101 through 150.
Delete Section 913 and substitute the following:

913.1 General Description
This section includes the requirements for reflective sheeting.

913.1.01 Definitions

- Reflective Sheeting Types:
  - Type I: Medium-intensity retroreflective sheeting (engineering grade) that is typically an enclosed lens glass-bead retroreflective material.
  - Type II: Medium-high-intensity retroreflective sheeting (super engineering grade), that is typically enclosed lens glass-bead retroreflective material.
  - Type III: High-intensity retroreflective sheeting that is typically an encapsulated glass-bead retroreflective material.
  - Type IV: High-intensity retroreflective sheeting that is typically an unmetallized microprismatic retroreflective element material.
  - Type V: Super-high-intensity retroreflective sheeting that is typically a metallized microprismatic retroreflective element material. This material is typically used for delineators.
  - Type VI: Elastomeric high-intensity retroreflective sheeting without adhesive that is typically a vinyl microprismatic retroreflective material. This material is typically used for orange temporary roll up signs.
  - Type VII: Super-high-intensity retroreflective sheeting that is typically an unmetallized microprismatic retroreflective element material.
  - Type VIII: Super-high-intensity retroreflective sheeting that is typically an unmetallized microprismatic retroreflective element material.
  - Type IX: Very-high-intensity retroreflective sheeting that is typically an unmetallized microprismatic retroreflective element material.
  - Type X: Super-high intensity retroreflective sheeting that is typically an unmetallized microprismatic retroreflective element material.

913.1.02 Related References

A. Standard Specifications
   General Provisions 101 through 150.
B. Referenced Documents
   1. ASTM D 4956
   2. QPL 29

913.2 Materials

913.2.01 Type I, II, III, IV, V, VI, VII, VIII, IX, and X Reflective Sheeting

A. Requirements
   1. Use reflective sheeting that meets the requirements of ASTM D 4956.
   2. Use reflective sheeting as listed in QPL 29.
   3. Use reflective sheeting that has been evaluated by the National Transportation Product Evaluation Panel (NTPEP) test facility or other approved test facility.
   4. Submit the following to the Department:
      a. A certificate with each lot or shipment stating the following:
         • The material supplied will meet all the test requirements listed herein.
         • You have performed the specified tests to ensure compliance.
         • You will submit test results upon request.

B. Fabrication
   General Provisions 101 through 150.

C. Acceptance
   1. The Engineer will reject reflective sheeting in the following situations:
      a. The material fails to meet any one of the designated requirements.
      b. The material meets the requirements but later fails during sign fabrication or in actual field use. Cracks, wrinkles, delamination, color change, or abnormal loss of reflectivity constitute failure.
      c. Natural causes deteriorate the material to the extent that:
         1) The sign is ineffective for its intended purpose as defined in Subsection 913.2.01.C.1.b above.
         2) The average nighttime reflective brightness does not meet the outdoor weathering requirements of ASTM D4956.

D. Materials Warranty
   Transfer to the Department a performance warranty for Type I, II, III, IV, V, VI, VII, VIII, IX, or X reflective sheeting issued by the manufacturer.

   Ensure that the warranties cover the full replacement cost, including material and labor.

   Include in these warranties a provision that the warranty is subject to a transfer to the Department.

   Submit a warranty from the manufacturer that states that the reflective sheeting—processed, applied to sign blank materials, and cleaned—meets the outdoor weathering photometric requirements of ASTM D 4956.

Office of Materials and Research
Section 917—Reflectors and Nonreflective Characters

Delete Subsection 917.1 and substitute the following:

917.1 General Description
This section includes the requirements of demountable characters with Type IX reflective sheeting, and direct-applied, nonreflective characters.

917.1.01 Related References
A. Standard Specifications
   Section 106—Certification of Materials
   Section 913—Reflectorizing Materials
B. Referenced Documents
   ASTM B 209 (B 209M)
   ASTM D 822

Delete Subsection 917.2.01 and substitute the following:

917.2.01 Demountable Characters with Type IX Reflective Sheeting
A. Requirements
   1. Use Type IX reflective sheeting letters, numerals, symbols, and borders that meet the requirements of Subsection 913.2.02, Type IX.
   2. Use a silver color, unless otherwise specified on the Plans.
   3. Apply the characters to aluminum flat frames as recommended by the sheeting manufacturer.
   4. Use flat frames (letter, numerals, symbols and borders) made from aluminum sheet 0.032 in (0.813 mm) thick matching ASTM B 209 (209M), Alloy 3003-H14.
   5. Submit to the Department:
      • One letter of a predominant size and type to be used on the Project.
      • A certificate to the Engineer stating that the material used on the Project is the same as the sample submitted.
Section 917—Reflectors and Nonreflective Characters

B. Fabrication

1. Before applying any sheeting, properly degrease, etch, and treat each frame with a light, tight amorphous chromate-type coating.

2. Mechanically apply the reflective sheeting to the prepared flat aluminum frames. Use the proper equipment as prescribed by the sheeting manufacturer.

3. When recommended by the sheeting manufacturer, coat the completed demountable letters, numerals, symbols and borders with a clear finish approved by the sheeting manufacturer.
   Apply the clear coat to the sheeting surface to ensure the sheeting has a fully glossy coat and a complete edge seal.

4. Ensure that the finished letters, numerals, symbols, and borders show careful workmanship, are clean cut, sharp, and have a plane surface.

5. Use the character size and shape to determine the hole spacing to mount the frame with aluminum rivets or other approved non-corrosive fasteners. Do not space holes more than 8 in (200 mm) on center.

C. Acceptance

The Department will accept the material based on test results of samples taken by the Department or of samples submitted by the manufacturer or fabricator, when directed. The sample shall consist of one letter of predominant size and type to be used on the Project. Samples submitted by the manufacturer or fabricator to the Engineer, shall include a certificate stating that the material used on the Project is the same as the sample submitted.

D. Materials Warranty

General Provisions 101 through 150.

Office of Materials and Research
Delete Section 919 and Substitute the following:

919.1 General Description
This section includes the requirements for raised pavement marker materials for use in reflective, ceramic, and channel markers.

919.1.01 Related References
A. Standard Specifications
   General Provisions 101 through 150.

B. Referenced Documents
   ASTM C 424
   ASTM C 373
   ASTM D 2240
   ASTM D 4280
   Federal Method TT-T-141, Method 4252

919.2 Materials
A. Requirements
   Do not use any marker materials until the laboratory approves it.
   1. Use raised pavement marker sources as listed in QPL 76.
   2. Use raised pavement markers of the type shown in the Plans or specified in the proposal. This Specification references markers as follows:

<table>
<thead>
<tr>
<th>Type</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>One-way, one-color, 4 x 2 in (100 mm x 50 mm), reflective</td>
</tr>
<tr>
<td>2</td>
<td>Two-way, one-color, 4 x 2 in (100 mm x 50 mm), reflective</td>
</tr>
<tr>
<td>3</td>
<td>Two-way, two color, 4 x 2 in (100 mm x 50 mm), reflective</td>
</tr>
<tr>
<td>4</td>
<td>Round white, yellow or black ceramic, non reflective</td>
</tr>
</tbody>
</table>
Section 919—Raised Pavement Markers

<table>
<thead>
<tr>
<th></th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>5</td>
<td>Oval white, yellow or black ceramic, non-reflective</td>
</tr>
<tr>
<td>6</td>
<td>Oval white or yellow ceramic, reflective</td>
</tr>
<tr>
<td>7</td>
<td>White or yellow ceramic jiggle bar, non-reflective</td>
</tr>
<tr>
<td>8</td>
<td>White or yellow ceramic jiggle bar, reflective</td>
</tr>
<tr>
<td>9</td>
<td>White or yellow channel, non-reflective</td>
</tr>
<tr>
<td>10</td>
<td>White or yellow channel, reflective</td>
</tr>
<tr>
<td>11</td>
<td>Two-way, one-color, 4 x 4 in (100 mm x 100 mm), reflective</td>
</tr>
<tr>
<td>12</td>
<td>One-way, one color, 4 x 4 in (100 mm x 100 mm), reflective</td>
</tr>
<tr>
<td>13</td>
<td>Two-way, two color, 4 x 4 in (100 mm x 100 mm), reflective</td>
</tr>
<tr>
<td>14</td>
<td>Two-way, one color, flexible reflective</td>
</tr>
<tr>
<td>15</td>
<td>One-way, one color, flexible reflective</td>
</tr>
</tbody>
</table>

3. Certification
   Submit a certification to the Engineer from the manufacturer showing the physical properties of the markers and their conformance to this Specification.

4. Packaging
   Pack shipments in containers that are acceptable to common carriers.
   a. Pack the containers to ensure delivery in perfect condition.
   b. Clearly mark each package of pavement markers with the size, color, type, and lot number.
   c. You are liable to replace any damaged shipments.

919.2.01 Raised Retro-Reflective Pavement Markers (Type 1, 2, 3, 11, 12, and 13)

A. Requirements
   1. Use raised retro-reflective pavement makers that meets the requirements of ASTM D 4280, designation H.
   2. Use raised retro-reflective pavement makers as listed in QPL 76.
   3. Use raised retro reflective pavement markers that have been evaluated by the National Transportation Product Evaluation Panel (NTPEP) test facility or other approved test facility.

B. Fabrication
   General Provisions 101 through 150

C. Acceptance
   The Department will give conditional approval to retro reflective pavement markers evaluated by the National Transportation Product Evaluation Program (NTPEP), the Georgia Department of Transportation, or other Department-approved test facilities and place them on QPL 76.
   All white and yellow retro reflective pavement markers must meet the requirements of this Specification and the following field performance requirement.
   a. Conditional QPL Placement: The Department may add markers on a conditional basis to QPL 76. These markers must maintain an average coefficient of luminous intensity for 12 months of not less than 25% of the values shown in Table 1 of ASTM D 4280.
   b. Final Acceptance or Rejection: The Department will accept or reject markers based on the marker maintaining an average coefficient of luminous intensity of 0.2 cd/ftc for 24 months.

919.2.02 Flexible Reflective Markers (Type 14 and 15)

A. Requirements
   Use markers manufactured by extruding plastic into an “L” shape, with nominal dimensions of 4 in (100 mm) long x 2 in (50 mm) high (vertical face) x 1 in (25 mm) wide (base leg). Ensure that the markers have the following:
Section 919—Raised Pavement Markers

- A pressure-sensitive adhesive with a paper release liner to the bottom of the base leg.
- Strips of metallized acrylic reflective sheeting on either one or both sides of the vertical face.
- A clear plastic cover to protect the reflective strip. Ensure that the cover withstands a chip-seal operation and is easily removed after the operation.

1. Hardness
   a. Select five random markers
   b. Use ASTM D 2240 to determine the Shore A hardness
   c. The Department will reject markers whose body and clear protective cover hardness is less than 80.

B. Fabrication

   General Provisions 101 through 150.

919.2.03 Ceramic Pavement Markers (Type 4, 5, 6, 7, and 8)

A. Requirements
   1. Use ceramic pavement markers made from a heat-fired, white, vitreous, ceramic base and a heat fired, opaque, glazed surface to produce the properties required in these Specifications.
      a. Do not place glaze on the marker bottom where it connects to the road surface.
      b. Thoroughly and evenly mature the markers. Ensure that they have no defects that affect appearance and serviceability.
      c. Use reflective ceramic markers that meet the specific intensity of each reflective surface according to Table 1 of ASTM D 4280.
      d. Ensure that the mean thickness of the glazed surface is at least 0.005 in (0.13 mm) when measured at least 0.25 in (6 mm) from the edge of the marker.
      e. Ensure that the water absorption of the ceramic markers does not exceed 2 percent of the original dry weight when tested according to ASTM C 373.
      f. Ensure that the glazed surface does not craze, spoil, or peel when passed through one cycle of the Autoclave test at 250 psi (1724 kPa) (ASTM C 424).
   2. Use the designated colors for the white and yellow markers.
      a. Ensure that the colors are uniform.
      b. Ensure that black matches Federal Color No. 595-27038.
      c. Determine the color by visually comparing each marker with calibrated standards having CIE Chromaticity Coordinate limits. Determine the limits with Federal methods of test TT-T-141, Method 4252, using a rectangle with the following corner points:

<table>
<thead>
<tr>
<th></th>
<th>1</th>
<th>2</th>
<th>3</th>
<th>4</th>
<th>(90MGO)</th>
</tr>
</thead>
<tbody>
<tr>
<td>White</td>
<td>.290</td>
<td>.310</td>
<td>.296</td>
<td>.330</td>
<td>.320</td>
</tr>
<tr>
<td>Yellow</td>
<td>.435</td>
<td>.445</td>
<td>.435</td>
<td>.544</td>
<td>.456</td>
</tr>
</tbody>
</table>

B. Fabrication

   General Provisions 101 through 150.

C. Acceptance
   1. Use a random sample of five markers for lens impact strength, temperature cycling and compressive strength tests specified in ASTM D 4280.
   2. Use the following table to determine if the markers pass the tests.

<table>
<thead>
<tr>
<th>Markers that Pass</th>
<th>Department Action</th>
</tr>
</thead>
<tbody>
<tr>
<td>5 of 5</td>
<td>Accept the lot.</td>
</tr>
<tr>
<td>3 or less of 5</td>
<td>Reject the lot; no resample allowed.</td>
</tr>
</tbody>
</table>
4 of 5

<table>
<thead>
<tr>
<th>20 of 25 retested</th>
<th>19 or less of 25 retested</th>
</tr>
</thead>
<tbody>
<tr>
<td>Accept the lot.</td>
<td>Reject the lot; no resample allowed.</td>
</tr>
</tbody>
</table>

3. Compressive Strength Test
   a. The markers pass if the average compressive load of all five markers is at least 1,500 psi (6.7 kN). No individual marker shall be less than 1,200 psi (5.3 kN).

D. Materials Warranty
   General Provisions 101 through 150.

919.2.04 Channel Pavement Markers (Type 9 and 10)

A. Requirements
   1. Use channel pavement markers made of either a heat-fired, white, vitreous, ceramic base with a heat-fired, opaque, glazed surface, or a 9 gauge (3.9 mm) steel body with a heat-fired porcelain finish.
      a. Ensure both ceramic and steel channel markers have no defects that affect appearance and serviceability.
      b. Ensure that the mean thickness of the glazed surface of ceramic channel markers is at least 0.005 in (0.13 mm) when measured at least 0.25 in (6 mm) from the edge of the marker.
      c. Ensure that mean thickness of the porcelain finish on the steel channel markers is at least 0.030 in (0.76 mm).
      d. Ensure that the water absorption of the ceramic markers does not exceed 2.0 percent of the original dry weight when tested according to ASTM C 373.
      e. Ensure that the surface of the markers do not craze, spoil, or peel when passed through one cycle of the Autoclave test at 250 psi (1724 kPa) (ASTM C 424).
   2. Use the designated colors for the white and yellow markers.
      a. Ensure that the colors are uniform.
      b. Determine the color by visually comparing them with calibrated standards having CIE Chromaticity Coordinate limits. Determine the limits with Federal methods of test TT-T-141, Method 4252, using a rectangle with the following corner points:

      | (90MGO) |
      |---------|
      | 1 | 2 | 3 | 4 |        |
      |---|---|---|---|---------|
      |   |   |   |   | 80 min. |
      |   |   |   |   |        |
      | 1 | .290 | .316 | .310 | .296 | .330 | .320 | .310 | .344 | 80 min. |

B. Fabrication
   General Provisions 101 through 150.

C. Acceptance
   1. Ensure that Type 10 markers meet the specific intensity of each reflective surface according to Table 1 in ASTM D 4280.
   2. Use a random sample of five markers for lens impact strength, temperature cycling and compressive strength tests specified in ASTM D 4280.
   3. Select two of the five markers and subject them to all the required tests.
   4. Use the following table to determine if the markers pass the tests.

<table>
<thead>
<tr>
<th>Markers that Pass</th>
<th>Department Action</th>
</tr>
</thead>
<tbody>
<tr>
<td>2 of 2</td>
<td>Accept the lot.</td>
</tr>
<tr>
<td>0 of 2</td>
<td>Reject the lot; no resample allowed.</td>
</tr>
<tr>
<td>1 of 2</td>
<td>Retest the three remaining markers.</td>
</tr>
<tr>
<td>----------------</td>
<td>-------------------------------------</td>
</tr>
<tr>
<td>3 of 3 retested</td>
<td>Accept the lot.</td>
</tr>
<tr>
<td>2 or less of 3 retested</td>
<td>Reject the lot; no resample allowed</td>
</tr>
</tbody>
</table>

D. Materials Warranty

General Provisions 101 through 150.
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  

Project Number:  CSNHS-0006-00(043)  
P.I. Number:  0006043  
Cherokee County  

Section 925—Traffic Signal Equipment  

Add the following to Subsection 925.1.01.B:  


Add the following to Subsection 925.2.15.A.1:  

Provide LED modules that are pixelated (5mm) for all circular indications, arrow indications, and hand and person pedestrian displays.  

Delete Subsection 925.2.19.A and substitute the following:  

925.2.19 Pedestrian Push Button Station  

A. Requirements  

Ensure that Pedestrian push buttons are of tamperproof construction and consist of a direct push type button and single momentary contact switch in cast aluminum housing. The pushbutton cover shall also be of cast aluminum. The housing and cover shall be free of voids, pits, dents, molding sand excessive foundry grinding marks. Exterior surface shall be smooth and cosmetically acceptable, free of molding fins, cracks and other exterior blemishes.  

Provide housing and cover with an alodine conversion coating so as to provide a proper base for paint adhesion. Finish the housing with baked enamel and paint the push button housing and Pedestrian heads highway yellow (unless otherwise specified by the Engineer).  

The assembly shall be capable of being mounted to a flat or curved surface. Ensure the assembly includes the appropriate sign as shown in the Plan Details.  

Ensure that any screws or bolts are stainless steel and vandal proof. Provide the unit with a 0.5 inch (13 mm) threaded opening with plug.  

Ensure that the assembly is weatherproof and so constructed that when properly installed, it will be impossible to receive an electrical shock under any weather condition.
Ensure that Pedestrian Pushbuttons are integrated with a sign as shown in the standard details. Provide the sign size as indicated on the Plans. GDOT will allow an adapter of cast aluminum. GDOT will allow one of three options:

1. The use of a 9 inch (229 mm) by 15 inch (381 mm) cast aluminum plate adapter to upgrade existing push button station, 9 inch (229 mm) by 12 inch (305 mm).
2. Push button station assembly 9 inch (229 mm) by 15 inch (381 mm) sign w/round pushbutton adapter.
3. The use of a 9 inch (229 mm) by 15 inch (381 mm) cast aluminum plate adapter to upgrade existing push button station, 5 inch (127 mm) by 7 inch (178 mm).

Ensure that the Pedestrian Push Button sign adapter plate is, die-cast aluminum and separate, such that it is interchangeable.

Ensure that the Pedestrian Push Button sign adapter assembly be, die-cast aluminum and attached, prior to shipping.

The plate shall be finished with highway yellow baked enamel paint (unless otherwise specified by the Engineer).

Ensure the button assembly is configured to be a mechanical switch with ball and 2 inch mushroom plunger.

Delete Subsection 925.2.34.A and substitute the following:

925.2.34 Prefabricated Controller Cabinet Base

A. Requirements

Provide controller cabinet bases that are precast polymer concrete and grey in color. Ensure the prefabricated controller cabinet base has the correct bolt pattern for the cabinet(s) to be installed. Provide prefabricated controller cabinet bases with UNC inserts as shown on plans. UNC inserts shall be stainless steel and be designed for a minimum of 15 foot-pounds of torque.

Ensure that prefabricated controller cabinet bases are designed to withstand wind loading of 125 mph (200 km/h) with the cabinets as shown in the Plans mounted. Ensure that prefabricated controller cabinet bases are designed for a minimum static vertical load of 5,000 pounds over a 10 inch (254 mm) by 10 inch (254 mm) by 1 inch (25 mm) thick distribution plate and withstand a tested load of 7,500 pounds. Ensure that prefabricated controller cabinet bases are designed for a minimum lateral load of 1800 pounds over an 18 inch (457 mm) by 24 inch (610 mm) by 1 inch (25 mm) steel plate applied to the longest side and shall withstand a tested load of 2700 pounds. The prefabricated controller cabinet base shall withstand a 50 foot-pound impact administered with a 12-pound weight having a “C” tup without puncture or splitting, in accordance with ASTM D2444. The prefabricated controller cabinet base shall meet the requirement of ASTM D543 Section 7, Procedure 1. Provide a copy of all test reports from a certified lab along with the materials certification package.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043

Cherokee County

Section 934—Rapid Setting Patching Materials for Portland Cement Concrete

Delete Section 934 and substitute the following:

934.1 General Description
This section includes the requirements for rapid setting patching materials used in Portland cement concrete.

934.1.01 Related References
A. Standard Specifications
   Section 886—Epoxy Resin Adhesives

B. Referenced Documents

<table>
<thead>
<tr>
<th>AASHTO</th>
<th>ASTM</th>
</tr>
</thead>
<tbody>
<tr>
<td>T 97</td>
<td>C 31/C31M</td>
</tr>
<tr>
<td>T 260</td>
<td>C 109/C 109M</td>
</tr>
<tr>
<td></td>
<td>C 140</td>
</tr>
<tr>
<td></td>
<td>C 666</td>
</tr>
</tbody>
</table>

Federal Hazardous Products Labeling Act

QPL-27

934.2 Materials

934.2.01 Rapid Setting Patching Materials
A. General Requirements
   1. Use rapid setting patching materials that have the following characteristics:
      • Are nonmetallic.
      • Have a color similar to Portland cement concrete.
      • Can be mixed and placed like concrete.
      • Have accelerated hardening characteristics.
      • Yield a permanent patch in concrete that can withstand traffic within 2 hours.
For a list of sources, see QPL-27.
2. **Type I**
   Use Type I to patch reinforced or nonreinforced horizontal Portland cement concrete surfaces.

3. **Type II**
   Use Type II to patch only nonreinforced horizontal Portland cement concrete surfaces.

4. **Type III**
   Use Type III to patch reinforced vertical or overhead Portland cement concrete surfaces.

5. Classify Type I, Type II, and Type III as follows:
   a. Class A, Premixed: Use these materials as received by adding water or an activator solution, according to the manufacturer’s instructions.
      
      **NOTE: DO NOT add extra aggregate to Class A patching material without approval from the Office of Materials and Research.**
   
      b. Class B, Non-Premixed: These materials contain no aggregate. Add aggregate (fine and/or coarse) according to the manufacturer’s recommendations.

6. **Type IV**
   Use elastomeric patching material to patch nonreinforced horizontal Portland cement concrete surfaces.

7. **Type V**
   Use two-component cross linked resins to patch nonreinforced horizontal Portland cement concrete surfaces.

8. **Type VI**
   Use cementitious, rapid setting, structural repair mortar to patch nonreinforced horizontal Portland cement concrete surfaces.

**B. Physical Requirements**

1. Use Type I, Type II, and Type III patching materials that have been evaluated by the National Transportation Product Evaluation Program (NTPEP) or other approved test facility, and meet the following requirements.

<table>
<thead>
<tr>
<th>Requirement</th>
<th>Measurement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Flow of Mortar, Type I and Type II</td>
<td>100% minimum</td>
</tr>
<tr>
<td>Type III</td>
<td>80% to 100%</td>
</tr>
<tr>
<td>Flexural Strength, Minimum</td>
<td>500 psi (3.5 MPa) in 24 hours</td>
</tr>
<tr>
<td>Compression Strength, Minimum</td>
<td></td>
</tr>
<tr>
<td>2 hours</td>
<td>1,200 psi (8.5 MPa)</td>
</tr>
<tr>
<td>24 hours</td>
<td>3,000 psi (20 MPa)</td>
</tr>
<tr>
<td>7 days (moisture cure)</td>
<td>4,000 psi (27.5 MPa)</td>
</tr>
<tr>
<td>Absorption, Maximum</td>
<td>10%</td>
</tr>
<tr>
<td>Shear Bond, Minimum</td>
<td>200 psi (1.5 MPa) in 24 hours</td>
</tr>
<tr>
<td>Freeze Thaw Durability Factor</td>
<td>75% of the reference concrete after 300 freeze-thaw cycles</td>
</tr>
<tr>
<td>Total Chlorides</td>
<td></td>
</tr>
<tr>
<td>Type I and Type III</td>
<td>0.6 lb/yd³ (0.4 kg/m³) maximum</td>
</tr>
<tr>
<td>Type II</td>
<td>No limits</td>
</tr>
</tbody>
</table>
C. Fabrication

1. Packaging
   a. Package this material in strong, moisture-proof paper bags or other suitable containers that can withstand shipping, normal handling, and storage without breaking.
   b. Clearly label each container of the components of a patching system with the following information:
      • Component designation, if two components.
      • Manufacturer’s batch number.
      • Mixing ratio and directions.
      • Potential hazards and precautions displayed according to the Federal Hazardous Products Labeling Act.

D. Acceptance

1. Follow the mixing instructions of the manufacturer to create test specimens.
2. Air-cure all test specimens except for the 7-day moisture cure cubes.
3. Test Types I, II, and III using the following methods:

<table>
<thead>
<tr>
<th>Test</th>
<th>Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Flow of mortar</td>
<td>ASTM C 230</td>
</tr>
<tr>
<td>Flexural strength</td>
<td>AASHTO T 97</td>
</tr>
<tr>
<td>Compressive strength</td>
<td>ASTM C 109/C 109M or C 31/C 31M, whichever is applicable</td>
</tr>
<tr>
<td>Absorption</td>
<td>ASTM C 140</td>
</tr>
<tr>
<td>Shear bond strength</td>
<td>See Subsection 934.2.01.C, “Acceptance”, Step 4</td>
</tr>
<tr>
<td>Rapid freeze thaw</td>
<td>ASTM C 666</td>
</tr>
<tr>
<td>Total chlorides</td>
<td>AASHTO T 260</td>
</tr>
</tbody>
</table>

4. Shear Bond Strength
   a. Place a Type II epoxy resin adhesive meeting the requirements of Section 886 on the surface of a cured mortar bar 16 x 3 x 3 in (400 x 75 x 75 mm).
   b. Cast a 16 x 2 x 0.5 in (400 x 50 x 13 mm) rapid-setting material patch in the center of the mortar base.
   c. Air-cure the test sample for 24 hours.
   d. Saw the mortar bar base and the cured rapid setting material patch into 2 in (50 mm) segments for testing.
   e. Use a holding device and plunger to apply a load at a rate of 0.05 in (1.3 mm) per minute to the patch until the patch fails.
   f. Read the load in pounds (newtons) on the plunger.
   g. Calculate the shear bond strength in pounds per square inch by dividing the load in pounds by the interfacial area of the patch in square inches. Calculate the metric equivalent for shear bond strength in MPa by dividing the load in newtons by gravitational acceleration (9.81 m/s²).

5. Use Type IV, Type V and Type VI patching materials that have been evaluated by NTPEP, and received a subjective field rating of ≥ 4 on an ascending scale from 1 to 5.
6. The Department will reject a patching system that meets all the requirements of this Specification, but does not work as required in actual use.

E. Materials Warranty

Ensure that the material has a minimum storage life of at least 1 year under conditions of 40° to 90° F (4° to 32° C) and maximum relative humidity of 90 percent.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

Project Number: CSNHS-0006-00 (043)
P.I. Number: 0006043
Cherokee County

SPECIAL PROVISION

Add the following:

ESCROW BID DOCUMENTATION

Scope and Purpose

The purpose of this specification is to preserve the bid documents of the successful bidder (Contractor) for use by the parties in any claims or litigation between the Department and Contractor arising out of this contract. The Contractor shall submit a legible copy of bid documentation used to prepare the bid for this contract to the Department. Such documentation shall be placed in escrow with a banking institution or other bonded document storage facility and preserved by that institution/facility as specified in the following sections of this specification.

Bid Documentation

The term "bid documentation" as used in this specification means all writings, working papers, computer printouts, charts, and all other data compilations which contain or reflect information, data, and calculations used by the Contractor to determine the bid in bidding for this project. The term "bid documentation" includes, but is not limited to, Contractor equipment rates, Contractor overhead rates, labor rates, efficiency or productivity factors, arithmetic extensions, and quotations from subcontractors and material suppliers to the extent that such rates and quotations were used by the Contractor in formulating and determining the amount of the bid. The term "bid documentation" also includes any manuals which are standard to the industry used by the Contractor in determining the bid for this project. Such manuals may be included in the bid documentation by reference. Such reference shall include the name and date of the Publication and the Publisher. The term does not include bid documents provided by the Department for use by the Contractor in bidding on this project.

Submittal of Bid Documentation

The Contractor shall submit the bid documentation to the Department in a container suitable for sealing, no later than ten calendar days following award of the Contract by the Department. The Department will not issue a Notice to Proceed until the acceptable documentation has been received. The container shall be clearly marked "Bid Documentation" and shall also show on the face of the container the Contractor's name, the date of submittal, the Project Number, the P.I. Number, the Contract Number, and the County.

Affidavit

In addition to the bid documentation, an affidavit, signed under oath by an individual authorized by the Contractor to execute bidding proposals shall be included. The affidavit shall list each bid document with sufficient specificity so
a comparison may be made between the list and the bid documentation to ensure that all of the bid documentation listed in the affidavit has been enclosed. The affidavit shall attest that the affiant has personally examined the bid documentation, that the affidavit lists all of the documents used by the Contractor to determine the bid for this project, and that all such bid documentation has been included.

**Verification**

Upon receipt of the bid documentation authorized representatives of the Department and the Contractor will verify the accuracy and completeness of the bid documentation compared to the affidavit. Should a discrepancy exist the Contractor shall immediately furnish the Department with any other needed total documentation. The Department, upon determining that the bid documentation is complete, will, in the presence of the Contractor's representative, immediately place the complete documentation and affidavit in the container and seal it. Both parties will deliver the sealed container to a banking institution or other bonded document storage facility selected by the Department for placement in a safety deposit box, vault or other secure accommodation.

**Duration and Use**

The bid documentation and affidavit shall remain in escrow during the life of the Contract or until such time as the Contractor notifies the Department of his intention to file a claim or his initiation of litigation against the Department related to the Contract. Notification of the Contractor's intention to file a claim or litigation against the Department shall be sufficient evidence for the Department to obtain the release and custody of the bid documentation. If no such notification is received and the Contractor has signed the final Standard Release Form, the Department shall instruct the banking institution or other bonded document storage facility to release the sealed container to the Contractor.

The Contractor agrees that the sealed container placed in escrow contains all of the bid documentation used to determine the bid and that no other bid documentation shall be utilized by the Contractor in litigation over claims brought by the Contractor arising out of this contract.

**Refusal or Failure to Provide Bid Documentation**

Failure or refusal to provide bid documentation shall be deemed either:

1. Failure to execute the Contract if the Contract has not yet been executed or,
2. Material breach of the Contract if the Contract has been executed.

Should the Contractor fail to execute the Contract as stated in 1 above, the Department will retain the bid bond. Refusal of the Contractor to provide adequate documentation after execution of the Contract will be considered material breach of the Contract and the Contractor will be declared in default of the Contract. The Department may, at its option terminate the contract for default. These remedies are not exclusive and the Department may take such other action as is available to it under the law.

**Confidentiality of Bid Documentation**

The bid documentation and affidavit in escrow are, and will remain, the property of the Contractor. The Department has no interest in, or right to, the bid documentation and affidavit other than to verify the contents and legibility of the bid documentation unless notification of the intention to file claim is received or litigation ensues between the Department and Contractor. In the event of such notification or litigation, the bid documentation and affidavit shall become the property of the Department.

**Cost and Escrow Instructions**

The cost of the escrow will be borne by the Department. The Department will provide escrow instructions to the banking institution or other bonded document storage facility consistent with this specification.

**Escrow Agreement**

A copy of the Escrow Agreement the successful bidder will be required to sign is attached. The successful bidder (contractor) agrees that they will sign the Escrow Agreement. Should the Contractor fail to sign the Escrow Agreement, when presented, the Department will retain the bid bond. If the Contract has been executed, and the Contractor fails to sign the Escrow Agreement, the Contractor may be declared in default of the Contract.

**Payment**

There will be no separate payment for compilation of the data, container or cost of verification of the bid documentation. All costs shall be included in the overall Contract bid price.
Escrow Agreement
For
Bid Documents

THIS AGREEMENT is made and entered into this _____ day of ________, 20____, by and among the Department of Transportation, an agency of the State of Georgia, hereinafter called the "DEPARTMENT"; ________________________; hereinafter called the "CONTRACTOR"; and ________________________, hereinafter called the "ESCROW AGENT".

WHEREAS, the DEPARTMENT awarded a project on ________________________, 20__, based on a bid proposal submitted by the CONTRACTOR, hereinafter called the "PROPOSAL", for the construction of Project Number ______________________, County(ies), Georgia, hereinafter called the "PROJECT", pursuant to which the CONTRACTOR shall cause the work therein to be constructed; and

WHEREAS, the DEPARTMENT and CONTRACTOR are desirous of entering into an Escrow Agreement, to provide for specific contingencies governing the escrow and control of the PROPOSAL bid documentation; hereinafter called "BID DOCUMENTS"; and

WHEREAS, the DEPARTMENT and CONTRACTOR desire the ESCROW AGENT to hold the BID DOCUMENTS of the CONTRACTOR;

NOW THEREFORE, for and in consideration of the mutual covenants contained herein, it is agreed by and between the parties hereto that:

ARTICLE I
ESCROW BID DOCUMENTATION

The parties hereto agree to the establishment of Escrow of the BID DOCUMENTS for the PROPOSAL pursuant to the Specifications, Supplemental Specifications, or Special Provisions pertaining to construction under the contract. It is the understanding of the parties hereto that the DEPARTMENT shall pay the ESCROW AGENT, as determined by separate agreement, for the escrow of the BID DOCUMENTS submitted to the ESCROW AGENT under the terms of this Agreement.

ARTICLE II
ACKNOWLEDGMENT

By its signature below, the ESCROW AGENT hereby acknowledges receipt from the DEPARTMENT and CONTRACTOR of a sealed container bearing the CONTRACTOR'S name, address and PROJECT Number assigned by the DEPARTMENT and containing, as specified by the affidavit of the CONTRACTOR, the PROPOSAL BID DOCUMENTS for the aforementioned PROJECT.

ARTICLE III
DEPOSIT OF BID DOCUMENTS

The PROPOSAL BID DOCUMENTS shall remain on deposit with the ESCROW AGENT until those conditions of release, as specified in ARTICLE IV, RELEASE FROM ESCROW, are met. As long as the BID DOCUMENTS remain in escrow with the ESCROW AGENT, the ESCROW AGENT shall not allow any person access, to gain possession, or to in any way interfere with the sealed BID DOCUMENT container.

ARTICLE IV
RELEASE FROM ESCROW

Upon being presented, by the DEPARTMENT, with a CONTRACTOR signed final Standard Release Form for the contract for the PROJECT, the ESCROW AGENT shall deliver to the CONTRACTOR the sealed container bearing the CONTRACTOR'S name and address and project number on it. The ESCROW AGENT is also authorized to release the BID DOCUMENT sealed container to the DEPARTMENT without the CONTRACTOR'S signed consent subject to the following conditions:

1. The CONTRACTOR has provided written notification to the DEPARTMENT of the CONTRACTOR'S intention to file a claim related to the contract for the PROJECT; or

2. The CONTRACTOR has initiated litigation against the DEPARTMENT relating to the contract for the PROJECT.

Prior to any release from escrow to the DEPARTMENT the ESCROW AGENT shall verify that either condition of release to the DEPARTMENT, as stated above, has been met by providing written notice to the CONTRACTOR of the ESCROW AGENT'S intention to release the PROPOSAL BID DOCUMENTS to the DEPARTMENT. Such written notice from the ESCROW AGENT shall be sent by certified mail no less than ten (10) calendar days prior to release to the DEPARTMENT. Upon any release from escrow of the PROPOSAL BID DOCUMENT container the ESCROW AGENT shall cause the execution of Exhibit A, Escrow Release for
PROPOSAL BID DOCUMENTS, as attached hereto and incorporated herein as if fully contained, by the party receiving the BID DOCUMENT container.

**ARTICLE V**
**INDEMNITY**

The CONTRACTOR agrees to indemnify and hold the ESCROW AGENT harmless against any loss, claim, damage, liability or expenses incurred in connection with any action, suit, proceeding, claim or alleged liability arising from this Escrow Agreement, provided, however, that the ESCROW AGENT shall not be so indemnified or held harmless for its negligence or acts of bad faith by it or any of its agents or employees.

**ARTICLE VI**
**NOTICES**

All notices and other communication shall be in writing and shall be deemed to have been duly given and delivered if mailed by certified mail, return receipt requested, postage prepaid to the addresses stated herein:

**DEPARTMENT:**

Georgia Department of Transportation

ATTN: Treasurer

600 West Peachtree Street

Atlanta, Georgia 30308

**CONTRACTOR:**

**ESCROW AGENT:**

**ARTICLE VII**
**DUTIES OF ESCROW AGENT**

The duties and responsibilities of the ESCROW AGENT shall be limited to those expressly set forth herein and the ESCROW AGENT shall act only in accordance with this ESCROW Agreement.
Notwithstanding specific provisions hereunder, the ESCROW AGENT shall at all times act upon and in accordance with the joint written instructions of the DEPARTMENT and CONTRACTOR.

ARTICLE VIII.
LAWS
This Escrow Agreement shall be deemed to have been executed in Fulton County, Georgia and the laws of the State of Georgia shall apply.

ARTICLE IX
ASSIGNMENT
This Escrow Agreement shall not be assigned without the written consent of all the parties hereto.

ARTICLE X
SURVIVAL OF CONTRACT
Except as may be expressly modified, all terms and conditions of this Escrow Agreement remain in full force and effect. The establishment of this Escrow Agreement is limited solely by the contingency of release of the PROPOSAL BID DOCUMENTS by the CONTRACTOR to the DEPARTMENT, as established by Article IV, Release From Escrow. Nothing contained herein shall alter the rights of the parties hereto.

The covenants herein contained shall, except as otherwise provided, accrue to the benefit of and be binding upon the successors and assigns of the parties hereto.
IN WITNESS WHEREOF, the parties hereunto set their hands and seals the day above first written.

CONTRACTOR:      ESCROW AGENT:

BY:                BY:  

(SEAL)            (SEAL)

TITLE:            TITLE:  

WITNESS            WITNESS

DEPARTMENT OF TRANSPORTATION

BY:  

__________________________

TITLE:  STATE TRANSPORTATION OFFICE ENGINEER

__________________________

WITNESS

ESCROW CONTAINER SEAL NUMBERS:
Exhibit A

ESCROW RELEASE
OF
BID DOCUMENTS

This is to certify that on this ___________ day of __________, 20__, the sealed container identified as:

“Bid Documentation”

CONTRACTOR:

PROJECT NUMBER:

P.I. NUMBER:

CONTRACT NUMBER:

DATE OF SUBMITTAL:

(Evidence by Agreement dated ______________).

was released from escrow and personally handed to the below named individual acknowledging receipt, representing the CONTRACTOR/DEPARTMENT, by the ESCROW AGENT upon the presentation of the required documentation pursuant to Article IV, Release from Escrow, of the agreement dated ______________. 20__, a copy of such documentation is attached hereto.

Acknowledgment of Receipt:

Acknowledgment of Release:

_____________________________________________________________________________________

ESCROW AGENT

ESCROW CONTAINER SEAL NUMBERS:
AFFIDAVIT

STATE OF GEORGIA
COUNTY OF FULTON

COMES NOW __________________________, ____________ who, after having been duly sworn, on oath, state and depose as follows:

1.

This Affidavit is based upon the personal knowledge of the Affiant.

2.

__________________________ submitted a bid on Georgia Department of Transportation Project ____________________________, ______________ COUNTY(IES) which bid was the low, responsive bid, and a Contract has been entered into between __________________ and the Georgia Department of Transportation, known as Contract No. ____________________________.

3.

This Affidavit is given in compliance with the special provision entitled “ESCROW BID DOCUMENTATION” forming part of the Contract Documents of Contract No. ____________________________.

4.

The Affiant attests that, in his capacity for ____________________________, he is personally aware the “Bid Documentation” which was used by the Company in determining, formulating, and submitting the bid on Project No. ____________________________, ______________ COUNTY(IES).

5.

The Affiant further states that he has examined the bid documentation which has been placed in a sealed container marked “Bid Documentation”, and that all such Bid Documentation utilized by the Company in determining, formulating, and submitting its bid is contained in the sealed container so marked.

6.

Each bid document contained in the sealed container is separately listed on Exhibit A, which is attached hereto and incorporated herein as fully as if included in this Affidavit at this paragraph 6.
Further Affiant sayeth not.

________________________
(Company Name)

By: ______________________________

________________________
(Name)

Its: ______________________________

________________________
(Title)

Sworn to and subscribed before me this ________ day of _______________________, 20_____.

________________________
NOTARY PUBLIC

My Commission expires:________________________
MEMORANDUM OF UNDERSTANDING

between the
Georgia Department of Transportation (hereafter the DEPARTMENT)

Atlanta Gas Light (hereafter the OWNER)

Whereas the DEPARTMENT proposes to undertake a design-build project to construct a new interchange on I-575 @ Rope Mill Connector within the existing right of way, in the City of Woodstock, Cherokee County, Georgia by contract through competitive bidding procedures; and,

Whereas the DEPARTMENT will accomplish the design-build project through a Design Consultant, Design Consultant Team and/or Contractor hereafter referred to as CONTRACTOR; and,

Whereas the OWNER has the following utility facilities which may need to be adjusted or relocated as a result of the proposed contract:

Type of facility or facilities of the OWNER:

[ ] Domestic water mains and distribution lines and associated appurtenances.

[ ] Sanitary Sewer facilities and/or Storm Drainage System

[ ] Electrical Distribution (overhead and underground) wires, poles, etc.

[ ] Electrical Transmission (overhead and underground) wires, poles, etc.

[✓] Natural Gas Distribution Facilities (underground)

[ ] Telecommunications facilities and equipment

[ ] Cable TV facilities.

[ ] Street Lighting

[ ] Internet Data Service

Whereas the OWNER desires the following to be installed as new additional facilities during the proposed contract.

Insert detailed description of proposed new additional utility installations:

None
Design Responsibilities for adjusted, relocated, and new additional utility facilities:

______ (1a) Whereas the OWNER does not have adequate staff to perform the design functions for the adjustment or relocation of its facilities and is willing to have this design work included in the contract to be let by the DEPARTMENT.

______ (2a) Whereas the OWNER is willing to have this design work included in the contract to be let by the DEPARTMENT.

✓ (3a) Whereas the OWNER will perform its own relocation or adjustment design.

If method (1a) or (2a) for the adjustment, relocation or installation of new facilities is selected, all design necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

_____ None.

Excluded Items:

Construction Responsibilities for adjusted, relocated, and new additional utility facilities:

______ (1b) Whereas the OWNER does not have adequate equipment and staff to adjust its facilities and is willing to have this work included in the contract to be let by the DEPARTMENT.

______ (2b) Whereas the OWNER is willing to have this work included in the contract to be let by the DEPARTMENT.

✓ (3b) Whereas the OWNER will perform its own relocations or adjustments.

If method (1b) or (2b) for the adjustment, relocation or installation of new facilities is selected, all work necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

_____ None.
The following is hereby mutually agreed to and understood by both parties:

1. The identification of existing facilities including preparation of base maps plans will be accomplished by the DEPARTMENT prior to award of the project contract and thereafter supplemented by the CONTRACTOR.

2. The CONTRACTOR shall coordinate reviews of the information and obtain acceptance from the OWNER and DEPARTMENT when required. However, the OWNER shall apply for and obtain any required permits from the DEPARTMENT and perform any final design or proprietary design needed to administer its own relocation work if the work will not be included in the project contract (list any work not included in the project in space provided above). If the preliminary plans indicate that no conflict exists, and the OWNER accepts this information, the OWNER shall provide a letter of “no conflict” to the CONTRACTOR.

3. The CONTRACTOR will research the property interest of each OWNER and present the findings to the DEPARTMENT and OWNER for approval. The CONTRACTOR will coordinate resolution of any disputed items. The plans and estimate for the utility work shall be subject to approval of both the DEPARTMENT and the OWNER prior to construction. If the OWNER chooses to include the subject utility work in the project construction and the research indicates that no property interest exists, and the OWNER cannot refute this finding with evidence that would substantiate the property interest in legal proceedings, the OWNER shall provide confirmation in writing that OWNER will reimburse the DEPARTMENT for any adjustment or relocations necessary; and an agreement will be prepared and executed in accordance with the DEPARTMENT’S “Utility Accommodation Policy and Standards Manual”. If the OWNER chooses to perform its own relocations and the OWNER holds no property interest as stated above; the OWNER shall confirm in writing that the OWNER will relocate its own facilities at no cost to the DEPARTMENT and the CONTRACTOR.

4. All construction engineering and contract supervision shall be the responsibility of the DEPARTMENT and the CONTRACTOR to assure that all utility work included in the project contract is accomplished in accordance with plans and specifications and to consult with the OWNER before authorizing any changes or deviations which affect the OWNER’s facility.

5. For Utility work included in the contract, the OWNER or the OWNER’s Consultant shall have the right to visit and inspect the work at any time and advise the CONTRACTOR and the DEPARTMENT’S Engineer of any observed discrepancies or potential problems. The DEPARTMENT agrees to notify the OWNER when all utility work is completed and ready for final inspection by the OWNER.

6. Upon completion of the utility work included in the contract and upon certification by the DEPARTMENT’S Engineer that the work has been completed in accordance with the plans and specifications, the OWNER will accept the adjusted, relocated, and additional facilities and will thereafter operate and maintain said facilities located within public right of way.
subject to the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect without further cost to the DEPARTMENT or it’s CONTRACTOR.

7. For the purpose of utility coordination, relocation and reimbursement matters, the OWNER shall cooperate with the CONTRACTOR in the same manner as if directly with the DEPARTMENT in accordance with the laws of Georgia, the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect between the DEPARTMENT and OWNER. The OWNER agrees to cooperate in good faith with the CONTRACTOR and to respond to all requests for information or meetings required to reach a resolution of any disputed items.

The Memorandum of Understanding will be incorporated into the project contract by reference or Exhibit.

APPROVED FOR THE OWNER BY:

(Signature)

Project Engineer

(Date)

APPROVED FOR THE DEPARTMENT BY:

(Signature)

STATE UTILITIES ENGINEER

(Date)
MEMORANDUM OF UNDERSTANDING

between the

Georgia Department of Transportation (hereafter the DEPARTMENT)

AT&T (hereafter the OWNER)

Whereas the DEPARTMENT proposes to undertake a design-build project to construct a new interchange on I-575 @ Rope Mill Connector within the existing right of way, in the City of Woodstock, Cherokee County, Georgia by contract through competitive bidding procedures; and,

Whereas the DEPARTMENT will accomplish the design-build project through a Design Consultant, Design Consultant Team and/or Contractor hereafter referred to as CONTRACTOR; and,

Whereas the OWNER has the following utility facilities which may need to be adjusted or relocated as a result of the proposed contract:

Type of facility or facilities of the OWNER:

- Domestic water mains and distribution lines and associated appurtenances.
- Sanitary Sewer facilities and/or Storm Drainage System
- Electrical Distribution (overhead and underground) wires, poles, etc.
- Electrical Transmission (overhead and underground) wires, poles, etc.
- Natural Gas Distribution Facilities (underground)
- Telecommunications facilities and equipment
- Cable TV facilities.
- Street Lighting
- Internet Data Service

Whereas the OWNER desires the following to be installed as new additional facilities during the proposed contract.

Insert detailed description of proposed new additional utility installations:

AT&T WILL NEED TO MAINTAIN EXISTING BURIED CABLE IN CONDUIT ALONG OLD ROPE MILL RD., AT&T MAY NEED TO ADJUST MANHOLE LIDS IF NECESSARY.
Design Responsibilities for adjusted, relocated, and new additional utility facilities:

_____ (1a) Whereas the OWNER does not have adequate staff to perform the design functions for the adjustment or relocation of its facilities and is willing to have this design work included in the contract to be let by the DEPARTMENT.

_____ (2a) Whereas the OWNER is willing to have this design work included in the contract to be let by the DEPARTMENT.

✓ (3a) Whereas the OWNER will perform its own relocation or adjustment design.

If method (1a) or (2a) for the adjustment, relocation or installation of new facilities is selected, all design necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

_____ None.

Excluded Items:

________

Constructions Responsibilities for adjusted, relocated, and new additional utility facilities:

_____ (1b) Whereas the OWNER does not have adequate equipment and staff to adjust its facilities and is willing to have this work included in the contract to be let by the DEPARTMENT.

_____ (2b) Whereas the OWNER is willing to have this work included in the contract to be let by the DEPARTMENT.

✓ (3b) Whereas the OWNER will perform its own relocations or adjustments.

If method (1b) or (2b) for the adjustment, relocation or installation of new facilities is selected, all work necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

_____ None.
The following is hereby mutually agreed to and understood by both parties:

1. The identification of existing facilities including preparation of base maps plans will be accomplished by the DEPARTMENT prior to award of the project contract and thereafter supplemented by the CONTRACTOR.

2. The CONTRACTOR shall coordinate reviews of the information and obtain acceptance from the OWNER and DEPARTMENT when required. However; the OWNER shall apply for and obtain any required permits from the DEPARTMENT and perform any final design or proprietary design needed to administer its own relocation work if the work will not be included in the project contract (list any work not included in the project in space provided above). If the preliminary plans indicate that no conflict exists, and the OWNER accepts this information, the OWNER shall provide a letter of “no conflict” to the CONTRACTOR.

3. The CONTRACTOR will research the property interest of each OWNER and present the findings to the DEPARTMENT and OWNER for approval. The CONTRACTOR will coordinate resolution of any disputed items. The plans and estimate for the utility work shall be subject to approval of both the DEPARTMENT and the OWNER prior to construction. If the OWNER chooses to include the subject utility work in the project construction and the research indicates that no property interest exists, and the OWNER cannot refute this finding with evidence that would substantiate the property interest in legal proceedings, the OWNER shall provide confirmation in writing that OWNER will reimburse the DEPARTMENT for any adjustment or relocations necessary; and an agreement will be prepared and executed in accordance with the DEPARTMENT’S “Utility Accommodation Policy and Standards Manual”. If the OWNER chooses to perform it own relocations and the OWNER holds no property interest as stated above; the OWNER shall confirm in writing that the OWNER will relocate its own facilities at no cost to the DEPARTMENT and the CONTRACTOR.

4. All construction engineering and contract supervision shall be the responsibility of the DEPARTMENT and the CONTRACTOR to assure that all utility work included in the project contract is accomplished in accordance with plans and specifications and to consult with the OWNER before authorizing any changes or deviations which affect the OWNER’s facility.

5. For Utility work included in the contract, the OWNER or the OWNER’s Consultant shall have the right to visit and inspect the work at any time and advise the CONTRACTOR and the DEPARTMENT’S Engineer of any observed discrepancies or potential problems. The DEPARTMENT agrees to notify the OWNER when all utility work is completed and ready for final inspection by the OWNER.

6. Upon completion of the utility work included in the contract and upon certification by the DEPARTMENT’S Engineer that the work has been completed in accordance with the plans and specifications, the OWNER will accept the adjusted, relocated, and additional facilities and will thereafter operate and maintain said facilities located within public right of way.
subject to the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect without further cost to the DEPARTMENT or its CONTRACTOR.

7. For the purpose of utility coordination, relocation and reimbursement matters, the OWNER shall cooperate with the CONTRACTOR in the same manner as if directly with the DEPARTMENT in accordance with the laws of Georgia, the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect between the DEPARTMENT and OWNER. The OWNER agrees to cooperate in good faith with the CONTRACTOR and to respond to all requests for information or meetings required to reach a resolution of any disputed items.

The Memorandum of Understanding will be incorporated into the project contract by reference or Exhibit.

APPROVED FOR THE OWNER BY:

[Signature] 01/15/2008
(Signature) (Date)

SPECIALIST - FACILITY DESIGN
(Title)

APPROVED FOR THE DEPARTMENT BY:

[Signature] 3.24.2008
(Signature) (Date)

STATE UTILITIES ENGINEER
MEMORANDUM OF UNDERSTANDING

between the
Georgia Department of Transportation (hereafter the DEPARTMENT)
City of Woodstock (hereafter the OWNER)

Whereas the DEPARTMENT proposes to undertake a design-build project to construct a new interchange on I-575 @ Rope Mill Connector within the existing right of way, in the City of Woodstock, Cherokee County, Georgia by contract through competitive bidding procedures; and,

Whereas the DEPARTMENT will accomplish the design-build project through a Design Consultant, Design Consultant Team and/or Contractor hereafter referred to as CONTRACTOR; and,

Whereas the OWNER has the following utility facilities which may need to be adjusted or relocated as a result of the proposed contract:

Type of facility or facilities of the OWNER:

☒ Domestic water mains and distribution lines and associated appurtenances.

☐ Sanitary Sewer facilities and/or Storm Drainage System

☐ Electrical Distribution (overhead and underground) wires, poles, etc.

☐ Electrical Transmission (overhead and underground) wires, poles, etc.

☐ Natural Gas Distribution Facilities (underground)

☐ Telecommunications facilities and equipment

☐ Cable TV facilities.

☒ Street Lighting

☐ Internet Data Service

Whereas the OWNER desires the following to be installed as new additional facilities during the proposed contract.

Insert detailed description of proposed new additional utility installations:
Design Responsibilities for adjusted, relocated, and new additional utility facilities:

_____ (1a) Whereas the OWNER does not have adequate staff to perform the design functions for the adjustment or relocation of its facilities and is willing to have this design work included in the contract to be let by the DEPARTMENT.

✓ (2a) Whereas the OWNER is willing to have this design work included in the contract to be let by the DEPARTMENT.

_____ (3a) Whereas the OWNER will perform its own relocation or adjustment design.

If method (1a) or (2a) for the adjustment, relocation or installation of new facilities is selected, all design necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

✓ None.

Excluded Items:

Construction Responsibilities for adjusted, relocated, and new additional utility facilities:

_____ (1b) Whereas the OWNER does not have adequate equipment and staff to adjust its facilities and is willing to have this work included in the contract to be let by the DEPARTMENT.

✓ (2b) Whereas the OWNER is willing to have this work included in the contract to be let by the DEPARTMENT.

_____ (3b) Whereas the OWNER will perform its own relocations or adjustments.

If method (1b) or (2b) for the adjustment, relocation or installation of new facilities is selected, all work necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

✓ None.
The following is hereby mutually agreed to and understood by both parties:

1. The identification of existing facilities including preparation of base maps plans will be accomplished by the DEPARTMENT prior to award of the project contract and thereafter supplemented by the CONTRACTOR.

2. The CONTRACTOR shall coordinate reviews of the information and obtain acceptance from the OWNER and DEPARTMENT when required. However, the OWNER shall apply for and obtain any required permits from the DEPARTMENT and perform any final design or proprietary design needed to administer its own relocation work if the work will not be included in the project contract (list any work not included in the project in space provided above). If the preliminary plans indicate that no conflict exists, and the OWNER accepts this information, the OWNER shall provide a letter of “no conflict” to the CONTRACTOR.

3. The CONTRACTOR will research the property interest of each OWNER and present the findings to the DEPARTMENT and OWNER for approval. The CONTRACTOR will coordinate resolution of any disputed items. The plans and estimate for the utility work shall be subject to approval of both the DEPARTMENT and the OWNER prior to construction. If the OWNER chooses to include the subject utility work in the project construction and the research indicates that no property interest exists, and the OWNER cannot refute this finding with evidence that would substantiate the property interest in legal proceedings, the OWNER shall provide confirmation in writing that OWNER will reimburse the DEPARTMENT for any adjustment or relocations necessary; and an agreement will be prepared and executed in accordance with the DEPARTMENT’S “Utility Accommodation Policy and Standards Manual”. If the OWNER chooses to perform its own relocations and the OWNER holds no property interest as stated above; the OWNER shall confirm in writing that the OWNER will relocate its own facilities at no cost to the DEPARTMENT and the CONTRACTOR.

4. All construction engineering and contract supervision shall be the responsibility of the DEPARTMENT and the CONTRACTOR to assure that all utility work included in the project contract is accomplished in accordance with plans and specifications and to consult with the OWNER before authorizing any changes or deviations which affect the OWNER’s facility.

5. For Utility work included in the contract, the OWNER or the OWNER’s Consultant shall have the right to visit and inspect the work at any time and advise the CONTRACTOR and the DEPARTMENT’S Engineer of any observed discrepancies or potential problems. The DEPARTMENT agrees to notify the OWNER when all utility work is completed and ready for final inspection by the OWNER.

6. Upon completion of the utility work included in the contract and upon certification by the DEPARTMENT’S Engineer that the work has been completed in accordance with the plans and specifications, the OWNER will accept the adjusted, relocated, and additional facilities and will thereafter operate and maintain said facilities located within public right of way.
subject to the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect without further cost to the DEPARTMENT or it’s CONTRACTOR.

7. For the purpose of utility coordination, relocation and reimbursement matters, the OWNER shall cooperate with the CONTRACTOR in the same manner as if directly with the DEPARTMENT in accordance with the laws of Georgia, the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect between the DEPARTMENT and OWNER. The OWNER agrees to cooperate in good faith with the CONTRACTOR and to respond to all requests for information or meetings required to reach a resolution of any disputed items.

The Memorandum of Understanding will be incorporated into the project contract by reference or Exhibit.

APPROVED FOR THE OWNER BY:

[Signature]

[Title]

APPROVED FOR THE DEPARTMENT BY:

[Signature]

STATE UTILITIES ENGINEER

2-11-08

3-24-2008
MEMORANDUM OF UNDERSTANDING

between the

Georgia Department of Transportation (hereafter the DEPARTMENT)

Comcast (hereafter the OWNER)

Whereas the DEPARTMENT proposes to undertake a design-build project to construct a new interchange on I-575 @ Rope Mill Connector within the existing right of way, in the City of Woodstock, Cherokee County, Georgia by contract through competitive bidding procedures; and,

Whereas the DEPARTMENT will accomplish the design-build project through a Design Consultant, Design Consultant Team and/or Contractor hereafter referred to as CONTRACTOR; and,

Whereas the OWNER has the following utility facilities which may need to be adjusted or relocated as a result of the proposed contract:

Type of facility or facilities of the OWNER:

- Domestic water mains and distribution lines and associated appurtenances.
- Sanitary Sewer facilities and/or Storm Drainage System
- Electrical Distribution (overhead and underground) wires, poles, etc.
- Electrical Transmission (overhead and underground) wires, poles, etc.
- Natural Gas Distribution Facilities (underground)
- Telecommunications facilities and equipment
- Cable TV facilities.
- Street Lighting
- Internet Data Service

Whereas the OWNER desires the following to be installed as new additional facilities during the proposed contract:

Insert detailed description of proposed new additional utility installations:

*Comcast has fiber optic cable within the limits of construction for this project.*
Design Responsibilities for adjusted, relocated, and new additional utility facilities:

_____(1a) Whereas the OWNER does not have adequate staff to perform the design functions for the adjustment or relocation of its facilities and is willing to have this design work included in the contract to be let by the DEPARTMENT.

_____(2a) Whereas the OWNER is willing to have this design work included in the contract to be let by the DEPARTMENT.

√(3a) Whereas the OWNER will perform its own relocation or adjustment design.

If method (1a) or (2a) for the adjustment, relocation or installation of new facilities is selected, all design necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

______None.

Excluded Items:________________________________________________

______________________________

Construction Responsibilities for adjusted, relocated, and new additional utility facilities:

_____(1b) Whereas the OWNER does not have adequate equipment and staff to adjust its facilities and is willing to have this work included in the contract to be let by the DEPARTMENT.

_____(2b) Whereas the OWNER is willing to have this work included in the contract to be let by the DEPARTMENT.

√(3b) Whereas the OWNER will perform its own relocations or adjustments.

If method (1b) or (2b) for the adjustment, relocation or installation of new facilities is selected, all work necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

______None.
The following is hereby mutually agreed to and understood by both parties:

1. The identification of existing facilities including preparation of base maps plans will be accomplished by the DEPARTMENT prior to award of the project contract and thereafter supplemented by the CONTRACTOR.

2. The CONTRACTOR shall coordinate reviews of the information and obtain acceptance from the OWNER and DEPARTMENT when required. However; the OWNER shall apply for and obtain any required permits from the DEPARTMENT and perform any final design or proprietary design needed to administer its own relocation work if the work will not be included in the project contract (list any work not included in the project in space provided above). If the preliminary plans indicate that no conflict exists, and the OWNER accepts this information, the OWNER shall provide a letter of “no conflict” to the CONTRACTOR.

3. The CONTRACTOR will research the property interest of each OWNER and present the findings to the DEPARTMENT and OWNER for approval. The CONTRACTOR will coordinate resolution of any disputed items. The plans and estimate for the utility work shall be subject to approval of both the DEPARTMENT and the OWNER prior to construction. If the OWNER chooses to include the subject utility work in the project construction and the research indicates that no property interest exists, and the OWNER cannot refute this finding with evidence that would substantiate the property interest in legal proceedings, the OWNER shall provide confirmation in writing that OWNER will reimburse the DEPARTMENT for any adjustment or relocations necessary; and an agreement will be prepared and executed in accordance with the DEPARTMENT’S “Utility Accommodation Policy and Standards Manual”. If the OWNER chooses to perform it own relocations and the OWNER holds no property interest as stated above; the OWNER shall confirm in writing that the OWNER will relocate its own facilities at no cost to the DEPARTMENT and the CONTRACTOR.

4. All construction engineering and contract supervision shall be the responsibility of the DEPARTMENT and the CONTRACTOR to assure that all utility work included in the project contract is accomplished in accordance with plans and specifications and to consult with the OWNER before authorizing any changes or deviations which affect the OWNER’s facility.

5. For Utility work included in the contract, the OWNER or the OWNER’s Consultant shall have the right to visit and inspect the work at any time and advise the CONTRACTOR and the DEPARTMENT’S Engineer of any observed discrepancies or potential problems. The DEPARTMENT agrees to notify the OWNER when all utility work is completed and ready for final inspection by the OWNER.

6. Upon completion of the utility work included in the contract and upon certification by the DEPARTMENT'S Engineer that the work has been completed in accordance with the plans and specifications, the OWNER will accept the adjusted, relocated, and additional facilities and will thereafter operate and maintain said facilities located within public right of way.
subject to the DEPARTMENT'S "Utility Accommodation Policy and Standards, 1988 edition" and any agreements in effect without further cost to the DEPARTMENT or it's CONTRACTOR.

7. For the purpose of utility coordination, relocation and reimbursement matters, the OWNER shall cooperate with the CONTRACTOR in the same manner as if directly with the DEPARTMENT in accordance with the laws of Georgia, the DEPARTMENT'S "Utility Accommodation Policy and Standards, 1988 edition" and any agreements in effect between the DEPARTMENT and OWNER. The OWNER agrees to cooperate in good faith with the CONTRACTOR and to respond to all requests for information or meetings required to reach a resolution of any disputed items.

The Memorandum of Understanding will be incorporated into the project contract by reference or Exhibit.

APPROVED FOR THE OWNER BY:

[Signature]

ENGINEER

[Title]

DATE: 3-4-2008

APPROVED FOR THE DEPARTMENT BY:

[Signature]

STATE UTILITIES ENGINEER

DATE: 3-24-2008
MEMORANDUM OF UNDERSTANDING
between the
Georgia Department of Transportation (hereafter the DEPARTMENT)
and
Georgia Power Co. (hereafter the OWNER)

Whereas the DEPARTMENT proposes to undertake a design-build project to construct a new interchange on I-575 @ Rope Mill Connector within the existing right of way, in the City of Woodstock, Cherokee County, Georgia by contract through competitive bidding procedures; and,

Whereas the DEPARTMENT will accomplish the design-build project through a Design Consultant, Design Consultant Team and/or Contractor hereafter referred to as CONTRACTOR; and,

Whereas the OWNER has the following utility facilities which may need to be adjusted or relocated as a result of the proposed contract:

Type of facility or facilities of the OWNER:

_____ Domestic water mains and distribution lines and associated appurtenances.
_____ Sanitary Sewer facilities and/or Storm Drainage System
X _____ Electrical Distribution (overhead and underground) wires, poles, etc.
_____ Electrical Transmission (overhead and underground) wires, poles, etc.
_____ Natural Gas Distribution Facilities (underground)
_____ Telecommunications facilities and equipment
_____ Cable TV facilities.
_____ Street Lighting
_____ Internet Data Service

Whereas the OWNER desires the following to be installed as new additional facilities during the proposed contract.

Insert detailed description of proposed new additional utility installations:

_____ None

Design Responsibilities for adjusted, relocated, and new additional utility facilities:
(1a) Whereas the OWNER does not have adequate staff to perform the design functions for the adjustment or relocation of its facilities and is willing to have this design work included in the contract to be let by the DEPARTMENT.

(2a) Whereas the OWNER is willing to have this design work included in the contract to be let by the DEPARTMENT.

(3a) Whereas the OWNER will perform its own relocation or adjustment design.

If method (1a) or (2a) for the adjustment, relocation or installation of new facilities is selected, all design necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

None.

Excluded Items: All GPC distribution facilities.

Construction Responsibilities for adjusted, relocated, and new additional utility facilities:

(1b) Whereas the OWNER does not have adequate equipment and staff to adjust its facilities and is willing to have this work included in the contract to be let by the DEPARTMENT.

(2b) Whereas the OWNER is willing to have this work included in the contract to be let by the DEPARTMENT.

(3b) Whereas the OWNER will perform its own relocations or adjustments.

If method (1b) or (2b) for the adjustment, relocation or installation of new facilities is selected, all work necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

None.

Excluded Items: All GPC distribution facilities.
The following is hereby mutually agreed to and understood by both parties:

1. The identification of existing facilities including preparation of base maps plans will be accomplished by the DEPARTMENT prior to award of the project contract and thereafter supplemented by the CONTRACTOR.

2. The CONTRACTOR shall coordinate reviews of the information and obtain acceptance from the OWNER and DEPARTMENT when required. However, the OWNER shall apply for and obtain any required permits from the DEPARTMENT and perform any final design or proprietary design needed to administer its own relocation work if the work will not be included in the project contract (list any work not included in the project in space provided above). If the preliminary plans indicate that no conflict exists, and the OWNER accepts this information, the OWNER shall provide a letter of “no conflict” to the Department.

3. The CONTRACTOR will research the property interest of each OWNER and present the findings to the DEPARTMENT and OWNER for approval. The CONTRACTOR will coordinate resolution of any disputed items. The plans and estimate for the utility work shall be subject to approval of both the DEPARTMENT and the OWNER prior to construction. If the OWNER chooses to include the subject utility work in the project construction and the research indicates that no property interest exists, and the OWNER cannot refute this finding with evidence that would substantiate the property interest in legal proceedings, the OWNER shall provide confirmation in writing that OWNER will reimburse the DEPARTMENT for any adjustment or relocations necessary; and an agreement will be prepared and executed in accordance with the DEPARTMENT’S “Utility Accommodation Policy and Standards Manual”. If the OWNER chooses to perform it own relocations and the OWNER holds no property interest as stated above; the OWNER shall confirm in writing that the OWNER will relocate its own facilities at no cost to the DEPARTMENT and the CONTRACTOR.

4. All construction engineering and contract supervision shall be the responsibility of the DEPARTMENT and the CONTRACTOR to assure that all utility work included in the project contract is accomplished in accordance with plans and specifications and to consult with the OWNER before authorizing any changes or deviations which affect the OWNER’s facility.

5. For Utility work included in the contract, the OWNER or the OWNER’s Consultant shall have the right to visit and inspect the work at any time and advise the CONTRACTOR and the DEPARTMENT’S Engineer of any observed discrepancies or potential problems. The DEPARTMENT agrees to notify the OWNER when all utility work is completed and ready for final inspection by the OWNER.

6. Upon completion of the utility work included in the contract and upon certification by the DEPARTMENT’S Engineer that the work has been completed in accordance with the plans and specifications, the OWNER will accept the adjusted, relocated, and additional facilities and will thereafter operate and maintain said facilities located within public right of way subject to the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect without further cost to the DEPARTMENT or it's CONTRACTOR.

7. For the purpose of utility coordination, relocation and reimbursement matters, the OWNER shall cooperate with the CONTRACTOR in the same manner as if directly with the DEPARTMENT in accordance with the
laws of Georgia, the DEPARTMENT'S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect between the DEPARTMENT and OWNER. The OWNER agrees to cooperate in good faith with the CONTRACTOR and to respond to all requests for information or meetings required to reach a resolution of any disputed items.

The Memorandum of Understanding will be incorporated into the project contract by reference or Exhibit.

APPROVED FOR THE OWNER BY:

Mark [Signature]

February 28, 2008

Date

Project Manager DOT / Joint Use

Title

APPROVED FOR THE DEPARTMENT BY:

[Signature]

3-24-2008

Date

STATE UTILITIES ENGINEER
Delete paragraph one of Subsection 103.02 and substitute the following:

If a Contract is Awarded, it will be Awarded to the lowest reliable bidder whose Proposal shall have met all the prescribed requirements. The Contract will be Awarded, if at all, within 50 calendar days after the opening of the Proposals, unless a longer period is specified in the Proposal or the successful Bidder agrees in writing a longer period for the Award.
**DEPARTMENT OF TRANSPORTATION**  
**STATE OF GEORGIA**  
**SPECIAL PROVISION**  

**PROJECT: CSNHS-0006-00(043), CHEROKEE COUNTY, PI #0006043**

Section 107 – Legal Regulations and Responsibility to the Public

Subsection 107.18, “Acquisition of Right-of-Way” is modified by the addition of the following:

<table>
<thead>
<tr>
<th>Parcel No.</th>
<th>Owner</th>
<th>Title/Date</th>
<th>Possession</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>Deer Run Homeowners Association</td>
<td>Title – April 2, 2009</td>
<td>*Jan. 15, 2010</td>
</tr>
<tr>
<td>2</td>
<td>Alfred Jr. &amp; Richard Means</td>
<td>Title – April 2, 2009</td>
<td>Nov. 13, 2009  Option</td>
</tr>
<tr>
<td>3</td>
<td>Daniel &amp; Kathleen Parodi</td>
<td>Title – April 2, 2009</td>
<td>Oct. 07, 2009  R/W Deed</td>
</tr>
<tr>
<td>4</td>
<td>Kevin Newsome</td>
<td>Title – April 2, 2009</td>
<td>*Jan. 15, 2010</td>
</tr>
<tr>
<td>5</td>
<td>Settendown Partnership</td>
<td>Title – April 2, 2009</td>
<td>Oct. 05, 2009  R/W Deed</td>
</tr>
<tr>
<td>6</td>
<td>Cody, Cody, Venable &amp; Adams</td>
<td>Title – April 2, 2009</td>
<td>Nov. 05, 2009  Esmt</td>
</tr>
<tr>
<td>7</td>
<td>Ridgewalk Holdings, LLC</td>
<td>Title – April 2, 2009</td>
<td>*Dec. 21, 2009</td>
</tr>
<tr>
<td>8</td>
<td>Ridgewalk Holdings, LLC</td>
<td>Title – April 2, 2009</td>
<td>*Dec. 21, 2009</td>
</tr>
<tr>
<td>9</td>
<td>City of Woodstock</td>
<td>Title – April 2, 2009</td>
<td>Public R/W owned by City</td>
</tr>
<tr>
<td>10</td>
<td>Meridian Development</td>
<td>Title – April 2, 2009</td>
<td>*Mar. 01, 2010</td>
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<tr>
<td>11</td>
<td>ABK &amp; TVK Investments, LLC</td>
<td>Title – April 2, 2009</td>
<td>*Feb. 15, 2010</td>
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<td>Title – April 2, 2009</td>
<td>*Feb. 15, 2010</td>
</tr>
<tr>
<td>13</td>
<td>Larry Lusk &amp; Dan Thornton</td>
<td>Title – April 2, 2009</td>
<td>*Feb. 15, 2010</td>
</tr>
<tr>
<td>14</td>
<td>Jeanine Minzey</td>
<td>Title – April 2, 2009</td>
<td>*Feb. 15, 2010</td>
</tr>
<tr>
<td>15</td>
<td>Bank of North Ga</td>
<td>Title – April 2, 2009</td>
<td>*Jan. 15, 2010</td>
</tr>
<tr>
<td>16</td>
<td>Bank of North Ga</td>
<td>Title – April 2, 2009</td>
<td>*Jan. 15, 2010</td>
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<td>18</td>
<td>Bank of North Ga</td>
<td>Title – April 2, 2009</td>
<td>*Jan. 15, 2010</td>
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<tr>
<td>19</td>
<td>Meridian Developers, LLC</td>
<td>Title – April 2, 2009</td>
<td>*Mar. 01, 2010</td>
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<td>20</td>
<td>Bank of North Ga</td>
<td>Title – April 2, 2009</td>
<td>*Jan. 15, 2010</td>
</tr>
</tbody>
</table>

*These are anticipated dates. The Contractor is to verify possession through the Department before entering upon this property for any reason.

Physical construction may proceed when authorization is granted, but the contractor will take no action that will result in unnecessary inconvenience, disproportionate injury or any action coercive in nature to occupants of residences, businesses, farms, or nonprofit organizations which have not been moved from the right of way.
Depending on construction sequencing, the lack of possession and use of the above referenced parcels could interfere with construction operations and the contractor’s ability to perform as specified.

The Contractor shall not commence work on any occupied parcel until the Department has title and possession of parcels.
Add the following to Subsection 107.23:

G. Protection of Federally Threatened and/or Endangered Species

The following conditions are intended as a minimum to protect the Cherokee darter (*Etheostoma scotti*) and its habitat during any activities involving the unnamed perennial tributary to Little River (Stream 5). This includes any soil disturbing work along the creek banks of Stream 5.

1. The Contractor shall advise all project personnel employed to work on this project about the presence, appearance, and habitat of the federally protected Cherokee darter and that there are civil and criminal penalties for harming, harassing, or killing the Cherokee darter which is protected under the Endangered Species Act of 1973. The Contractor shall ensure that all workers implementing environmental protection measures, designed to minimize take, are fully informed as to the site sensitivity and legal responsibilities of non-conformity to these measures.

2. Provisions of the erosion control plan shall be implemented to the extent possible prior to major grading operations.

3. Work shall not be allowed in Stream 5 from March 1st through June 15th.

4. Temporary erosion control devices shall be installed before any other work will be allowed to be performed.

5. Existing vegetation and forested areas will be preserved to the extent possible. The contractor will be allowed to only clear and grub within the construction limits of the project and other areas necessary for access, staging and mobility within the project area as is delineated on the plans.

6. All slopes will be covered with erosion control mats and planted with a fast-growing grass mixture, in accordance to GDOT’s standard specifications. Stockpiled materials shall be placed to prevent rain runoff from washing the materials into the creek. A double row of silt fence will be placed along the construction limits. A double row of Type C and a single row of Type A silt fence will be placed between the construction limits and the designated stream buffers prior to the commencement of clearing and grubbing.

7. The 25-foot wide vegetative buffer zone will be maintained on either side of Stream 5 for as long as possible prior to the start of construction of the proposed culvert for the crossing of Stream 5 (i.e., any required clearing of the 25-foot buffer of Stream 5 shall not be conducted until immediately before the culvert construction activities are scheduled to begin.) Orange fencing will be placed around the 25-foot buffer of Stream 5 to help prevent accidental clearing of the buffer prior to that time. All clearing and grubbing will be kept to a minimum.

8. All erosion control devices shall be closely monitored. When one-third of the capacity of any device has been reached, the device shall be immediately cleaned out. As maintenance is performed on silt fences, silt gates, slope drains, filtration ponds, and other erosion control devices, the materials removed shall be placed in such a manner to prevent these materials from entry into the creek.
9. Seepage or drips into Stream 5 shall be kept to a minimum. Immediately prior to placing any concrete for the construction of the new structure, all forms shall be positively sealed to minimize seepage or drips into the creek during construction of the culvert extension.

10. The Contractor’s worksite erosion control supervisor (WECS) shall monitor all erosion control devices on a daily basis. When a visible increase in turbidity is observed in the creek that cannot be attributed to a rainfall event or construction activity associated with the Stream 5 culvert, construction shall be stopped until the source can be determined. Immediate corrective measures shall be taken before work will be allowed to continue.

11. The Contractor will be expected to immediately modify the erosion control plan to correct any circumstances that may cause or allow pollutants from the worksite to enter Stream 5 or damage the creek's habitat.

12. Prohibit the use of borrow sites or stockpiling of dirt within 200 feet of the stream banks or elsewhere where runoff from the site would increase stream sedimentation.

13. Equipment staging areas and equipment maintenance areas (particularly for oil changes) shall be located at least 200 feet from stream banks to minimize the potential for wash water, petroleum products, or other contaminants from construction equipment entering the river.

14. In order to protect the habitat of the Cherokee darter the Contractor shall not use pesticides, herbicides (including those for Right-of –Way maintenance), or fertilizers within 200 feet of Stream 5 during and after construction activities.

The Contractor shall fabricate and erect 4 signs labeled with the lettering “ESA” (indicating Ecologically Sensitive Areas) which will advise the Department’s Maintenance crew not use chemicals (i.e., pesticides or fertilizers) in the area indicated by the signs. The Contractor shall place signs along the boundaries of the stream buffer at Stream 5 at four locations as designated on the plans. The distance from the edge of pavement will be determined according to MUTCD standards and must be within view from the road. Metal signs, which are 12 inches wide by 12 inches high, shall be fabricated for this purpose as depicted in the Sign Detail included in this package (Figure 1). The signs shall be mounted on metal posts, the bottom edge of the sign being positioned five feet high relative to the ground surface. The signs shall be in accordance with Sections 636, 910, 912, and 913 of the current Georgia Standard Specifications. The signs shall have white, reflecterized lettering and graphics with green, reflecterized background (see Sections 636, 910, 912, and 913 of the Georgia Standard Specifications).

15. In the event any incident occurs that causes harm to the Cherokee darter or that could be detrimental to the continued existence of the Cherokee darter along the project corridor, the Contractor shall report the incident immediately to the Project Engineer who in turn will notify:

   a. U.S. Fish and Wildlife Service, Athens Field Office at (706) 613-9493;
   b. Federal Highway Administration, Georgia Division at (404) 562-3630;
   c. Glenn Bowman, P.E., Georgia Department of Transportation, Office of Environment/Location at (404) 631-1101 or (404) 326-5871.

In the event of possible harm to the Cherokee darter, all activity shall cease pending Section 7 consultation by the Department with the U. S. Fish and Wildlife Service and the lead Federal Agency.

16. Following project completion, a report summarizing any incidents with the above listed species shall be submitted by the Contractor to the:

   a. the Project Engineer;
   b. U.S. Fish and Wildlife Service, 105 West Park Drive, Suite D, Athens, GA 30606;
   c. Federal Highway Administration, 61 Forsyth Street, S.W., Suite 17T100, Atlanta, GA 30303;
   d. Nongame/Endangered Wildlife Program, Georgia Department of Natural Resources, 2065 US Hwy. 278 SE, Social Circle, GA 30025 and;
   e. Georgia Department of Transportation, Office of Environmental Services, 600 West Peachtree St. NW, 16th Floor, Atlanta, GA 30308.

17. All costs pertaining to any requirement contained herein shall be included in the overall bid submitted unless such requirement is designated as a separate Pay Item in the Proposal.
A. **General Requirements**

The Contractor shall provide a project schedule using the critical path method (CPM).

The schedule shall show the various activities of work in sufficient detail to demonstrate a reasonable and workable plan to complete the work in the specified contract time.

The Contractor shall be responsible for ensuring that all work sequences are logical and that the schedule indicates a coordinated plan. The schedule shall indicate the order and interdependence of activities and the sequence for accomplishing the work.

The schedule shall include sufficient detail to allow the Engineer to readily identify the work and evaluate the progress of each activity. The schedule shall include activities for all work to be performed by the Contractor, subcontractors and suppliers. The schedule shall also include activities specific to the project to be performed by the Department, other governmental bodies, regulatory agencies, utilities and any others necessary to complete the work. The schedule shall reflect the scope of work, construction phasing, maintenance of traffic requirements, environmental requirements, utility and railroad coordination, coordination with other contractors, intermediate completion dates, the contract completion date, and any other work or milestones established in the contract. The schedule shall include activities for working and shop drawing preparation, Contractor submittals, submittal review time by the Department, material procurement and fabrication, and the delivery of materials, plant, and equipment and other similar activities.

Failure by the Contractor to include any element of work or to accurately reflect the relationships among the work activities required for performance of the Contract does not excuse the Contractor from completing all work within the specified time at no additional time or cost to the Department, notwithstanding the acceptance of the schedule by the Department.

The Contractor shall be responsible for implementing and executing the work specified in the contract in strict conformance with the CPM of record. The CPM of record shall be the Contractor’s work plan for completing the entire Contract as specified in the Contract Documents. Failure of the Contractor to adhere to the CPM of record will be cause for the Engineer to deny all requests for additional compensation or extensions in the Contract duration and may result in the Engineer withholding pay estimates.

The existence of a CPM of record including any required update(s), as stated in 108.03.D is a condition precedent to the Contractor having any right to the granting of an extension of contract time or any monetary compensation arising out of any delay. Contractor’s failure to have a CPM of record, including any required update(s) , for the period of potential impact, or in the event the CPM of record and applicable updates do not reflect the actual status of the project or fail to accurately show the true critical or non-critical activities for the period of potential
impact, will result on any entitlement determination as to time and money for such period of potential impact being limited solely to the Department’s analysis and identification of the actual critical and non-critical activities.

B. Definitions
The following definitions shall be applied to the terms used in this specification and shall not be taken to modify in any way the definitions in 101.01 through 101.83.

Activity – A discrete, identifiable task or operation that takes time, has a definable start and stop date, furthers the work’s progress, and can be used to plan, schedule, and monitor a project.

Activity Calendar – A set of days assigned to a specific activity on which work for the activity may be scheduled.

Activity Calendar Day – A day on which work is scheduled to be performed on a specific activity.

Activity Identification (ID) Number – A unique, alphanumeric, identification code assigned to a specific activity.

Activity Network Diagram – A graphic representation of a CPM schedule, including a timescale, which shows the relationships among activities.

As-built Schedule – The last monthly update schedule depicting the actual start and finish dates for each activity.

Bar Chart – A graphic representation of a schedule without relationship lines displayed. A timescale appears along the horizontal axis.

Baseline Schedule – A baseline CPM schedule showing the original plan to complete the entire project.

Calendar Day – As defined in 101.12.

Calendar ID – An alphanumeric identification code assigned to an activity calendar.

Constraint – A restriction imposed on the start or finish dates of an activity that modifies or overrides the activity’s logic relationships.

Contract Completion Date – The original date or revised date specified in the contract for completion of the project.

Controlling Activity – Activity or work item having the least amount of float, which if not completed as scheduled would result in delaying the scheduled completion date of the project.

CPM of Record – A baseline, monthly update, or a revised schedule accepted by the Department.

Critical Activity – Any activity on the critical path.

Critical Path – The longest path of activities which determines the scheduled completion date of the project.

Data Date – The first day in an Initial or a Baseline Schedule and the first day for performance of the work remaining in a Monthly Update Schedule or a Revised Schedule.

Free Float – The amount of time an activity can be delayed and not delay a successor.

Initial Schedule – A schedule showing the original plan for the first 60 calendar days of work.

Intermediate Completion Date – A date specified in the contract for completion of an interim milestone, phase or other portion of the project.

Milestone – An activity with no duration that is typically used to represent the beginning or end of the project or an interim phase. Includes, but is not limited to, intermediate completion dates and the contract completion date.

Monthly Update Schedule – A schedule produced by incorporating the project’s actual progress into the baseline schedule or a revised schedule.

Open End – The condition that exists when an activity has either no predecessor or no successor, or when an activity’s only predecessor relationship is a finish-to-finish or only successor relationship is a start-to-start.

Original Duration – The estimated time, expressed in activity calendar days, required to perform an activity.

Predecessor – An activity that is defined by schedule logic to precede another activity. A predecessor may control the start or finish date of its successor.
**Relationship** – The interdependence among activities. Relationships link an activity to predecessors and successors.

**Remaining Duration** – The estimated time, expressed in activity calendar days, required to complete an activity.

**Revised Schedule** – A revised schedule that includes a significant modification to the schedule’s logic or activity durations.

**Schedule** – A Critical Path Method (CPM) schedule showing activities organized by relationships to depict the plan for execution of a project.

**Scheduled Completion Date** – The completion date forecast by the CPM schedule. The schedule may also forecast intermediate completion dates for milestones, phases, or other portions of the project.

**Successor** – An activity that is defined by schedule logic to succeed another activity. The start or finish date of a successor may be controlled by its predecessor.

**Total Float** – The amount of time an activity can be delayed and not delay the contract completion date.

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**C. Submissions**

1. **Initial Schedule**

   The Contractor shall submit an initial schedule to the Engineer for acceptance no later than the day of the pre-construction conference. The submittal shall include a detailed schedule for the first 60 calendar days of work in accordance with 108.03.D and a summary of activities for the balance of the contract in accordance with 108.03. The schedule shall show milestones for intermediate and contract completion dates no later than those specified in the contract. All specified closure or restriction periods, non-work periods or any other time restrictions in the contract shall be addressed in the initial schedule. The initial schedule will be discussed at the pre-construction conference. No work within the project limits shall be performed until the initial schedule is submitted.

   The Engineer will provide written notice within 7 calendar days after receipt of the initial schedule that the schedule is either accepted or rejected or that additional information is requested. The Engineer’s written notice will identify the reasons for rejection or what additional information is requested. Within 7 calendar days of the Engineer’s written notice, the Contractor shall submit a corrected initial schedule that addresses the reasons for rejection and provides any additional information requested.

2. **Baseline, Monthly Update and Revised Schedules**

   The Contractor shall submit a baseline schedule in accordance with 108.03.D no later than 45 calendar days after the first day of work.

   If a baseline schedule is not submitted within the allotted time, the Engineer may order work on the project to be suspended until a baseline schedule is submitted. If the baseline schedule is not accepted within 90 days of the first day of work, the Engineer may order work on the project to be suspended until a baseline schedule is accepted. When the Engineer notifies the Contractor that the baseline schedule has been accepted, it will become the CPM of record.

   The Contractor shall submit a monthly update to the baseline schedule in accordance with 108.03.D no later than 7 calendar days after the end of each month during the contract beginning with the month after approval of the baseline schedule. The update schedule shall depict the status of the project at the end of the month just completed. The update schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work. If a monthly update schedule submittal is more than 30 days past due, the Engineer may withhold pay estimates until monthly update schedule submittals are current. When the Engineer notifies the Contractor that the update to the baseline schedule has been accepted, it will become the CPM of record for the period between its data date and the data date of the next accepted update or revision.

   The Contractor shall submit a revised schedule when any of the following occur:

   (a) A monthly update schedule projects a delay of 14 calendar days or more to an intermediate or contract completion date.

   (b) There is an actual delay in any critical activity of 7 calendar days or more.

   (c) There is a significant difference, as determined by the Engineer, between the scheduled and actual work sequence.
The Engineer will notify the Contractor in writing when a revised schedule is required due to any of the reasons listed above. The Contractor shall submit a revised schedule in accordance with 108.03.D no later than 14 calendar days after notification by the Engineer. The revised schedule shall depict the status of the project on the date of the Engineer’s notification for a revised schedule. The schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work.

The Engineer may request a revised schedule due to changes in the work. The Engineer will notify the Contractor in writing when a revised schedule is requested. Changes that may initiate a request for a revised schedule include, but are not limited to:

(a) An increase or decrease in the amount of work due to extra work or force account work.
(b) An approved increase or decrease in contract time.
(c) A Department requested change in the phasing or contract time of the project.

When a revised schedule is requested by the Engineer, the Contractor shall submit a revised schedule in accordance with 108.03.D no later than 14 calendar days after notification by the Engineer. The revised schedule shall depict the status of the project on the date of the Engineer’s request for a revised schedule. The schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work.

If a revised schedule is not submitted within 30 days of written notification, the Engineer may order work on the project to be suspended until the revised schedule is submitted. If a revised schedule is not accepted within 60 days of the written notification, the Engineer may order work on the project to be suspended until the revised schedule is accepted.

The Contractor may request to submit a revised schedule to reflect changes in the plan to perform the work. When a revised schedule is proposed by the Contractor, the Contractor shall provide the Engineer with a written notice 7 calendar days prior to submitting the revised schedule. The notice shall include a description of the changes to be made and the reason for the changes. The revised schedule shall be submitted in accordance with 108.03.D and shall depict the status of the project on the date of the Contractor’s written notice to the Engineer. The schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work.

A revised schedule submitted at the request of the Contractor will not be considered to be the CPM of record until accepted by the Engineer.

3. As-Built Schedule

The Contractor shall submit an as-built schedule in accordance with 108.03.D no later than 30 calendar days after completion of contract work. The as-built schedule shall depict the actual start and finish dates for each activity.

The as-built schedule shall contain a written certificate signed by the Contractor’s project manager stating “To my knowledge, the enclosed as-built schedule reflects the actual start and finish dates of the actual activities for the project contained herein.”

4. Acceptance of Schedules

The Engineer will provide written notice within 14 calendar days of receipt of a baseline, monthly update, revised or as-built schedule that the schedule is either accepted or rejected or that additional information is requested. The Engineer’s written notice will identify the reasons for rejection or what additional information is requested. Within 14 calendar days of the Engineer’s written notice, the Contractor shall submit a corrected baseline, monthly update, revised or as-built schedule that addresses the reasons for rejection and provides any additional information requested. If the Engineer does not respond in writing to the submittal of a schedule within the allotted time, the schedule shall be considered to be accepted.

The Engineer’s review of a submitted schedule will be for compliance with the specifications. Acceptance by the Engineer does not relieve the Contractor of responsibility for the accuracy or feasibility of the schedule. Acceptance of a schedule does not constitute a modification of the contract or endorsement or validation of the Contractor’s logic, activity durations, or assumptions in creating the schedule. Acceptance of a schedule does not guarantee that the project can be performed or completed as scheduled. Omissions and errors in a schedule shall be corrected by submittal of a revised schedule and will not entitle the Contractor to additional compensation or contract time.

5. Float and Scheduled Completion
Total float belongs to the project and shall be a shared commodity between the Contractor and the Department. Float is not for the exclusive use or benefit of either party. Either party has full use of float until it is depleted.

Float generated during the course of the project due to the efficiencies of either party shall be considered to be part of total float.

Negative float generated during the course of the project will not be a basis for requesting time extensions. Time extension requests shall be developed in accordance with Section 108.07.E.

A schedule may indicate a scheduled completion date in advance of the contract completion date. However, the Department shall not be liable in any way for the Contractor's failure to complete the project prior to the contract completion date. Any costs incurred by the Contractor as a result of such a failure shall be the responsibility of the Contractor. The Contractor will not be entitled to claim or recover any such cost from the Department.

An accepted schedule that indicates a scheduled completion date in advance of the contract completion date will be considered to have total float equal to the time between the scheduled completion date and the contract completion date.

When a contract has an incentive/disincentive date for all or a portion of the work, and the Contractor elects to show a completion date in advance of the contract completion date, the Contractor shall show an activity in the schedule for accruing incentive. The duration of this activity shall be the length of time, in calendar days, between the Contractor’s scheduled completion date and the contract completion date. The Contractor may assign the activity a duration up to and including the number of days necessary to earn the specified maximum incentive. Any time between a scheduled completion date earlier than the maximum incentive date and the maximum incentive date will be considered to be part of total float.

**D. Schedule Technical Requirements**

The schedule shall be generated using software completely compatible with MS Project 2007 or later version.

The schedule shall at a minimum include codes for project phase, location of work and responsibility for each activity. Additional coding fields may be used if approved by the Engineer.

Each schedule submittal shall include the following:

1. A letter of transmittal identifying the schedule submission.
2. A narrative report in accordance with 108.03.E
3. A compact disc (CD) containing an electronic file of the schedule in MS Project 2007 or later version format that is completely compatible with and may be directly imported into MS Project 2007 or later version without any loss or modification of data or need for any conversion or other software. Any electronic schedule file submitted by the Contractor that is not completely compatible with the Department’s MS Project 2007 or later version software will be rejected.
4. The critical path in a bar chart format created using the longest path sort, grouped (banded) by phase and sorted by early start dates.
5. A full color plot of the activity network diagram on 11 x 17 or 22 x 34 paper in landscape layout in a time scale suitable to provide sufficient detail. Other formats may be accepted with prior approval by the Engineer. Arrows shall be used to show the relationships between activities on the activity network diagram. The critical path and critical activities shall be plotted in red.

Each page of the schedule plot shall at a minimum include a column for each of the following:

- a. Activity ID
- b. Activity description
- c. Original duration
- d. Remaining duration
- e. Early start
- f. Early finish
- g. Late start
h. Late finish
i. Total Float
j. Percent Complete

Each page of the schedule plot shall include a legend with the following information:
   a. Contract number
   b. District
   c. Name of the Contractor
   d. Project location
   e. Notice to Proceed date
   f. Original contract completion date
   g. Revised contract completion date (as applicable)
   h. Data date

6. A predecessor/successor report with the following information for each activity:
   a. Activity ID and description
   b. Original duration
   c. Remaining duration
   d. Calendar ID
   e. Early start date
   f. Early finish date
   g. Late start date
   h. Late finish date
   i. Total float
   j. Relationship type
   k. Lags
   l. Constraints

Each activity shown on the CPM schedule shall meet the following requirements:

1. Each activity shall have a unique activity identification number which shall not be modified or re-assigned to a different work activities once assigned to an activity in the network logic diagram.
2. Each activity shall have a unique description specifying the work type and location.
3. Each activity description shall describe work associated with only one operation.
4. All non-procurement activities shall have an original duration not to exceed 20 calendar days unless approved by the Engineer.
5. Activities for submittal reviews by the Department shall allow reasonable durations.
6. Each activity, except the first activity, shall have at least one predecessor.
7. Each activity, except the last activity, shall have at least one successor.
8. Activity relationships shall be finish-to-start with no leads or lags, finish-to-finish or start-to-start with lags less than the predecessor’s duration.
9. The use of lags with a negative value shall not be allowed on any activity relationship type.

10. All activities shall have their start and finish tied to the logic of the schedule.

11. Activities shall not have constraints unless approved by the Engineer, with the exception of, when used for “Project Start”, and “Project Completion”.

12. Each activity shall have an activity calendar identified. A project may have more than one activity calendar, but only one activity calendar shall be applicable to each activity.

13. Seasonal weather conditions shall be considered and included in the planning of all work.

The CPM schedule shall only be calculated using retained logic. Use of Progress Override option will not be allowed. Schedule duration shall be contiguous. Total float shall be calculated based on finish dates.

**E. Narrative Reports**

The Contractor shall submit a narrative report with each schedule submission pertaining to the work identified in the schedule. For the initial and baseline schedule submittals, the narrative report shall include the following:

1. An explanation of the overall plan to complete the project, including where the work will begin and how the work and crews will progress through the project.

2. An explanation of the use and application of the workdays per week, number of shifts per day, number of hours per shift, holidays observed and how the schedule accommodates adverse weather days for each month.

3. Description of the work to be completed each season for multi-year projects.

4. A description of any problems or issues.

5. A description of any delays, including identification of any delayed activities, the type of delay, the cause of the delay, the effect of the delay on other activities and project milestones and identification of actions required to mitigate the delay.

6. A description of the critical path.

7. An explanation of the use of any allowed constraints, including the reason and purpose for each constraint.

8. A statement describing the status of any required permits.

9. A statement describing the reason for the use of each lag.

For each monthly update and revised schedule submittal, the narrative report shall include the following:

1. A description of the status of the scheduled completion date, focusing on any changes since the previous submission.

2. An explanation if the scheduled completion date is projected to occur after the contract completion date.

3. An explanation if any contract milestone dates are projected to occur after the dates set out in the contract.

4. A description of the work performed since the last schedule update.

5. A description of unusual labor, shift, equipment or material conditions or restrictions encountered.

6. A description of any problems encountered or anticipated since the last schedule update.

7. A statement that identifies any current and anticipated delays. A discussion of delays in the narrative report does not constitute notice in accordance with 105.13.B.9. The statement should include identification of the delayed activity, the type of delay, the cause of the delay, the effect of the delay on other activities and project milestones and identification of actions required to mitigate the delay.

8. A description of the critical path.

9. A description of changes in the critical path and any contract milestone dates from the last schedule update.
10. A report that identifies the changes made between the previous schedule update and the current proposed schedule, including, but not limited to:
   a. Data date
   b. Completion date
   c. Activity code assignments
   d. Schedule calculation options
   e. Activity descriptions
   f. Added activities
   g. Deleted activities
   h. Added activity relationships
   i. Deleted activity relationships
   j. Activity original durations
   k. Activity remaining durations
   l. Activity actual start and finishes
   m. Percent complete
   n. Constraints
   o. Activity coding for phase, location and assigned contractor

11. A statement providing status of pending items, including, but not limited to:
   a. Permits
   b. Change orders
   c. Time extension requests

**F. Extension of Contract Time**

If the Contractor believes work on the contract has been delayed for reasons beyond its control, a written request for extension of contract time may be submitted in accordance with 108.07.E. Upon receipt of the request, the Department will perform a delay analysis by comparing the CPM of Record at the time of the delay with as-built information of the work actually performed. The analysis will identify the actual critical path of work and progress on the critical path and will be used to determine the amount of contract time extension that may be warranted.

The Contractor’s request for extension of time shall include its own analysis of the delay using the CPM of Record at the time of delay and as-built information of work actually performed.

The determination of contract time extension will be based solely on the Department’s analysis.

Delays or suspensions of work due to the Contractor’s failure to comply with the specifications will not be cause for additional compensation or extension of contract time.

**G. Method of Measurement**

Initial, baseline and as-built schedules will not be measured for payment.

Monthly update schedules and revised schedules requested by the Engineer will be measured as each for each such schedule accepted by the Engineer.

Revised schedules submitted at the request of the Contractor will not be measured for payment.

Time impact analysis will not be measured for payment.
**H. Basis of Payment**

Initial, baseline and as-built schedules will be paid for at the contract unit price for lump sum for Schedule. Payment for CPM Schedule will be made on the first estimate after acceptance of the baseline schedule.

Monthly update schedules and revised schedules requested by the Engineer will be paid for at the contract unit price for each upon acceptance by the Engineer.

Payment will be made under CONSTRUCTION COMPLETE which will include the following:

- CPM Schedule
- CPM Schedule, Monthly Update
- CPM Schedule, Revised

All costs necessary to provide initial, baseline and as-built CPM schedules, including the cost of the project scheduler, shall be included in the cost of the pay item for CONSTRUCTION COMPLETE.

All costs necessary to provide monthly update CPM schedules shall be included in the cost of the pay item for CONSTRUCTION COMPLETE.

All costs necessary to provide revised CPM schedules requested by the Engineer shall be included in the cost of the pay item for CONSTRUCTION COMPLETE.
Delete Subsection 108.06 and substitute the following:

The Engineer has the authority to suspend the Work wholly or in part, for as long as he may deem necessary, because of unsuitable weather, or other conditions considered unfavorable for continuing the Work, or for as long as he may deem necessary by reason of failure of the Contractor to carry out orders given, or to comply with any provisions of the Contract. If the performance of all or any portion of the Work is suspended or delayed by the Engineer, in writing, for an unreasonable period of time (not originally anticipated, customary, or inherent to the construction industry) and the Contractor believes that additional compensation and/or contract time is due as a result of such suspension or delay, the Contractor shall submit to the Engineer, in writing, a request for adjustment within 7 calendar days of receipt of the notice to resume work. The request shall set forth the reasons and support for such adjustment.

Upon receipt, the Engineer will evaluate the Contractor's request. If the Engineer agrees that the cost and/or time required for the performance of the Contract has increased as a result of such suspension and the suspension was caused by conditions beyond the control of, and not the fault of, the Contractor, its suppliers, or subcontractors at any approved tier, and not caused by weather, the Engineer will make an adjustment (excluding profit) and modify the Contract in writing accordingly. The Engineer will notify the Contractor of his/her determination whether or not an adjustment of the Contract is warranted.

No contract adjustment will be allowed unless the Contractor has submitted the request for adjustment within the time prescribed.

No contract adjustment will be allowed under this clause to the extent that performance would have been suspended or delayed by any other cause, or for which an adjustment is provided for or excluded under any other term or condition of this Contract.
Section 108 – Prosecution and Progress

Add the following to Subsection 108.08.C:

For this project, an overall completion date has been established. In order to minimize the disruption of normal traffic flow, separate completion times are specified for those portions of the work that requires closing of lanes or detours as specified in Subsection 150.11.

1. Failure to open those roadways as detailed in Special Provision 150.11 A (I-575) will result in the assessment of liquidated damages in the amount of $2,000.00 per hour or portion thereof until the road is open to traffic.

2. Failure to open those roadways as detailed in Special Provision 150.11 B will result in the assessment of liquidated damages in the amount of $1,000.00 per hour or portion thereof until the road is open to traffic.

3. Failure to submit to the Department a SUE Utility Impact Analysis “UIA” in the Department’s prescribed format within 90 days of Notice to Proceed shall result in assessment of liquidated damages in rate of $1,000.00 per calendar day until the UIA has been received.

4. Failure to submit to the Department a Preliminary Utility Status Report within 160 days after the Notice to Proceed has been given for the contract shall result in assessment of liquidated damages in rate of $1,000.00 per calendar day until the report has been received.

5. Failure to complete the grading and construction of the noise barrier within 270 days after the Department authorizes the Contractor to begin land disturbing activities shall result in assessment of liquidated damages in rate of $1,000.00 per calendar day until the grading and construction of the noise barriers have been completed.

These rates are cumulative and in addition to the Liquidated Damages which may be assessed in accordance with Subsection 108.08 for failure to complete the overall project on time.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA
SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 150—Traffic Control

Add the following to Section 150:

150.11 SPECIAL CONDITIONS

A. For I-575, the Contractor shall not install lane closures, perform flagging, or move equipment on the travel way between the hours of 5:30 a.m. to 9:00 p.m. Monday thru Friday and between the hours of 10:00 a.m. to 9:00 p.m. Saturday and Sunday. Failure to adhere to these restrictions will result in liquidated damages as Specified in Sub section 108.08.

B. For Ridgewalk Parkway and all other side streets, the Contractor shall not install lane closures, perform flagging, or move equipment on the travel way that interferes with traffic flow between the hours of 5:30 a.m. to 9:00 a.m. and 3:30 p.m. to 7:00 p.m. Monday thru Sunday. Failure to adhere to these restrictions will result in liquidated damages as Specified in Sub section 108.08.
DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  

SPECIAL PROVISION  

Project Number: CSNHS-0006-00(043)  
P.I. Number: 0006043  
Cherokee County  

SECTION 151-Mobilization  

Delete Section 151 and add the following:  

When mobilization is used as a basis for partial payment for the lump sum items as described in subsection 999.7 of the Special Provision 999 – Design-Build, then the percentage of mobilization value used to determine the partial payment of lump sum bid items will be determined as follows:  

1. One percent (1%) of the schedule of payment value for mobilization will be paid as part of the lump sum bid each month of non ground breaking activities. No more than ten percent (10%) of mobilization shall be used in calculating partial payment before Notice to Proceed for ground breaking activities has been issued.  

2. When Notice to Proceed for ground breaking activities is issued, the next progress payment is 50 percent of the amount of the schedule of payment for mobilization or 3 percent of the total original bid amount for Construction Complete, whichever is less of the amount of the schedule of payment for mobilization minus any previous percent paid.  

3. When 5 percent of the original bid amount for Construction Complete is earned, the next progress payment is 100 percent of the amount of the Schedule of Value for mobilization, or 3 percent of the total bid amount for Construction Complete, whichever is less, minus any previous payments.  

4. Any amount for mobilization itemized in the Schedule for payment in excess of 3 percent of the original bid amount for Construction Complete is paid when work on the Project is complete.  

5. The total sum of the payments shall not exceed the original amount as shown in the schedule of payment for this item as submitted by the Contractor.
No separate measurement will be made for this item. Payment will be part of price bid for CONSTRUCTION COMPLETE.
Georgia Department of Transportation

State of Georgia

Special Provision

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 500—Concrete Structures

Delete Subsection 500.1 and substitute the following: *When high performance concrete is required in accordance with special provision 999.*

This work consists of manufacturing and using High Performance Portland cement concrete to construct precast-prestressed concrete bridge members as shown in the plans and using normal weight Portland cement concrete to construct structures as shown in the Plans.

Add the following to Subsection 500.1.02.A:

Section 831—Admixtures

Add the following to Subsection 500.1.02.B:

AASHTO T 277

Add the following to Subsection 500.1.03.A:

**High Performance Concrete Mix Designs**

The Fabricator is responsible for all concrete mix designs. Ensure that concrete mixes contain enough cement to produce workability within the water-cement ratio specified in Table 1A—High Performance Concrete Mix Table, below.

Submit a mix design for approval to the Office of Materials and Research. Include the sources and actual quantity of each ingredient and laboratory results that demonstrate the ability of the design to attain both the required compressive strength and chloride permeability at 56 days.

Include laboratory compressive strength test results of at least eight test cylinders prepared and cured according to AASHTO T 126. Ensure these test cylinders are made from two or more separate batches with an equal number of cylinders made from each batch.

Also include laboratory chloride permeability test results of at least two test specimens prepared and tested according to AASHTO T 277. Ensure these test specimens are made from two or more separate batches with an equal number of specimens made from each batch.
### Table 1A—High Performance Concrete Mix Table

<table>
<thead>
<tr>
<th>Class of Concrete</th>
<th>Coarse Aggregate Size No.</th>
<th>(1) Minimum Cement Factor</th>
<th>Maximum Water/Cement ratio</th>
<th>(2) Slump Acceptance Limits Lower-Lower</th>
<th>Entrained Air Acceptance Limits % Lower-Lower</th>
<th>(3) Minimum Compressive Strength at 56 days (psi)</th>
<th>Maximum Chloride Permeability at 56 days (Coulombs)</th>
</tr>
</thead>
<tbody>
<tr>
<td>&quot;AAA HPC&quot;</td>
<td>67</td>
<td>650</td>
<td>.330</td>
<td>2</td>
<td>7</td>
<td>3.5</td>
<td>6.5</td>
</tr>
<tr>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td>Beams – As shown on the Plans Piling – 5000</td>
<td>Beams – 3,000 Piling – 2,000</td>
</tr>
</tbody>
</table>

#### English

**1.** Determine the slump acceptance after the addition of high-range water reducer.

**2.** Determine the minimum compressive strength at 56 days using 4 in. diameter x 8 in. high (100 mm x 200 mm) cylinders.

*Add the following to Subsection 500.2 Table 3:*

- Fly Ash 831.2.03.A.1
- Silica Fume 831.2.03.A.4

*Add the following note to Subsection 500.2 Table 3:*

4. Use Type I or III Portland cement in High Performance concrete. Do not use air-entraining cement.

*Add the following to Subsection 500.3.04.D.4:*

f. For High Performance concrete, fly ash may be used as an additive at an addition rate not to exceed 15% of the cement by weight.

*Add the following to Subsection 500.3.04.D:*

6. Silica Fume

   Silica Fume may be used as an additive at an addition rate not to exceed 10% of the cement by weight.
Delete Subsection 500.3.05.T.9.c and substitute the following:

c. After belting, dragging, or brooming, groove the bridge deck and approach slabs perpendicular to the center line as follows:

1) Do not begin grooving until the bridge deck is cured according to Subsection 500.3.05.Z, “Cure Concrete”.
2) If necessary, groove in conjunction with planing required to make the surface corrections specified in Subsection 500.3.06.D, “Bridge Deck Surface Check”. Wait until the concrete is hard enough to support the equipment without distorting.
3) Cut Grooves into the hardened concrete using a mechanical saw device capable of producing grooves 0.125 in (3 mm) wide, 0.125 in (3 mm) deep, and 0.5 in (13 mm) apart, center-to-center.
4) Extend the grooves across the slab to within 1 ft (300 mm) of the gutter lines.

Office of Materials and Research
ADD THE FOLLOWING TO 511.2 MATERIALS, B. FABRICATION:

2. REINFORCEMENT STEEL COUPLERS. When couplers are indicated on the Plans, use Lenton Mechanical Bar Splices, Bar-Grip Systems manufactured by Dayton Barsplice, Inc., Dywidag Thread Bar Reinforcing Systems, or equal.

For the coupler system, develop a minimum of 125% of the guaranteed yield strength of the reinforcing steel to be spliced. Limit the total slip of the reinforcing bars within the splice sleeve after loading to 30 kips per square inch (207 MPa) and relaxing to 3 kips per square inch (21 MPa) to no more than the following, as measured between gauge points clear of the splice sleeve: 0.010 of an inch (.25mm) for reinforcing bars no. 14 (43) or smaller, or 0.030 of an inch (.76mm) for reinforcing bars no. 18 (57).

Make test specimens in the presence of the Engineer or his authorized representative using reinforcing steel consigned for the work. A test specimen consists of a splice made at the job site to connect two 24 inch (600mm) or longer bars using the same splice materials, position, location, and equipment, and following the same procedures to be used to make splices in the work. Prior to incorporating couplers into the work, make and test three specimens that meet the above criteria.

To qualify a coupler product as an equal to those listed above, perform an initial test using five sample couplers selected at random from the couplers consigned to the work. The coupler product is qualified if test results indicate compliance with the requirements shown above. When a test representing a Lot of couplers fails to meet the strength requirement of 125% of the guaranteed yield strength of the reinforcing steel, test four more couplers. If all four tests indicate compliance with the strength requirement, the remaining couplers in the Lot may be incorporated into the work.

Perform all testing required above by the Office of Materials and Research or at a testing laboratory approved by the Department.

If threaded couplers are used, equip them with approved devices which will prevent rotation after installation.

After installation, clean all couplers with a power wire brush or by other approved methods and recoat the couplers with a material prepared and recommended by the coating manufacturer.

Install couplers in strict accordance with the coupler manufacturer’s instructions and as approved by the Engineer.

All costs for the couplers, test samples (including reinforcing steel for tests) and testing of couplers shall be included in the costs of reinforcing steel.
Delete Subsection 511.4 and substitute the following:

511.4 Measurement
No separate measurement will be included for this item.

511.4.01 Limits
A. Construction of Minor Items
   No measurement or payment will be made for the cost of bar reinforcement steel used in constructing minor items. Payment shall be made under CONSTRUCTION COMPLETE.

B. Prestressed Concrete Bridge Members
   Bar reinforcement steel in prestressed concrete bridge members will be considered a component part of the members. Payment shall be made under CONSTRUCTION COMPLETE.

C. Handrail End Posts
   Reinforcement steel in handrail end posts that are a part of the superstructure or substructure will be considered part of the superstructure or substructure items. Payment shall be made under CONSTRUCTION COMPLETE.

D. Lap Splices
   Extra reinforcement steel in lap splices permitted for convenience at splices not shown on the Plans will not be measured for payment. Payment shall be made under CONSTRUCTION COMPLETE.

Delete Subsection 511.5 and substitute the following:

511.5 Payment
Payment will be made under CONSTRUCTION COMPLETE. Includes full compensation for all costs to conform to approved plan details, reinforcement steel in concrete bridges, culverts, concrete handrailings, concrete parapets, barriers, and other items, including minor items, requiring reinforcing steel to complete the Item.
SECTION 524 – DRILLED CAISSON FOUNDATIONS

524.1 General Description
This Work consists of furnishing all labor, materials, equipment, tools and services necessary for construction of drilled caisson foundations and includes all incidentals and additional work in conjunction therewith. Adhere to the Department’s Plans, Special Provisions and Standard and Supplemental Specifications for all Work.

524.2 Materials
Use materials that meet the requirements of the Standard Specifications with the following exceptions:

- Use non-air-entrained Class AA concrete with a coarse aggregate size of No. 67 stone and a slump at time of placement of between 7 and 9 inches (175 and 225 mm). Use 10 percent additional cement and a retarder or water reducing agent in all concrete.

- Use Grade 60 (Grade 420) reinforcing bars that conform to ASTM 615 (ASTM A 615M). If wire spirals are used, use spirals that conform to ASTM A 82.

- Use Grade 2 steel casing that conforms to ASTM A 252.

- Use water that conforms to Section 880 of the Standard Specifications.

524.3 Construction Requirements
524.3.01 Personnel
Construct drilled caissons and supervise the work with personnel who are experienced in this type work. Visit and examine the work site and all conditions, and take into consideration all such
conditions that may affect the work. At least thirty days prior to beginning drilled caisson work, submit to the Engineer for review and approval the following proof of the ability of the personnel to construct drilled caisson foundations:

1. Evidence of the successful completion of at least five projects similar in concept and scope to the proposed foundation. Include names, addresses and telephone numbers of the owners’ representatives for verification.

2. Résumés of foreman and drilling operators to be employed on this project. Provide evidence showing that the drill operator has experience and knowledge of the drill rig to be used on the project. The Department will be sole judge of the qualifications of the foreman and drill rig operator.

3. A detailed sequence of construction for drilled caisson work that describes all materials, methods and equipment to be used, including, but not limited to the following:
   - casing sizes with proposed top and tip elevations
   - drilling equipment including the manufacturer’s specifications on the drill rig
   - methods and equipment for stabilizing and cleaning shaft excavations
   - methods of materials handling and disposal
   - methods and equipment for placing concrete
   - equipment to mix, circulate, contain and de-sand slurry
   - details of tremie or pump line sealing methods
   - details of reinforcement placement, including support and centralization methods

Do not begin drilled caisson construction until the qualifications, construction plan and methods have been approved in writing by the Engineer.

524.3.02 Sequence of Events

1. After the Engineer’s acceptance of the qualifications and methods, and prior to construction, attend a meeting with the Engineer to review specifications, discuss details of construction methods and equipment, review contingency plans in the event that problems occur, and other issues.

2. Demonstrate the adequacy of methods, materials and equipment on a demonstration caisson that will not become part of the completed structure. Excavate this demonstration caisson with the same tools, methods, slurry type, and to the same diameter and maximum depth of the production caissons. Use the same type reinforcing cage and same type slurry that will be used on the load test and production caissons. Do not leave casing in place unless permitted by the Engineer. Construct this demonstration caisson in the river channel at a location approved by the
Engineer no closer than five caisson diameters to the existing and proposed bridge foundations, and no further than ten caisson diameters from the existing and proposed bridge foundations, and to a depth 20% deeper than the deepest caisson used on this project (as measured from the top of the caisson or the high tide elevation, whichever is higher, to the tip of the caisson).

Include all costs of materials and labor required to construct the demonstration caisson in the price bid for CONSTRUCTION COMPLETE.

3. For drilled caissons designed with a Factor of Safety of 2.0, provide a load test. After constructing the demonstration caisson and prior to constructing the production caissons, perform a load test on a separate non-production load test caisson. Construct the load test caisson with the same tools, reinforcement, stabilization and excavation methods, and to the same diameter of the production caissons. Construct the load test caisson in the river channel at a location approved by the Engineer no closer than five caisson diameters to the existing or proposed bridge foundations, and no further than ten caisson diameters from the existing or proposed bridge foundations, and to an elevation equal to the estimated caisson tip.

Include all costs of materials and labor required to construct and test the load test caisson in the price bid for CONSTRUCTION COMPLETE.”

4. If the demonstration or load test caisson(s) are constructed in a river, lake, or other open body of water, reinforcement and concrete will not be required above the river or lake bed elevation.

5. After the Engineer has accepted the results of the load tests and set the tip elevations of the production caissons, begin construction of the caissons as detailed in the Plans and Specifications. The Engineer will set the tip elevations of the production caissons no later than twelve calendar days after receiving the completed load test report.

524.3.03 Equipment

Use excavation and drilling equipment with a rated capacity (including power, torque and downward thrust) to excavate a caisson of the maximum specified diameter to a depth of 30 feet (9.1 meters) or 20 percent deeper than the deepest production caisson indicated on the Plans, as measured from the ground or high water surface elevation, whichever is higher.

524.3.04 Casing

Use casings if the elevation of the top of the caissons is at or below the ground or expected high water elevation at any time during construction. If casings are used, set the elevation at the top of the casing a minimum of 2 feet (600 mm) above the ground or 4 feet (1200 mm) above the expected high water elevation at the site, whichever is greater. Cut off any permanent casing used as shown on the Plans.

Use casing that is a metal shell of a thickness to withstand handling, internal and external pressures, and that is watertight, smooth and clean. If the elevation of the top of the caisson is below ground level or water level at the time of concrete placement, use an oversize casing from ground elevation to a point below the top of the caisson to prevent soil from caving into the fresh concrete. Do not
allow the top of the permanent casing, if required, to extend above the top of the drilled caisson. Use casing in all materials that do not have sufficient strength to safely remain open and stable during and after excavation.

When casing is used, do not use casing with an outside diameter less than the specified diameter of the caisson. That portion of the caisson below the casing may be slightly smaller than the normal outside diameter of the caisson. However, use drilling tools to excavate the caisson below the casing that are no smaller than the Plan diameter of the caisson minus 2 inches (50 mm). Do not leave casing in place unless permitted by the Engineer, and cut off any permanent casing as shown on the Plans.

Provide adequate equipment during concrete placement to prevent pulling up the reinforcing cage during casing extraction. The casing may be pulled in partial stages. Maintain a sufficient head of concrete above the bottom of the casing to overcome hydrostatic pressure. Extract the casing at a slow uniform rate with pull in line with the center of the caisson.

In open-water locations, provide containment at the top of the casing to prevent any material from spilling into the water. Install casing to a depth and in a manner that will produce a positive seal at the bottom of the casing. Do not allow water or other materials, into or out of the excavation area at or below the bottom of the casing.

Do not leave casings in place unless permitted by the Engineer. If casings that are to be removed become bound in the caisson excavation and cannot be practically removed, or if the permanent casing is lowered below the proposed tip elevation, drill the caisson excavation deeper and extend the caisson, including reinforcement, as directed by the Engineer to compensate for loss of capacity due to the presence of the casing. No compensation will be made for the casing remaining in the excavation. The additional length of caisson including excavation, reinforcing steel, concrete and other items incidental to the Work will be paid for at the unit bid price for drilled caissons.

524.3.05 Slurry

Use temporary full-depth casings, mineral or polymer slurry on this project to maintain the stability of the excavations. Manufacture mineral slurry from processed, high-sodium bentonite clays. Use polymer slurry that conforms to the manufacturer’s recommendations, that is site specific, and has been used successfully on a minimum of ten projects of similar size and scope. Adjust the percentage and specific gravity of the slurry used so that the stability of the excavation is maintained, and to allow for proper placement of the concrete.

When using mineral slurry, adhere to the following requirements:

1. Premixing: Mix the mineral slurry thoroughly in a clean, separate tank using clean water that meets the requirements of Section 880 of the Standard Specifications prior to placing the slurry in the excavation. Mix the mineral slurry with high-speed pumps for the time recommended by the manufacturer to allow for its complete hydration.

2. Testing: Provide the equipment necessary to sample the slurry at the bottom of the shaft and provide the equipment and materials to perform viscosity, density, pH and sand content tests on these same slurry samples. Perform all tests in the presence of the Engineer. Perform the viscosity, pH and density tests on the slurry taken from the mixing tanks prior to the introduction of the slurry into the excavation.
Conduct all tests at the end of each workday after drilling is completed and at the beginning of each workday before drilling resumes. Perform these tests on slurry samples collected from the depths and at the times determined by the Engineer to ensure that the slurry within the entire excavation meets these Specifications.

Perform sand content tests on slurry samples taken from the bottom of the shaft after placement of the reinforcing cage, but immediately before pouring concrete. Do not place concrete until all testing produces acceptable results.

a. Viscosity: Produce slurry with a viscosity within the range of 30 to 45 seconds per quart (32 to 48 seconds/liter), as measured by the Marsh Cone Method.

b. Density: Produce slurry with a density within the range of 66 to 73 pounds per cubic foot (1060 to 1170 kilograms per cubic meter). If the sidewalls are unstable, or if artesian flow is present, use a weighing additive to increase the density.

c. pH: Produce slurry with a pH within the range of 8 to 11. The pH of the mineral slurry may be adjusted with the use of soda ash.

d. Sand Content: Measure the sand content of the slurry at the bottom of the shaft by the sand content test just prior to concrete placement. When the sand content at the bottom of the shaft exceeds 4%, clean the bottom of the shaft using desanding or other equipment that is approved by the Engineer.

When using polymer slurry, adhere to the following requirements:

1. **Submittals:** A minimum of 30 working days prior to the use of polymer slurry, submit the following information to the Engineer:
   
a. A list of ten projects and locations where the polymer slurry has been successfully used on projects of similar size and scope.

b. Project owner names and contact phone numbers

c. Diameter and depth of drilled caissons used on these projects.

Do not use the polymer slurry until the Engineer has reviewed and approved the submittal in writing.

2. **Manufacturer’s Representative:** Ensure that a representative of the polymer slurry manufacturer is on site to provide assistance and guidance with the construction of the test excavation (if applicable), the demonstration caisson (if applicable), the load test caisson (if applicable), and the first two production caissons. Ensure that this representative is also available for on-site assistance if problems with the polymer slurry are encountered with the construction of the remaining production caissons. The cost of all on-site assistance and representation will be considered incidental to the cost of the drilled caissons.

3. **Premixing:** Mix the polymer thoroughly in a clean, separate vessel using clean water that meets the requirements of section 880 of the Standard Specifications prior to placing the slurry in the excavation. Add polymer to water flowing through a hose, across a stationary surface into a vessel. Mix the polymer for the time recommended by the manufacturer to allow the polymer to develop adequate viscosity to be self-suspending.
4. Testing: Provide the equipment necessary to sample the polymer slurry from the bottom of the excavation, from the upper portion of the excavation, and from the slurry supply tank or vessel at regular intervals during the excavation process. Provide the equipment and materials needed to perform density, viscosity, pH, and sand content tests on these slurry samples. Perform all tests in the presence of the manufacturer’s representative and the Engineer. Perform the viscosity, pH and density tests on the polymer slurry taken from the mixing tank or vessel prior to the introduction of the polymer slurry into the excavation. After the polymer slurry is in the excavation, perform all tests (i.e. viscosity, density, pH, and sand content) at the bottom and at the upper section of the excavation, at intervals determined by the Engineer. Maintain written records, showing viscosities, pH values, densities, sand content, times, dates, and depth or locations from which samples were taken.

Perform sand content, density, viscosity, and pH during the static period (the period when the polymer slurry is stabilized and shows no further change over a 30-minute interval during which the excavation is completely static), from mid-point of the excavation and from within 24” (610 mm) of the bottom. Do not place concrete until all testing produces acceptable results as follows:

a. Viscosity: Produce polymer slurry with a viscosity within the range of 30 to 125 seconds/quart (32 to 132 seconds/liter) during drilling and less than or equal to 60 seconds/quart (63 seconds/liter) just prior to placing concrete, as measured by the Marsh Cone Method.

b. Density: Produce polymer slurry with a density within the range of 64 lb/ft$^3$ (1025* kg/m$^3$) to 67 lb/ft$^3$ (1073* kg/m$^3$). A weighing additive may be used to increase the density of the polymer slurry if the sidewalls are unstable or if artesian flow is present.

c. pH: Produce polymer slurry with a pH within the range of 8 to 11. The pH of the mix water may be adjusted with the use of soda ash.

d. Sand Content: Measure the sand content of the polymer slurry from the bottom and from the upper portion of the excavation just prior to concrete placement. When the sand content at the bottom of the shaft exceeds 1%, clean the bottom of the shaft using desanding or other equipment that is approved by the Engineer.

* When approved by the Engineer, slurry may be used in salt water, and the allowable densities may be increased by 2 lb/ft$^3$ (32 Kg/m$^3$).

Use slurry with a temperature of at least 40°F (4.4°C) during testing.

524.3.06 Protection of Existing Structures

Monitor structures for settlement that are within a distance of ten shaft diameters or the estimated shaft depth, whichever is greater, in a manner approved by the Engineer. Record elevations to an accuracy of .01 foot (3 mm). Record elevations before construction begins, during the driving of any required casings, during excavation or blasting, or as directed by the Engineer.

Document thoroughly the condition of the structures with descriptions and photographs made both before and after drilled caissons are constructed. Document all existing cracks, and provide copies of all documentation to the Engineer.
At any time settlement of .05 foot (15 mm) or damage to the structure is detected, immediately stop the source of vibrations, backfill any open drilled shaft excavations and contact the Engineer for instructions.

524.3.07 Excavation

Drill and excavate all caissons through whatever substances and to the elevations required. Excavate near the tip elevation in the presence of the Engineer. Stabilize all excavations with slurry to control the excavation diameter and prevent sidewall sloughing, cave-ins or excessive sediment build-up on the excavation bottom. Provide the stabilization prior to excavation.

Use the same tools, stabilization and excavation methods on the production caissons that were used on the accepted demonstration caisson. Construct additional demonstration excavations with no additional cost to the Department, and with no increase in contract time if any changes are made in the tools, excavation and stabilization methods on production caissons from those methods previously demonstrated and accepted.

When casing is not specifically required on the Plans, fill in any over-excavation with Class AA concrete at no additional cost to the Department. Dispose of excess concrete, grout, displaced water and materials removed from the caisson excavation in areas approved by the Engineer, and in accordance with any Federal, State, or local code or ordinance. Verify the accuracy and existence of all applicable codes, ordinances or other regulations prior to disposing materials.

Maintain the fluid level within the casing at a minimum of 4 feet (1.2 meters) above the level of the expected high water elevation or hydrostatic pressure head, whichever is greater, at all times so that unbalanced hydrostatic and/or soil pressures will not cause the collapse of the drilled caisson sidewalls or bottom. In the event of a sudden and/or significant loss of fluid in the excavation, stop construction until a method to stop fluid loss, or until an alternate construction procedure, has been approved by the Engineer.

Conduct excavation near the tip elevation in the presence of the Engineer for determination of the quality of materials encountered. The Engineer will inspect and approve the bottom of each shaft prior to setting the reinforcing cage and pouring concrete. The Engineer may adjust the caisson tip elevation if unsuitable foundation conditions are encountered at the plan tip elevation. Clean the bottom of the excavation so that it is firm, level, and free of sediment or debris. Use a bailing bucket, air lift, or submersible pump to perform the final cleaning of the excavation.

If the excavation below casing remains open for more than 18 hours, over-ream the sides of the excavation with a grooving tool, over-reaming bucket, or other approved equipment to increase the shaft radius a minimum of ½ inch (12 mm) and a maximum of 3 inches (75 mm). Perform the over-reaming and provide and place additional concrete required at no additional cost to the Department, and with no increase in Contract time.

Do not allow any excavation below casing to remain open longer than 36 hours without commencing concrete placement.
524.3.08 Reinforcing Steel

Assemble a cage of reinforcing steel and place it as a unit immediately prior to concrete placement. Assemble the cage so that the clearance between the cage and side of the caisson will be at least 5 inches (125 mm), and the clearance between the cage and bottom of the caisson will be 3 inches (75 mm).

If the caisson is lengthened, extend all reinforcement to within 3 inches (75 mm) of the bottom. If a splice is required, do not locate the splice in the upper 50 feet (15.2 meters) of the caisson, unless shown on the Plans. Tie hoops or spirals to the caisson and column steel (vertical bars) at 100% of the junctions with double wire figure-eight ties. Do not weld the reinforcing steel. Support the cage from the top in a concentric manner to minimize its slumping downward during concrete placement and/or extracting of the casing.

Check the elevation of the top of the steel cage before and after casing extraction. Any upward movement of the steel not exceeding 2 inches (50 mm) or any downward movement thereof not exceeding 6 inches (150 mm) will be acceptable. Any upward movement of the concrete or displacement of the steel beyond the above limits will be cause for rejection. Tie and support the reinforcing steel in the caisson so that the reinforcing steel will remain within allowable tolerances. Provide all temporary or permanent cage stiffeners, braces, helical ties, jigs, or bands that are required to maintain cage stiffness and shape during the assembly, lifting and placement of the reinforcement cage.

In uncased caissons, use only heavy-duty plastic rollers (wheels). In cased caissons, use heavy-duty non-corrosive plastic rollers (wheels) or steel chairs. Place rollers at a maximum interval of 8 feet (2.4 meters) along the cage to ensure concentric spacing for the entire cage length. Use one roller for each 1 foot (300 mm) of diameter of the cage, with a minimum of four rollers at each interval. Do not use concrete spacer blocks. Use rollers that are constructed of a material approved by the Engineer and that have sufficient bearing surface to provide lateral support to the reinforcing cage.

Use rollers of adequate dimension to provide the annular spacing between the outside of the reinforcing cage and the side of the excavated hole or casing as shown on the Plans. If an oversize casing is used, use rollers that will provide concentric spacing. Use pre-cast concrete or heavy-duty plastic bottom supports (feet/boots) to provide a spacing of 3 inches (75 mm) between the cage and caisson bottom.

524.3.09 Concrete

Mix and place all concrete in accordance with Section 500 of the Specifications where applicable and the requirements herein stated. Place concrete as soon as possible after all excavation is completed and reinforcing placed and supported. Place concrete continuously in the caisson to the top elevation of the caisson.

Place concrete using a gravity feed watertight tremie consisting of a pipe at least 8 inches (200 mm) in diameter with a hopper at the top. Concrete may be placed by pumping through a supply line if the Engineer approves this method. Provide a pump supply line with sections that have watertight couplings. Prevent concrete from mixing with fluid from the excavation within the tremie or pump supply line by sealing the end of the line with a foam plug or other device approved by the Engineer.
At the beginning of concrete placement, place the tremie on the bottom of the excavation until the tremie pipe and hopper are filled with concrete. Raise the tremie only enough to induce concrete flow and do not lift it further until the discharge end is immersed at least 10 feet (3 meters) into the deposited concrete. If concrete placement by pumping is used, secure the supply line in place so that the discharge end will not lift off the bottom of the excavation more than 6 inches (150 mm) until at least 10 feet (3 meters) of concrete has been placed. Embed the discharge end of the tremie or pump supply line in the concrete a minimum of 10 feet (3 meters) throughout the remainder of the concrete pour.

Place concrete continuously in the caisson to the top elevation of the caisson until good quality concrete is evident at the top of the caisson, to the satisfaction of the Engineer. Remove any concrete that becomes contaminated with slurry, soil, or other deleterious materials near the top of the caisson and replace it with uncontaminated concrete or chip the contaminated concrete back to sound concrete after the concrete has dried at no additional cost to the Department.

Once concrete placement in the caisson has begun, place all concrete in the caisson within two hours. Adjust the retarder or water reducing agent as approved by the Engineer, for the conditions encountered on the job so that the concrete remains in a workable plastic state throughout the pour. If a longer placement time is needed, provide a concrete design mix that will maintain a minimum 4 inches (100 mm) slump over the longer placement time, as demonstrated by a trial mix and slump loss test to the satisfaction of the Engineer. Repeat the slump loss test as directed by the Engineer when there is an increase of more than 10° Fahrenheit (5.5° Celsius) in ambient temperature from when the trial mix and slump loss tests were performed.

Prepare and cure the top surface of the caisson in accordance with the requirements of Section 500. Locate construction joints as indicated on the Plans. Provide a plan to the Engineer of how the concrete is to be placed and protected at the cut-off elevation to ensure that good quality concrete is placed at the top surface of the caisson. Do not place concrete until the Engineer has approved this plan. Provide a sump to channel displaced water away from the caisson. Do not discharge concrete, contaminated fluids, slurry, soil, or rock into any body of water.

During the twenty-four hour period immediately following the completion of the placement of concrete in the caisson, do not install or extract casing within 50 feet (15 meters) of the completed caisson, and do not excavate any caissons within 15 feet (4.5 meters) of the completed caisson. If the Engineer determines that any construction adversely affects the recently constructed caisson, cease such activities immediately.

Protect any portion of drilled caissons exposed to a body of water from the action of water by leaving the forms in place for a minimum of seven days after pouring the concrete. Remove the forms prior to seven days only if the concrete strength has reached 3000 psi (21 MPa) or greater as tested by cylinder breaks.

**524.3.10 Inspection**

Provide equipment for checking the dimensions and alignment of each caisson excavation. Check the dimensions and alignment of the excavations in the presence of the Engineer.
524.3.11 Tolerances

Adhere to the following construction tolerances for drilled caissons:

1. Construct the drilled caisson to within 3 inches (75 mm) of the plan position plane, at the top-of-caisson elevation. Adhere to a vertical alignment tolerance of ¼ inch (6 mm) per 12 inches (300 mm) of depth.

2. Place reinforcement in accordance with the requirements of Section 511 of the Standard Specifications and Sub-section 524.3.08. Tie column steel (vertical bars) to hoops and spirals at 100% of the junctions with double wire figure-eight ties.

3. Placed vertical caisson reinforcing bars, including bars extending into columns or footings to within ¼ inch (6 mm) of plan location. Place hoops or spirals to within 1 inch (25 mm) of their specified location. Adhere to a side form clearance of within ¼ inch (6 mm) of plan requirements.

4. Place the construction joint of the top of caissons used as caisson/column intermediate bents to within a tolerance of plus or minus 3 inches (75 mm) of the plan elevation.

5. Provide additional materials and labor necessary to correct out-of-tolerance caissons at no cost to the Department and with no increase in contract time.

524.4 Acceptability

In the event that significant voids are suspected in the concrete that were created during placement, verify the integrity of the caisson using a method that has been approved by the Engineer. If the caisson in question is found to be structurally deficient or out of tolerance in any way, the caisson will not be accepted unless corrective measures as approved by the Engineer are accomplished. Furnish additional materials and work necessary to effect corrections at no cost to the Department and with no increase in contract time.

524.5 Load Test

1. Description: This Work consists of furnishing all labor and materials necessary to conduct a bi-directional load test and to report the results to the Department. Conduct a load test if the drilled caissons are designed with a Factor of Safety of 2.0. Obtain the services of an instrument supplier approved by the Department to conduct the load test. Submit proof that the instrument supplier has successfully conducted at least five load tests using the bi-directional test device (Osterberg Cells or equal) to the Engineer. Use the bi-directional load test devices to test separately the shear resistance and end bearing of the caisson by loading the caisson in two directions (upward-shear resistance, downward-base shear and/or end bearing) or by loading the caisson using other approved methods capable of full separation of the upward shear and downward shear and downward base shear and/or end bearing. Place one of the bi-directional test devices at or near the bottom of the load test caisson and place the second bi-directional test device at an elevation recommended by the instrument supplier and approved by the Engineer. Use bi-directional test devices that are capable of applying sufficient loads so that the maximum soil end bearing and maximum side resistance of the caisson-
soil interfaces are measured, or the maximum extension of the bi-directional test devices is achieved during the test.

Conduct the load test in conjunction with the instrument supplier and supply material and labor before, during, and after the load test. Instrument the load test caisson as per Sub-section 524.6 (see Figure Nos. 1 and 2 for information). After the completion of the load test, cut off any portion of the caisson to a depth of 12 inches (300 mm) below stream bed elevation.

The tip elevations of the production caissons may be raised or lowered by the Department and will be set by the Engineer based on the results of the load tests no later than fourteen calendar days after the Engineer receives and accepts the completed load test reports.

2. Materials: Supply all materials required to install the load cells and conduct the load test, including, but not limited to the following:

a. Two load cells of the same size for the load test.

b. Fresh water from a source approved by the Engineer for mixing water-soluble oil provided by the instrumentation supplier to form the hydraulic fluid used to pressurize the load cell.

c. Materials sufficient to construct a stable reference beam system for monitoring the deflection of the caisson during testing. Support the reference beam system at a minimum distance of three diameters from the center of the caisson to prevent the beam’s disturbance. Where space is restricted, two good-quality, self-leveling surveyor’s levels may be used to monitor the caisson movements. In open water areas, protect or brace the test caissons and reference caissons against wave and current action.

d. Materials sufficient to construct a protected area (including provisions such as a tent or shed for protection of the load test equipment and personnel from inclement weather) of size and type required by the Engineer.

e. Electrical power as required for lights, instruments, welding, etc.

f. A beam or pipe system as required by the instrument supplier to support the placement of the load cell and instrumentation pipes and wires when a caisson rebar cage will not be used.

g. Remove materials from the load test caisson at the conclusion of the load test.

3. Equipment: Supply the equipment required to install the load cells, conduct the load test, and remove the load test apparatus as required, including, but not limited to the following:

a. Welding equipment and certified welding personnel, as required, to assemble the test equipment, attach pipes and fittings to the load cells, and prepare the work area.

b. Air compressor of minimum 150 CFM (4.2 CMM) to activate the pump.
c. Cranes or other lifting device for handling the load cells, pipes, and reinforcing cage or alternate instrument support system during the installation of the load cells during the performance of the testing.

d. Equipment and labor sufficient to erect the protected work area and monitoring reference beam system, constructed to the requirements of the Engineer.

e. Suitable operating and reference level platforms, as required for testing over water or in otherwise unstable foundation conditions. Submit to the Engineer for review and approval, a plan for the reference beams and platform system to be used during the load test at least two weeks prior to conducting the load test.

4. **Procedure:** Construct the load test caisson using the approved caisson installation techniques. Assemble the load cells, pipes and other attachments under the direction of the instrument supplier.

Place the load cell assemblies at the bottom of the load test caisson and at other specified locations on the cage. Welding of the rebar to the load cell is permissible.

After the load test caisson excavation has been constructed, inspected and accepted by the Engineer, place a quantity of concrete or grout approximately 6 inches to 12 inches (150 to 300 mm) thick at the base of the caisson by a method approved by the Engineer. Install the load cells and the reinforcing cage assembly in the test shaft under the direction of the instrumentation supplier and the Engineer so that the bottom load cell is resting firmly in/on the concrete/grout bed. Use the utmost care in handling the rebar cage/test equipment assembly so as not to damage the instrumentation during installation. Alternatively, lower the load cells and reinforcing cage assembly as one unit to the near-bottom of the shaft and place a bed of concrete 6 inches to 12 inches (150 to 300 mm) thick placed through a slick line using a concrete pump.

After installation of the load cells, place the concrete in the caisson in the manner specified for similar production caissons. Do not conduct the load test until the minimum compressive strength of the concrete is 3000 psi (21 MPa), as indicated by cylinder breaks. Type III high early cement may be used in the mix to reduce the time between placing concrete and testing if approved by the Engineer.

During the period required to perform the load test, do not vibrate casings into place in the foundation area near the load test. However, drilling may continue, provided that such drilling is for caissons located approximately 50 feet (15 meters) or more from the work area. If test apparatus show any signs of negative effects due to construction activities, cease such activities.

After the completion of the load test, and at the direction of the Engineer, remove any equipment, material, waste, etc.

5. **Report:** Supply the Engineer with five copies of a report of the load test within three calendar weeks after completion of the load test, as prepared by the instrumentation supplier or others approved by the Engineer.
524.6 Load Test Instrumentation Requirements

1. Description: This Work consists of furnishing strain gauges and rod tell-tales, as noted herein, for use in monitoring the load test. Provide and install the gauges and rod tell-tales at the locations directed by the Engineer. Provide shelter over the load test location to protect the gauges and other instrumentation from inclement weather. Replace any instrumentation devices damaged at no additional cost to the Department.

2. Materials: Provide the following type and number of strain gauges and rod tell-tales for the load test:

   a. Twelve vibrating wire embedment strain gauges set to measure compression that read to a maximum strain range of at least 3000 microstrains with a sensitivity of 1 microstrain. Provide waterproof gauges supplied with shielded multi-conductor electric cable, and with two connection devices or fasteners of a suitable type to securely join the gauges to a longitudinal reinforcement bar of the drilled caisson rebar cage. Provide access to the drilled caisson rebar cage to allow the instrument supplier to install the strain gauges. Install the gauges at intervals of approximately equal spacing throughout the rebar cage, or at the locations directed by the Engineer. Supply sufficient lengths of cable for each gauge to reach from the gauges to approximately 30 feet (10 meters) beyond the top of the casing. Perform the monitoring of the strain gauges during the load test. Provide a copy of all the readings to the Engineer at the completion of the load test.

   b. Provide six rod tell-tales to measure movement within the drilled caisson. Use rod tell-tales consisting of $\frac{5}{16}$ inch (8 mm) diameter flush-jointed stainless steel threaded rods that can be connected by means of standard threading couplings. Encase the tell-tales within a minimum $\frac{1}{2}$ inch (12 mm) diameter (ID) steel threaded pipe or $\frac{3}{4}$ inch (19 mm) diameter (ID) PVC flush-joint pipe. Provide and install the PVC or steel pipe. Install the tell-tales at the following points on the rebar cage, or as directed by the Engineer:

   i. Two (2) each at three-quarters of the caisson length from the top.
   ii. Two (2) each at the midpoint of the caisson.
   iii. Two (2) each at one-fourth of the caisson length from the top.

   Install and monitor the rod tell-tales. Provide a copy of all the readings to the Engineer at the completion of the load test. Remove the stainless steel rod tell-tales at the completion of the load test.
524.7 Non-destructive testing of drilled caissons

1. **Description:** This Work consists of furnishing testing services and equipment for conducting Crosshole Sonic Logging (CSL) on drilled caissons, providing and installing pipes, grouting of pipes, and all other equipment necessary to conduct sonic testing.

2. **General Requirements:** Use the nondestructive testing method called Crosshole Sonic Logging on all caissons including demonstration, load test and production caissons.

   Employ an experienced independent testing organization that has been approved by the Engineer to conduct the CSL tests. Conduct the testing a minimum of twenty-four hours after the placement of all concrete in the shaft, but no later than seven calendar days after placement.

   After the Engineer has accepted the production caissons, remove all water from CSL-access pipes, and then fill these pipes with grout that the Engineer has approved.

3. **Pipe installation:** Install six pipes in each production caisson to permit access for CSL testing. Use 1.5 to 2 inch (38 mm to 50 mm) inside diameter schedule 40 steel pipes or PVC pipes that have round, regular internal diameters free of defects or obstructions including any at pipe joints in order to permit the free, unobstructed passage of a 1.35 inch (33 mm) diameter source and receiver probes. In addition, use pipes that are watertight and free from corrosion with clean internal and external faces to ensure passage of the probes and a good bond between the concrete and the pipes.

   Fit each pipe with a watertight shoe on the bottom and a removable cap on the top. Securely attach the pipes to the interior of the reinforcement cage with a minimum cover of 3 inches (75 mm). The Engineer may allow the pipes to be installed on the outside of the cage if adequate cover and clearance are available. Install the pipes in each caisson in a regular, symmetric pattern such that each pipe is placed the maximum distance possible from each adjacent pipe, with an equal spacing around the perimeter of the cage. Prior to construction, submit the selection of pipe size and type, and the proposed method to install the pipes to the testing organization and to the Engineer. Do not install the pipes until the Engineer has approved the selection and installation method.

   Install the pipes as near to parallel as possible. Extend the pipes 6 inches (150 mm) above the caisson bottom and at least 3 feet (900 mm) above the caisson top. If the caisson top is subsurface, extend the pipes at least 2 feet (600 mm) above the ground or water surface. Use watertight joints at any joints that are required to achieve full-length pipes. Replace any pipes that are damaged during installation with new pipes. Fill the pipes with clean water within 4 hours after concrete placement, and cap the pipe tops to keep debris out of the pipes. Do not apply excess torque, hammering, or other stresses during the removal of caps that could break the bond between the pipes and the concrete.

4. **Typical CSL test equipment:** Typical CSL test equipment consists of the following components:

   a. A microprocessor-based CSL system for display of individual CSL records, analog-digital conversion and recording of CSL data, analysis of receiver responses and printing of CSL logs.

   b. Ultrasonic source and receiver probes for 1.5 or 2 inch (38 mm or 50 mm) I.D. pipe, as appropriate.
c. An ultrasonic voltage pulser to excite the source with a synchronized triggering system to start the recording system.

d. A depth measurement device to determine and record depths.

e. Appropriate filter/amplification and cable systems for CSL testing.

5. CSL logging procedures: Before the placement of concrete, plumb one pipe per shaft and record the pipe length, including a notation of the stickup of the pipe above the caisson tips. Provide the information on the caisson bottom and top elevations and/or length, along with construction dates to the Engineer and the testing organization before the CSL tests. Conduct the CSL tests between pairs of pipes. Allow the approved testing organization to determine which pairs of pipes are to be tested. Typically, perimeter and/or major diagonals are tested. Conduct additional testing in the event anomalies are detected at no additional cost to the Department.

Conduct the CSL tests with the source and receiver probes in the same horizontal plane unless test results indicate potential defects, in which case the questionable zone may be further evaluated with angled tests (source and receiver vertically offset in the pipes). Perform all CSL measurements at depth intervals of 0.2 feet (60 mm) or less, beginning from the bottom of the pipes to the top of each caisson. Pull the probes simultaneously, starting from the bottom of the pipes, over a depth-measuring device. Removed any slack from the cables prior to pulling, to provide for accurate depth measurements of the CSL records. Report any defects indicated by longer pulse-arrival times and significantly lower amplitude/energy signals to the Engineer, and conduct further tests as required by the Engineer to evaluate the extent of such defects. Additional non-destructive testing methods that may be used to evaluate possible defects include Singlehole Sonic Logging, Gamma-Gamma Nuclear Density Logging, and/or Surface Sonic Echo, and Impulse Response Tests.

6. CSL testing results: Supply five copies of the CSL tests in the form of a written report to the Engineer that includes the CSL logs with the following analysis:

   a. Initial pulse arrival time versus depth.

   b. Pulse energy/amplitude verses depth.

Provide a CSL log for each pipe pair tested with any defect zones indicated on the logs and discussed in the test report, as appropriate.

7. Evaluation of CSL test results: The Engineer will evaluate the CSL test results and determine whether or not the drilled caisson is acceptable.

If the Engineer determines that the drilled caisson is unacceptable based on the CSL tests, replace or core the caisson to allow further evaluation of the caisson. Perform either option at the direction of the Engineer, at no additional cost to the Department.

8. Core drilling of drilled shaft concrete: Core the tested caissons that are determined to be unacceptable by the CSL tests to determine the quality of the concrete. Obtain core samples from each defective caisson for the full depth of the caisson. Perform this work at no additional cost to the Department, and with no increase in contract time.

Retain an accurate log of cores and store the cores in a crate that is properly marked showing the caisson depth at each interval of core recovery. Transport the cores and five copies of the coring logs
to the Engineer. After the Engineer has accepted the production caissons, fill these core holes with grout that the Engineer has approved.

524.8 Measurement and Payment

No separate measurement for payment purposes shall be made for constructing drilled caissons. All costs for excavation, furnishing and placement of reinforcing steel and concrete in the caisson, all temporary casing, disposal of excavated materials, demonstration and load test caissons, providing, installing and measuring the instrumentation, bi-directional load test devices and CSL testing, coring and additional testing, and the cost of furnishing all tools, safety devices, labor, equipment and all other necessary items to complete the work shall be included in the price bid for CONSTRUCTION COMPLETE.

Office of Materials and Research
Add the following:

Delete Subsection 682.1 General Description and substitute the following:

682.1 General Description

This work includes furnishing and installing wire, cable, and conduit for roadway and structure lighting systems, complete or as indicated on the Plans. This work also consists of furnishing and installing a Multi-cell or Continuous Flexible Conduit System for Fiber Optic Cable, complete or as indicated on the Plans. The installation of conduit for fiber optic cable shall not require the presence of a qualified electrician on the job site.

Add the following to Subsection 682.2:

All multi-cell and continuous flexible conduit products shall meet the General Specifications as set out in these Special Provisions. Those products shall be installed, applied, inspected, and/or utilized in accordance with the Construction Section of these Special Provisions. Prior to any conduit work and within 60 days after Notice to Proceed, submit catalog sheets, engineering drawings, and maintenance procedures for review by the Engineer for all products and procedures in these Special Provisions to be used on the Project. If the products to be used are not specified within these Special Provisions or not listed separately and/or completely on the details of the Plans, submit catalog sheets, engineering drawings, factory specifications, a set of installation procedures, and a set of operation and maintenance procedures (for multi-cell conduit) for review by the Engineer. No work shall be done using these products until after submittals have been approved by the Engineer.

A summary of the products, their Sections, and each product’s specification included in these Special Provisions are listed below:

A. MARKING TAPE ........................................................................................................... Section 682.2.03
Visible marking tape, location and warning system.

B. CONDUIT, NONMETAL, TYPE 2 - POWER SERVICE ........................................... Section 682.2.04
All conduit shall be Schedule 40 polyvinyl chloride.
C. MULTI-CELL FACTORY INSTALLED DUCT SYSTEM................................. Section 682.2.05

The multi-cell innerducts shall be colored red, white, yellow, and orange, and utilized as noted: red = hybrid fiber optic cable; white = open spare/interconnect/control circuit; yellow = single mode fiber optic cable; and orange = multi-mode fiber optic cable.

D. CONDUIT DUCT BANK................................................................................ Section 682.2.06

Conduit duct bank shall be a configuration of high density polyethylene conduits.

E. CONDUIT, NONMETAL, TYPE 3................................................................. Section 682.2.07

F. CONDUIT, FIBERGLASS ........................................................................ Section 682.2.08

G. PULL TAPE ............................................................................................ Section 682.2.09

H. DUCT PLUGS .......................................................................................... Section 682.2.10

I. CONDUIT DETECTION WIRE ................................................................. Section 682.2.11

J. ELECTRICAL COMMUNICATION BOX ............................................... Section 682.2.13

K. ELECTRICAL COMMUNICATION BOX REHABILITATION.............. Section 682.2.14

Add the following Subsections: 682.2.02 – 682.2.14

682.2.02 MATERIAL CERTIFICATION

The following chart provides an outline of the submittal requirements for the equipment and components for this pay item. This chart shall be used as a guide and does not relieve the Contractor from submitting additional information to form a complete submittal package.

Submit submittal data for all equipment, materials, test procedures, and routine maintenance procedures required for these items within 60 calendar days after the Notice To Proceed and prior to any installation, unless noted otherwise in the Contract Documents.

Submit to the Engineer for approval, six (6) copies of the manufacturer’s descriptive literature (catalog cuts), technical data, operational documentation, service and maintenance documentation, and all other materials required within these Special Provisions.

Provide submittal data that is neat, legible, and orderly. Neatly organize each package of submittal data and separate by hardware item. Use the “Materials Certification Package Index and Transmittal Form”, contained in Section 105.02 of the Special Provisions, for each pay item to document and list all material and components that are included in the submittal package. Any submittal data submitted without the index/transmittal form or that is incomplete will be rejected.
Submittal data shall include complete technical and performance specifications on all elements of the conduit system. Below is a sample listing of submittal data requirements by 682.X.X subsection.

For **Subsection 682.2.03 Marking Tape** submit materials submittal data for the marking tape.

For **Subsection 682.2.04 Conduit, Nonmetal, Type 2 - Power Service** submit materials submittal data for the conduit, fittings, pull boxes, and electrical service wire.

For **Subsection 682.2.05 Multi-Cell Factory Installed Duct System** submit materials data for the conduit system, innerduct, outerduct, coupling body, fittings, accessories, bends and sweeps, installation procedures, and maintenance procedures.

For **Subsection 682.2.06 Conduit Duct Bank** submit materials submittal data for conduit, couplings, and coupling procedures.

For **Subsection 682.2.07 Conduit, Nonmetal, Type 3** submit materials submittal data for conduit, couplings, and coupling procedures.

For **Subsection 682.2.08 Conduit, Fiberglass** submit materials submittal data for conduit, couplings and fittings, and coupling and fittings procedures.

For **Subsection 682.2.09 Pull Tape** submit materials submittal data for pull tape and installation procedure.

For **Subsection 682.2.10 Duct Plugs** submit materials submittal data for duct plugs for empty conduit and duct plugs with cable installed.

For **Subsection 682.2.11 Conduit Detection Wire** submit materials submittal data for conduit detection wire and testing procedure.

For **Subsection 682.2.13 Electrical Communication Box, Type ____** submit materials submittal data for electrical communication box, ring and cover, conduit terminators, cable racks and hardware, sealant, and conduit alignment jigs or spacers.
For Subsection 682.2.14 Electrical Communication Box, Rehabilitation submit materials submittal data for cable racks and hardware.

682.2.03 MARKING TAPE SPECIFICATIONS

A. Requirement For Use
When fiber optic cable is installed underground in conduit or directly buried or when empty conduit is installed, install a dielectric marking tape directly over the conduit or cable below finished grade. The tape shall be installed for the full length of the cable or conduit run. When the conduit or cable is in a trench backfilled with full depth concrete, no marking tape shall be installed.

B. Printing
The color of the tape shall be orange with “GEORGIA DOT FIBER OPTIC CABLE-CALL 1-404-624-2661” printed every 6.5 ft (2.0 m).

C. Physical Properties
The tape shall be a dielectric, polyolefin film tape, 0.004 in. (0.1 mm) thick, 3 in. (76 mm) wide. The tape shall be constructed using material and ink colors which will not change when exposed to acids and other destructive substances commonly found in the soil.

The physical test methods along with typical properties and values are specified below:

<table>
<thead>
<tr>
<th>PROPERTY</th>
<th>METHOD</th>
<th>VALUE</th>
</tr>
</thead>
<tbody>
<tr>
<td>Standard Weight</td>
<td>ASTM-D2103</td>
<td>0.02 lb/ft² (0.1 kg/m²)</td>
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<tr>
<td>Thickness-Overall</td>
<td>ASTM-D2103</td>
<td>0.004 in. (0.1 mm)</td>
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<tr>
<td>3” Tensile Break-MD</td>
<td>ASTM-D882</td>
<td>35 lbf (160 N)</td>
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<tr>
<td>3” Tensile Strength-MD</td>
<td>ASTM-D882</td>
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</tr>
<tr>
<td>Elongation-TD</td>
<td>ASTM-D882</td>
<td>660%</td>
</tr>
<tr>
<td>PPT Resistance-MD</td>
<td>ASTM-D2582</td>
<td>12 lbf (53 N)</td>
</tr>
<tr>
<td>PPT Resistance-TD</td>
<td>ASTM-D2582</td>
<td>14 lbf (62 N)</td>
</tr>
<tr>
<td>Tear Strength-3” x 8”-MD</td>
<td>ASTM-D2261</td>
<td>24 lbf (110 N)</td>
</tr>
<tr>
<td>Tear Strength-3” x 8”-TD</td>
<td>ASTM-D2261</td>
<td>32 lbf (140 N)</td>
</tr>
</tbody>
</table>

PPT – Puncture Propagation Tear
MD/TD – Machine Direction/ Transverse Direction

682.2.04 POWER SERVICE, NONMETAL, TYPE 2 - POWER SERVICE

Install conduit as indicated in the Plans. Conduit and fittings shall be Schedule 40 unplasticized PolyVinyl Chloride (PVC) that meets Federal Specification WC-1904-A. If the conduit is shown in the plans crossing pavement, install the conduit under the pavement via the directional bore method in accordance with Section 615 and Details. If using the directional bore method of installation, install a Schedule 40, UL-listed conduit suitable for being installed via the directional bore method that does not require solvent welds. Install Type 2 pull boxes if in unpaved shoulder or concrete ground mounted electrical junction boxes if in pavement along the conduit route between the electrical service pole and the equipment cabinet requiring power. Install the pull boxes as described in Section 647 and in Details that meet requirements in Section 516.
925 except that the covers should be furnished with the logo “ELECTRICAL”. Make any repairs to pavement required as a result of the installation of electrical junction boxes in accordance with Department standards. Within the conduit and pull boxes, install electrical service wire that meets requirements in Section 922. Install any transformers as may be required because of voltage drops between the electrical service pole and the equipment cabinet requiring power.

682.2.05 MULTI-CELL “FACTORY INSTALLED” CONDUIT SYSTEM

A. DESCRIPTION
The multi-cell conduit system shall be a pre-assembled conduit manufactured from a 4 in. (102 mm) round outerduct containing either 3 or 4 factory installed round innerducts. The innerducts shall be held together in a square (4 conduit system) or triangular (3 conduit system) configuration by a system of spacers, bands, or other mechanism. The coupling system shall be resistant to water infiltration, air loss during cable installation and shall be capable of locking the system tightly together in order to not allow free twisting of the innerducts.

B. OUTERDUCT
All outerduct shall be 4 in. (102 mm) trade size and shall have a nominal 20 ft (6 m) lay length except for the steel conduit which shall have a minimum lay length of 10 ft (3 m). Types to be used shall be designated on the plans or in the proposal. All outerduct shall conform to the following requirements.

1. The outerduct shall have the following identification information:

   Line text height shall be at least ½ in. (10 mm). Text labeling shall occur a maximum of every 2 ft (0.6 m). The text shall be indelibly printed in high contrast to the conduit. The text shall be oriented to face up for underground installation; the text shall be oriented to face down for under bridge installation.

2. The duct shall be marked with data which will provide traceability of the manufacturer, plant location, date, shift, and machine of manufacturer.

3. Any additional wording on the conduit, such as “this side up” or “this side down”, shall be consistent with the installation orientation.

4. The spigot end of the duct shall have a circumferential insertion depth mark to insure that proper insertion depth is achieved. This mark is not required for spigots with threaded fittings.

C. INNERDUCT
Innerduct shall be manufactured from Poly-vinyl Chloride (PVC) or High Density Polyethylene (HDPE).

D. COUPLING BODY
The multi-cell conduit shall be joined by use of a coupling system which effectively seals the outerducts and innerducts but allows for expansion or contraction in the system.

E. ACCESSORIES AND FITTINGS
The multi-cell conduit system shall be furnished with all necessary fittings and accessories. These shall include, but shall not be limited to, coupling kits, lubrication fittings, repair kits, manhole terminator kits w/plugs, installation accessories, deflection fittings, and epoxy adhesive kits. Each multi-cell system shall offer a complete line of fixed, rigid bends and sweeps. For applications in which the multi-cell conduit system is specified on the Plans and/or by the Engineer to be attached to a bridge or other structure, bridge hanger assemblies, expansion joints, and conduit support devices shall be required. These hanger assemblies, expansion joints, and support devices shall be designed for application to the specific bridge or structure for which they will be used, and their materials and design shall be approved by the Department prior to their use.
F. MATERIALS

Provide the Engineer with Manufacturer’s test results for the required testing and certification in accordance with Subsection 106.05 of the Georgia Standard Specifications.

1. OUTERDUCT
   a. Schedule 40, Polyvinyl Chloride (PVC) Conduit - Schedule 40, polyvinyl chloride (PVC) conduit shall conform to the requirements of the National Electrical Manufacturers Association (NEMA) Standards Publication No. TC-6 and 8-2003, Type DB-120, except that the conduit shall be white in color and shall have a minimum 5 in. (127 mm) long integral bell to accommodate the coupling body.

   b. Type “C”, Polyvinyl Chloride (PVC) Conduit - Type “C,” polyvinyl chloride (PVC) conduit shall conform to the requirements of the National Electrical Manufacturers Association (NEMA) Standards Publication No. TC-6 and 8-2003, Type DB-120, except that the conduit shall be white in color and shall have a minimum 5 in. (127 mm) long integral bell to accommodate the coupling body.

   c. Steel Conduit - Rigid steel conduit shall meet the requirements of Sub-Section 923.2 of The Georgia Standard Specifications. All metal accessories and fitting used with the conduit shall be compatible and shall meet the galvanization requirements of Sub-Section 923.2.

   d. “Bullet Resistant” Fiberglass Conduit - Bullet resistant fiberglass conduit shall have a minimum wall thickness of 0.250 in. (6.35 mm). The conduit shall prevent the penetration of a 0.45 caliber slug fired from a distance of 20 ft (6 m). The conduit shall conform to the following requirements when tested in accordance with the referenced specification. All accessories and fittings, including outerduct couplings and expansion joints, shall meet all the same “bullet resistant” requirements as the conduit. All conduit and fittings shall be black.

   PHYSICAL AND MECHANICAL PROPERTIES TEST METHODS

   Ultimate Tensile Strength - 11,000 PSI (75800 kPa) Min. ASTM D 2105
   Dielectric Strength - 500 Volts/Mil. ASTM D 149
   Water Absorption - 1% Max. ASTM D 570
   Specific Gravity - 1.9 - 2.0 ASTM D 792
   Glass Content - 68 +- 2% API SPEC 15 LR
   Barcol Hardness - 58-52 ASTM D 2583

2. INNERDUCT (WITHIN MULTI-CELL)

   Innerducts shall be manufactured from polyvinyl chloride (PVC) or high density polyethylene (HDPE). Innerducts shall be factory treated with an atomized silicone or manufactured in a manner to reduce friction during pulling of fiber optic cable. Innerduct to be used in bends and sweeps shall have a minimum burn through time of 30 minutes when tested in accordance with Generic Requirement GR-356-CORE, Issue 1, October 1995. The dimensions of innerduct shall meet the requirements of the manufacturer’s catalog cuts approved by the Department.

   a. PVC INNERDUCT

      PVC innerduct shall be factory treated with an atomized silicone to reduce friction. The innerduct shall conform to the following requirements:

      COLOR OF INNERDUCTS NOMINAL SIZE
      3-way (2 gray & 1 white) 1 1/2” (38 mm)
      4-way (3 gray & 1 white) 1 1/4” (32 mm)

      Note: The white innerduct shall be located directly under the print line on the outerduct.

      Alternate innerduct colors shall be permitted only when requested in writing and upon receiving written approval from the Engineer.

   b. HDPE INNERDUCT

      HDPE innerduct shall have a permanent dry lubricant extruded within the inner wall and shall incorporate longitudinal ribs within the inner wall. HDPE innerduct shall conform to the following requirements:
COLOR OF INNERDUCTS  NOMINAL SIZE
3-way (yellow, orange, red)  1 1/2” (38 mm)
4-way (red, white, yellow, orange)  1 1/4” (32 mm)

Innerduct colors shall be oriented in a clockwise direction as shown above, looking at the spigot end of the multi-cell conduit system. The white innerduct for 4-way and yellow innerduct for 3-way shall be located directly under the print line on the outerduct.

Alternate innerduct colors shall be permitted only when requested in writing and upon receiving written approval from the Engineer.

3. COUPLING BODY
The coupling body shall be designed with either 3 or 4 bores as required. The coupling body shall be designed so that when the conduit is joined, the outer walls of the innerducts and the inner walls of the outerduct shall be sealed, providing an airtight seal from within the innerduct system and a watertight seal from the outside of the outerduct. The coupling body shall be tested for water tightness and air tightness per Bellcore TA-NWT-000356 and shall conform to the following specifications.

- Water infiltration: minimum 11-foot head or more for 7 days
- Air Tightness: minimum 100 PSI (690kPa)

4. BENDS AND SWEEPS
Each multi-cell system shall offer a complete line of fixed bends and sweeps. No flexible bends will be permitted. HDPE, PVC, and bullet resistant fiberglass bends and sweeps shall have compatible bell and spigot ends. Steel conduit bends and sweeps shall have compatible threads and reversing couplings for connection to the conduit. PVC innerducts shall not be allowed in bends and sweeps. In no case shall bends and sweeps exceed a 90 degree direction change. Bends and sweeps shall be available as follows:

- Fixed Bends: Fixed bends for steel conduit shall be available in no less than 4 ft (1.22 m) radii in 11 1/4 degrees, 22 1/2 degrees, 45 degrees, and 90 degree bends. Fixed bends for PVC and bullet resistant fiberglass multicell conduit shall be available in radii no less than the following:

<table>
<thead>
<tr>
<th>RADIUS DEGREE BEND</th>
<th>11 1/4 degrees</th>
<th>22 1/2 degrees</th>
<th>45 and 90 degrees</th>
</tr>
</thead>
<tbody>
<tr>
<td>4 ft. (1.22 m)</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>6 ft. (1.83 m)</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>9 ft. (2.74 m)</td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

682.2.06 CONDUIT DUCT BANK

A. MATERIAL
Install Conduit Duct Banks by configuring individual conduits into a continuous duct bank from termination point to termination point as shown in the Standard Details and other Contract Documents. Conduit Duct Bank, Type 1 shall include six 1-1/4” (32 mm) conduits and three 2” (51 mm) conduits. Conduit Duct Bank, Type 2 shall include eight 1-1/4” (32 mm) conduits and three 2”(51 mm) conduits. Conduit Duct Bank, Type 3 shall include four 2”(51 mm) conduits. Conduit Duct Bank, Type Special shall be as shown in the Plans.

Conduit shall be manufactured from virgin high-density polyethylene. Conduit shall be extruded from colored material for uniform full-thickness coloring. Where striping is required, a minimum of three colored longitudinal stripes of HDPE material shall be co-extruded on the conduit outer wall. The three stripes shall be equally spaced around the circumference and continuous for the entire length of conduit. Printed or embossed striping is not permitted.

All conduit shall be labeled with durable identification giving the name of the manufacturer, conduit size (inner diameter trade size and wall thickness/rating), manufacture/date codes, and sequential foot marking. Labeling shall occur at a maximum of every 2 ft (0.6 m).
Where required in the Contract Documents, conduits shall be located and secured in the conduit duct bank by conduit spacers configured into an assembly that is appropriate for the duct bank type.

1. 1-¼ in. (32 mm) Conduit
   1-¼ in. (32 mm) Conduit shall conform to ASTM D-3035 and meet the following requirements:
   - Smoothwall SDR 11
   - Nominal outer diameter: 1.660 in. (42.16 mm)
   - Minimum inner diameter: 1.313 in. (33.35 mm)
   - Minimum wall thickness: 0.151 in. (3.84 mm)

2. 2 in. (51 mm) Conduit
   2 in. (51 mm) Conduit shall conform to ASTM D-3035 and meet the following requirements:
   - Smoothwall SDR 11
   - Nominal outer diameter: 2.375 in. (60.32 mm)
   - Minimum inner diameter: 1.885 in. (47.88 mm)
   - Minimum wall thickness: 0.216 in. (5.49 mm)

3. Conduit Spacer
   Conduit spacers shall be steel or molded high impact polystyrene that is resistant to rot and moisture absorption. Spacers shall be manufactured to have an interlocking design such that spacers for different conduits can be assembled for the appropriate duct bank configuration. All spacers on the bottom of an assembly shall be “base” that includes a flat base with a minimum of 6 in² (3900 mm²) of bearing area for each bottom conduit.

B. COUPLING
   Make every effort to minimize coupling. Coupling shall only be permitted with the advance permission of the Engineer.

   Couplings shall be airtight and watertight. All couplings shall be installed in accordance with the conduit and the coupling manufacturer’s recommendations. Only couplings of the type specified below and approved by the conduit manufacturer are permitted.

   Couplings shall be accomplished only by hydraulic press-on or electro-fusion coupling methods. Use hydraulic press-on couplings of seamless tool-grade tubular aluminum with sealing barbs and center stop. Use hydraulic compression duct coupling tools and follow all manufacturer’s installation procedures, fully inserting both conduit sections to the coupling center stop. Use pre-fabricated electro-fusion couplings that are field-installed using the coupling manufacturer’s recommended automatic self-monitoring fusing machine and installation procedures. Do not use any other coupling methods.

C. TERMINATION
   Conduit duct banks shall be terminated in electrical communications boxes (ECBs) and pull boxes as shown in the Standard Details of the Contract Documents and in accordance with Section 647. Duct banks terminated in ECBs shall be installed into factory-installed knockout windows only, which shall be fully grouted and sealed around all conduits and to the full thickness of the box wall. Duct banks terminated in pull boxes shall be installed into factory-installed conduit terminators; conduit adhesive sealants recommended by the terminator and conduit manufacturers shall be used.
682.2.07 CONDUIT, NONMETAL, TYPE 3

A. MATERIAL
Conduit shall be manufactured from virgin high-density polyethylene (HDPE). Conduit shall be extruded from colored material for uniform full-thickness coloring. Where striping is required, a minimum of three colored longitudinal stripes of HDPE material shall be co-extruded on the conduit outer wall. The three stripes shall be equally spaced around the circumference and continuous for the entire length of conduit. Printed or embossed striping is not permitted. Unless otherwise noted in the Contract Documents, color code for conduit used for Type 3 installation shall comply with the Conduit Duct Bank Color Code schedule listed on the plan detail sheet.

All conduit shall be labeled with durable identification giving the name of the manufacturer, conduit size (inner diameter trade size and wall thickness/rating), manufacture/date codes, and sequential foot marking. The conduit shall also be labeled with the following: “Georgia DOT Cable – For Assistance Call 404-624-2661”. Labeling shall occur a maximum of every 4 ft. (1.2 m).

1. 1 in. (25 mm) Conduit
   1 in. (25 mm) Conduit shall conform to ASTM D-3035 and shall meet the following requirements:
   - Smoothwall SDR 11  
   - Nominal outer diameter: 1.315 in. (33.40 mm)
   - Minimum inner diameter: 1.030 in. (26.16 mm)
   - Minimum wall thickness: 0.120 in. (3.05 mm)

2. 1¼ in. (32 mm) Conduit
   1¼ in. (32 mm) Conduit shall conform to ASTM D-3035 and shall meet the following requirements:
   - Smoothwall SDR 11  
   - Nominal outer diameter: 1.660 in. (42.16 mm)
   - Minimum inner diameter: 1.313 in. (33.35 mm)
   - Minimum wall thickness: 0.151 in. (3.84 mm)

3. 1½ in. (38 mm) Conduit
   1½ in. (38 mm) Conduit shall conform to ASTM D-3035 and shall meet the following requirements:
   - Smoothwall SDR 11  
   - Nominal outer diameter: 1.900 in. (48.26 mm)
   - Minimum inner diameter: 1.506 in. (38.25 mm)
   - Minimum wall thickness: 0.173 in. (4.39 mm)

4. 2 in. (51 mm) Conduit
   2 in. (51 mm) Conduit shall conform to ASTM D-3035 and shall meet the following requirements:
   - Smoothwall SDR 11  
   - Nominal outer diameter: 2.375 in. (60.32 mm)
   - Minimum inner diameter: 1.885 in. (47.88 mm)
   - Minimum wall thickness: 0.216 in. (5.49 mm)

B. COUPLING
Make every effort to minimize coupling. Coupling shall only be permitted with the advance permission of the Engineer.

Couplings shall be airtight and watertight. All couplings shall be installed in accordance with the conduit and the coupling manufacturer’s recommendations. Only couplings of the type specified below and approved by the conduit manufacturer are permitted.

Couplings shall be accomplished only by hydraulic press-on or electro-fusion coupling methods. Use hydraulic press-on couplings of seamless tool-grade tubular aluminum with sealing barbs and center stop. Use hydraulic compression duct coupling tools and follow all manufacturer’s installation procedures, fully inserting both conduit sections to the coupling center stop. Use pre-fabricated electro-fusion couplings that are field-installed using the coupling manufacturer’s recommended automatic self-monitoring fusing machine and installation procedures. Do not use any other coupling methods.
C. TERMINATION

Install Type 3 conduits in pull boxes in accordance with Section 647 and the Standard Details of the Contract Documents. Unless otherwise shown in the Plans, install Type 3 conduits in different types of underground spaces as follows. Type 3 conduits shall be terminated in electrical communications boxes (ECBs) and Types 6 and 7 pull boxes using factory-installed terminators in the ECB or pull box; conduit adhesive sealants recommended by the terminator and conduit manufacturers shall be used. Type 3 conduits shall be terminated in Types 1, 2, 3, 4S and 5S pull boxes bonded to a PVC sweep through the open bottom. Type 3 conduits shall be terminated in Types 4 and 5 pull boxes directly through cored holes in the side walls in accordance with Section 647.

682.2.08 CONDUIT, FIBERGLASS

A. MATERIAL

Conduit shall be manufactured from fiberglass reinforced epoxy. The conduit shall be “bullet resistant”, capable of preventing the penetration of a 0.45 caliber slug fired from a distance of 20 ft. (6 m). The conduit shall conform to the following physical and mechanical properties when tested in accordance with the referenced specification. All accessories and fittings, including outerduct couplings and expansion joints, shall meet all the same “bullet resistant” requirements as the conduit. All conduit and fittings shall be black.

<table>
<thead>
<tr>
<th>PHYSICAL AND MECHANICAL PROPERTIES</th>
<th>TEST METHODS</th>
</tr>
</thead>
<tbody>
<tr>
<td>Ultimate Tensile Strength - 11,000 PSI (75800 kPa) Min.</td>
<td>ASTM D 2105</td>
</tr>
<tr>
<td>Dielectric Strength - 500 Volts/Mil.</td>
<td>ASTM D 149</td>
</tr>
<tr>
<td>Water Absorption - 1% Max.</td>
<td>ASTM D 570</td>
</tr>
<tr>
<td>Specific Gravity - 1.9 - 2.0</td>
<td>ASTM D 792</td>
</tr>
<tr>
<td>Glass Content - 68 +- 2%</td>
<td>API SPEC 15 LR</td>
</tr>
<tr>
<td>Barcol Hardness - 58-52</td>
<td>ASTM D 2583</td>
</tr>
</tbody>
</table>

All conduit shall conform to the following requirements:

a. The conduit shall have the following identification information:

b. The duct shall be marked with data which will provide traceability of the manufacturer, plant location, date, shift, and machine of manufacturer.

c. Any additional wording on the conduit, such as “this side up” or “this side down”, shall be consistent with the installation orientation.

d. The spigot end of the duct shall have a circumferential insertion depth mark to insure that proper insertion depth is achieved. This mark is not required for spigots with threaded fittings.

1. 2 in. (51 mm) Conduit
   2 in. (51 mm) Conduit shall meet the following requirements:
   - Nominal outer diameter: 2.500 in. (tolerance +0.028”/-0.018”)
   - [63.50 mm (tolerance +0.71/-0.46)]
   - Minimum inner diameter: 2.000 in. (50.80 mm)
   - Minimum wall thickness: 0.250 in. (6.35 mm)
2. 4 in. (102 mm) Conduit

4 in. (102 mm) Conduit shall meet the following requirements:

- Nominal outer diameter: 4.500 in. (tolerance +0.034”/-0.028”)
- Minimum wall thickness: 0.250 in. (6.35 mm)
- Minimum inner diameter: 4.000 in. (101.6 mm)

B. COUPLINGS AND FITTINGS

Coupling shall be by epoxy adhesive interference joint with bell and spigot or stop coupling fittings only. Couplings shall be airtight and watertight. All couplings shall be installed in accordance with the conduit and the coupling manufacturer’s recommendations. Only couplings of the same type of fiberglass as specified above are permitted.

Fixed bends and sweeps shall be used; no flexible bends are permitted. Bends and sweeps shall be compatible with the coupling requirements above. Bends and sweeps shall be of consistent radius and inner diameter, with a minimum radius of 10 times the inner diameter. In no case shall bends exceed a 90 degree direction change.

Where the fiberglass conduit is specified in the Plans and/or by the Engineer to be attached to a bridge or other structure, bridge hanger assemblies, expansion joints, deflection fittings, and conduit support devices are required and shall be designed for application to the specific bridge or structure for which they will be used. The Department shall approve all materials and design of bridge-attached conduit systems prior to any field installation. All bridge hanger assembly components that are in contact with the conduit’s outer surface shall be manufactured of the same fiberglass reinforced epoxy material or shall employ low-friction roller bushings.

C. TERMINATION

Fiberglass conduits shall be terminated in ECBs using factory-installed terminators in the ECB or by grouting and setting in a knockout window as shown in the Standard Details of the Contract Documents. Adhesive sealants recommended by the terminator and conduit manufacturers shall be used.

682.2.09 PULL TAPE

A. MATERIAL

Non-detectable pull tape shall be a polyester tape (Fibertek Part No. WP1250, NEPTCO Part No. WP1250P, or approved equal). The tape shall have the following properties:

- 1250 lb (567 kg) tensile strength
- flat, not round, construction
- printed foot markings
- pre-lubricated for reduced pulling tension at start of cable pull
- low susceptibility to absorption of moisture; moisture resistant

Detectable pull tape shall consist of a single 24 AWG copper wire with polyethylene or PVC jacket woven into a polyester tape (Fibertek Part No. WPT1250, NEPTCO Part No. DP1250P, or approved equal). The tape shall have the following properties:

- 1250 lbs. (567 kg) tensile strength
- flat, not round, construction
- printed foot markings
- pre-lubricated for reduced pulling tension at start of cable pull
- low susceptibility to absorption of moisture; moisture resistant
- corrosion resistant embedded conductor

B. INSTALLATION

Install pull tape, by hand pulling, blowing, or via vacuum method, into each empty conduit and innerduct and empty cell within a multi-cell conduit during conduit installation. Install the pull tape after conduit testing has been completed. Install and secure 5 ft (1.5 m) of slacked pull tape in each empty conduit or cell at each vault. Secure the pull tape by tying it to the duct plug for the conduit in which it is installed.
682.2.10 DUCT PLUGS

Install blank duct plugs in each empty conduit that enters an ECB, pull box, hub, or building entrance. The plug shall be sized to fit the conduit in which it is used and shall provide a watertight and airtight seal by use of mechanical expansion. No sealants or caulks shall be used. The duct plug shall have inner rings to which pull tape can be tied. All metallic components shall be stainless steel.

Install a fiber optic innerduct plug in each conduit that enters an ECB, pull box, hub, or building entrance and has a cable installed in it. The plug shall be sized to fit the conduit and cable with which it is used and shall be a split plug with a bushing assembly for sealing around the cable by mechanical compression. All metallic components shall be stainless steel.

Install a multi-conduit duct plug in each conduit that enters an ECB, pullbox, hub, or building entrance and has one or more innerducts installed in it. The plug shall be sized to fit the outer conduit and the innerducts with which it is used and shall have split holes for each innerduct with a bushing assembly for sealing around the innerducts by mechanical compression. Seal unused innerduct holes with the appropriate plug or solid bushing. All metallic components shall be stainless steel.

Install a multi-cable duct plug in each conduit that enters an ECB, pullbox, hub, or building entrance and has two or more cables installed in it. The plug shall be sized to fit the outer conduit and the cables (with appropriately-sized split bushings) with which it is used and shall have split holes for each cable with an overall bushing assembly for sealing around the cable bushings by mechanical compression. Where the conduit is 4-inch I.D. or greater, use a multi-cable duct plug with a minimum of four cable holes. Seal unused cable holes with the appropriate plug or solid bushing. All metallic components shall be stainless steel.

682.2.11 CONDUIT DETECTION WIRE

A. MATERIAL
Conduit detection wire shall be #10 AWG stranded green-insulated THWN or THHN-THWN conductor.

B. INSTALLATION
Install one conduit detection wire in the trench during conduit installation, directly below the conduit or at the same level as the conduit. All conduit installed by use of directional boring shall include the installation of a conduit detection wire. The conduit detection wire shall be pulled with, but not in, the bored conduit. If more than one conduit is being installed in a single bore, only one conduit detection wire shall be required.

When conduit detection wire installation is required in existing conduit, install one conduit detection wire in the existing conduit or in one of the existing innerducts.

The conduit detection wire shall be continuous and unspliced between pull boxes or vaults and shall enter the pull boxes or vaults at the same location as the conduit with which it is installed. Coil and secure 5 ft (1.5 m) of conduit detection wire in each pull box or vault.

C. TESTING
Perform a continuity or tone test after installation to confirm that a continuous run of conduit detection wire was installed between pull boxes or vaults. For conduit detection wire installed in a trench, test the conduit detection wire after backfilling, compaction, and ECB installation is complete. For conduit detection wire installed in a trench with full-depth conduit backfill, test the conduit detection wire before and after backfilling. The purpose of this test is to document that no damage or separation of the conduit detection wire has occurred during the installation of wire, backfilling of the trench, or ECB installation.

Prepare a test plan, supplying equipment, conducting the test and documenting the results. Submit a test plan at least 15 working days prior to the desired testing date. Testing shall not begin until the Engineer has approved the test plan, and all tests shall be conducted in the presence of the Engineer.

682.2.12 CONDUIT TESTING
Test every conduit after the conduit is installed and before cable or pull tape is installed. Perform testing on all conduit types in this Specification, including but not limited to each cell of multi-cell conduit, each conduit in duct banks, and each innerduct. All testing shall be performed using the procedures and mandrel size recommended by the conduit manufacturer. Testing shall be performed in the presence of the Engineer. Payment for all testing is included in CONSTRUCTION COMPLETE.

682.2.13 ELECTRICAL COMMUNICATION BOX

Design electrical communication box and cover in accordance with ASTM C-857-95. Ensure that the walls, floor, and roof be minimum 6 in. thicknesses. Form electrical communication box from 4500 psi concrete in accordance with Section 830. Manufacture and install the electrical communication box in accordance with Details which include the dimensions associated with each type of electrical communication box. Seal all joints and seams in the electrical communication boxes created from manufacture or final assembly with manufacturer-approved sealant.

Form electrical communication box with one (1) knockout window and three (3) conduit terminators for conduit, nonmetal, type 3, 2 in. on each wall of the electrical communication box as shown in the Details. The knockout window shall remain sealed unless used for conduit duct bank termination. Provide 1 in. to 1.5 in. separation between conduit terminators. Install conduit into terminators as shown in Plans and seal with manufacturer-approved sealant.

Install two (2) cable racks, minimum 54 in. in length, on each wall of the electrical communication box as shown in the Details. Install cable racks directly to the wall or use the shortest standoff bracket possible. Include cable support arms, 7 in. to 9 in. in length, with plastic or ceramic insulators with each rack. Install one (1) cable support arm per rack for each cable installed plus one (1) cable support arm per rack as spare. Manufacture all cable racks, cable support arms, and mounting/fastening hardware of hot-dipped galvanized steel.

Install electrical communication box on a 12 in. layer of compacted coarse aggregate. Terminate conduit duct banks as shown in the Details. Prior to grouting, compact backfill for the entire length of trench to within 10 ft. of the electrical communication box. Bundle conduit, as shown on conduit duct bank installation Details, with cable ties, wire, or duct tape. Secure and align individual conduits of conduit duct bank with conduit alignment jigs, ensuring that the conduits enter the electrical communication box level, straight, and perpendicular to the wall. Construct conduit alignment jigs of plywood or use conduit spacers in accordance with Section 682. Allow grout around individual conduits of conduit duct bank to set prior to final backfilling and paving around the electrical communication box. Do not use concrete for any backfill around the electrical communication box or the conduit approaches to the electrical communication box within 10 ft.

Install electrical communication boxes in the shoulder lane whenever possible, unless shown otherwise in the Plans. In the case of narrow shoulder lanes where the electrical communication box extends beyond the edge of pavement, backfill to the top of the electrical communication box. Never install any portion of the electrical communication box in the travel lane.

Electrical communication box covers shall be imprinted with “GEORGIA DOT COMMUNICATIONS”.

682.2.14 ELECTRICAL COMMUNICATION BOX REHABILITATION

Establish the location of the electrical communication box, recognizing that pavement may have been placed over the cover of the electrical communication box.

Open the cover of the electrical communication box which may include the use of power tools to accomplish and the removal of pavement.

Remove existing fiber optic cable coils temporarily ensuring no kinks or abrasions are made to the fiber optic cable.

Clean the interior of the electrical communication box and remove any debris, trash, mud, silt, and water.

Reseal all joints and seams in the electrical communication box with silicone sealant, type A as specified in Section 833.2.06.
Install two (2) cable racks per wall for inside wall widths greater than or equal to 36 in. Install one (1) rack per wall for inside wall widths less than 36 in. but greater or equal to 24 in. Install no racks for inside wall widths less than 24 in.

Cable rack height shall be equal to inside height of the electrical communication box minus 6 in. Install cable racks such that the bottom of the cable rack is no greater than 3 in. from the bottom of the electrical communication box.

Install cable racks such that the distance between successive racks and the electrical communication box corners is equal to the extent permitted by the presence of knockout windows and/or conduit terminators. Install cable racks directly to the wall or use the shortest standoff bracket possible. Include cable support arms, 7 in. to 9 in. in length, with plastic or ceramic insulators with each rack. Install one (1) cable support arm per rack for each cable previously installed or being installed as part of the project plus one (1) cable support arm per rack as spare. Manufacture all cable racks, cable support arms, and mounting/fastening hardware of hot-dipped galvanized steel.

Re-set the electrical communication box and cover assembly such that the cover is at the elevation of the paved shoulder lane. Install class A concrete HES and 2 in. of 12.5 mm superpave or concrete surface to match existing paved shoulder.

Label any unlabeled fiber optic cables in accordance with labeling requirements set forth in Section 935.

If a suitable unused conduit terminator does not exist and a conduit is being terminated into an existing electrical communication box, neatly core conduit entry hole in electrical communication box wall and seal around conduit with silicone sealant or grout as necessary to prevent soil and/or water intrusion into the electrical communication box.

Add the following to Subsection 682.3.05:

A. Multi-Cell Conduit System
Secure from the manufacturer or supplier of the multi-cell system and provide to the Department complete and comprehensive written installation manuals for the complete system. At the start of the multi-cell installation, have the manufacturer or supplier provide technical assistance, as needed. At any time during the construction process, ensure that the manufacturer or supplier provides technical assistance to the Contractor and/or the Department.

For multi-cell conduit system installation under bridges, only fiberglass or steel multi-cell conduit systems shall be used. Install expansion and deflection joints according to the multi-cell conduit system manufacturer’s and support hanger manufacturer’s recommendations. Steel couplings shall be securely tightened; fiberglass coupling shall be epoxied. Ensure that during the construction process and at the request of the Department, the multi-cell conduit system or support hanger manufacturer provides on-site technical assistance at no additional cost to the Department.

B. Continuous Flexible Conduit
Whenever possible, conduits shall be placed in continuous manufactured lengths without coupling.

Conduit shall be placed in the straightest orientation possible, reducing bends, rises, and waves. Conduits shall be held in place during backfilling when necessary to keep straight. Where field conditions require the trench to change direction and bends are necessary, the bends shall be formed in the with the trench and should be smooth and gentle and shall not have less than a 4 foot radius (as measured to the inside surface of the conduit)

Delete Subsection 682.4 and substitute the following:

No items shall be measured separately

Multi-cell conduit system, innerduct, conduit duct bank, fiberglass conduit, and conduit nonmetal type 3 will not be measured separately for payment. Work shall include, but not limited to cutting asphalt or concrete, trenching, installing, backfilling trench, restoring asphalt or concrete, drilling existing concrete shoulder, installing #4 rebar, replacement of existing transverse joint material, directional boring, and testing of multi-cell conduit system, innerduct, conduit duct bank, conduit nonmetal type 3, fiberglass conduit, marking tape, pull tape, duct plugs, and conduit detection wire shall be included in the overall cost of the multi-cell conduit system, innerduct, conduit duct bank, fiberglass conduit, and conduit nonmetal type 3.

Conduit detection wire installed in existing conduit will not be measured separately for payment.
Conduit, nonmetal, type 2 – power service will be not be measured separately for payment. No separate measurement will be made for type 2 pull boxes, electrical junction boxes, electrical wire, directional bores, transformers, pavement repair, or any other required materials.

Electrical communication box, type ____ will not be measured separately for payment. No separate measurement will be made for, cable racks, cable support arms, compacted backfill material, compacted coarse aggregate, pavement removal, or pavement installation.

Electrical communication box rehabilitation will not be measured separately for payment by the electrical communication box that was rehabilitated as previously defined. No separate measurement will be made for cable racks, cable support arms, pavement removal, or pavement installation.

*Delete Subsection 682.5 and substitute the following:*

Payment for all items shall be made under CONSTRUCTION COMPLETE.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

Project Number: CSNHS-0006-00(043)
P.I. Number: 0006043
Cherokee County

Section 713—Organic and Synthetic Material Fiber Blanket

Delete Section 713 and add the following:

713.1 General Description
This work includes furnishing and placing straw, excelsior, coconut fiber, wood fiber, or synthetic blankets over previously prepared and grassed (temporary or permanent) areas as shown on the Plans or as directed by the Engineer.

713.1.01 Definitions
- Straw Blanket: A machine-produced blanket of clean, weed-free, consistently thick straw from agricultural crops. The straw is evenly distributed over the entire area of the blanket.
- Excelsior Blanket: A machine-produced mat of curled wood excelsior. Eighty percent consists of 6 in (150 mm) or longer fiber evenly distributed over the entire blanket.
- Coconut Fiber Blanket: A machine-produced blanket of 100 percent coconut fiber evenly distributed over the entire blanket.
- Wood Fiber Blanket:
  - Type I—A machine-produced blanket manufactured with reprocessed wood fibers to a consistent thickness.
  - Type II—A hydraulically applied bonded fiber matrix which upon drying, adheres to the soil in the form of a continuous 100 percent coverage, biodegradable erosion control blanket
- Synthetic Fiber Blanket—A machine produced uniform blanket of ultraviolet degradable polypropylene staple fibers reinforced with ultraviolet degradable polypropylene netting.

713.1.02 Related References
A. Standard Specifications
   General Provisions 101 through 150.
B. Referenced Documents
   General Provisions 101 through 150.

713.1.03 Submittals
Use approved materials from QPL 62 without further testing. Otherwise, submit materials for testing before use.
Section 713—Organic and Synthetic Material Fiber Blanket

713.2 Materials
Use blankets that meet the following requirements for placement on slopes and waterways. For a list of organic material fiber blankets, see QPL 62.

A. Straw Blanket
Use blankets at least 48 in (1.2 m) wide and at least 3/8 in (9 mm) thick with a minimum dry weight of 0.5 lb/yd² (270 g/m²) and a stitch pattern and row spacing of no more than 2 in (50 mm).

Have the top side covered with a photo-degradable plastic mesh having a maximum mesh size of 1/2 in by 1/2 in (13 mm by 13 mm). Sew the mesh to the straw with biodegradable thread.

Use this blanket on slopes only.

B. Excelsior Blanket
Use a smolder-resistant blanket with the top side clearly marked. Use a blanket at least 48 in (1.2 m) wide and 1/4 in (6 mm) thick with a minimum dry weight of 0.8 lb/yd² (430 g/m²) and a stitch pattern and row spacing of no more than 2 in (50 mm).

- Slopes: Have the top side covered with a photo-degradable plastic mesh having a maximum mesh size of 1-1/2 by 3 in (38 by 75 mm).
- Waterways: Have the top and bottom sides of the blanket covered with a photodegradable plastic mesh having a maximum mesh size of 1 ½ x 3 in (38 x 75 mm), sewn to the fiber with biodegradable thread or otherwise bonded as approved by the Engineer.

C. Coconut Fiber Blanket
Use a blanket at least 48 in (1.2 m) wide and 1/4 in (6 mm) thick with a minimum dry weight of 0.5 lb/yd² (270 g/m²) and a stitch pattern and row spacing of no more than 2 in (50 mm). Use the blanket in waterways only.

Ensure that both sides of the blanket are covered with a photo-degradable plastic mesh with a maximum of 5/8 by 5/8 in (19 by 19 mm). Have the mesh sewn to the fiber with a breakdown-resistant synthetic yarn.

D. Wood Fiber Blanket
Type I
- Use a machine produced blanket manufactured to a consistent thickness using reprocessed wood fibers.
- Use a blanket at least 48 in (1.2 m) wide with a minimum dry weight of 0.35 lb/yd² (190 g/m²).
- Ensure that the top side of the blanket is covered with a photo-degradable plastic mesh with a maximum of 5/8 x 3/4 in (16 x 19 mm) securely bonded to the mat.
- Ensure the fibers do not contain a growth that inhibits germination.
- Use Type I blanket on shoulder construction on resurfacing projects.

Type II
For Shoulder Construction on Resurfacing Projects
- Ensure the bonded fiber matrix is composed of long strand wood fibers or cellulosic-based fibers held together by a bonding agent, which, upon drying, becomes insoluble and non-dispersable.
- Apply the matrix at the following at the rate of 3000 lbs/acre (3.4 Mg/ha) for shoulder construction on resurfacing projects.
- Do not apply the bonded matrix on saturated soils or immediately before, during or after rainfall. Allow the matrix to dry for at least 24 hours after installation. After drying period, ensure that the bonded fiber matrix does not inhibit the germination or growth of plants beneath and through the formed matrix blanket and that it does not form a water insensitive crust.
- If bonded fiber matrix is to be used, the application of straw mulch for grassing operations is not required.
Section 713—Organic and Synthetic Material Fiber Blanket

E. Synthetic Fiber Blanket

Use a blanket having a minimum net size of 5/8 x 3/4 inch (16 x 19 mm). Ensure the netting is securely bonded to the blanket and that the blanket conforms to the following physical properties:

<table>
<thead>
<tr>
<th>PROPERTY</th>
<th>MINIMUM VALUE</th>
<th>TEST METHOD</th>
</tr>
</thead>
<tbody>
<tr>
<td>Weight</td>
<td>1 oz/sq. yd (34 g/m²)</td>
<td></td>
</tr>
<tr>
<td>Roll Width</td>
<td>48 inch (1.2 m)</td>
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</tr>
<tr>
<td>Tensile Strength</td>
<td>6 lbs./in</td>
<td>ASTM D 1682 [6” (150 mm strip)]</td>
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</table>

Use Synthetic fiber blanket on slopes only.

F. Net-free Excelsior Blanket

Use only on 4:1 and flatter slopes.

Use a smolder-resistant blanket at least 48 in (1.2m) wide and ¼ in (6mm) thick with a minimum dry weight of 0.73 lb/yard² (390 g/m²).

Use a blanket consisting of curled, interlocking, Aspen fibers with a cross section of 0.038± 0.010 in X 0.018± 0.003 in (0.97±0.25mm X 0.45±0.08mm). Ensure that a minimum of 80% of the fibers are at least 6 in (150mm) long. Ensure that the fibers are stitched with biodegradable thread in a 2 in X 4 in ± 0.5 in (50mm X 100mm ±13mm) pattern.

G. Anchoring Staples

Use anchoring staples made from minimum 11-gauge wire, formed into a U shape. Ensure that the legs are at least 6 in (150 mm) long and the crown at least 1 in (25 mm) wide. Use staples rigid enough to penetrate the soil without distortion.

713.3 Construction Requirements

713.3.01 Personnel
General Provisions 101 through 150.

713.3.02 Equipment
General Provisions 101 through 150.

713.3.03 Preparation
Before placing the blanket, complete the grassing operations, smooth the area, and remove stones, lumps, roots, or other material that would prevent the blanket from laying snugly on the soil.

713.3.04 Fabrication
General Provisions 101 through 150.

713.3.05 Construction
A. Placing Blanket

Place blankets or mats vertically on slopes, beginning at the top of the slope and extending to the bottom of the slope. Horizontal installation of the blankets is not permitted.

Place the blanket within 24 hours after planting and before rain or watering. Place the blanket on slopes and waterways as follows:

1. On Slopes

Unroll the blanket with the netting on top and the fibers contacting the soil over the entire slope. When using two or more blankets to cover an area, overlay the joint 4 in (100 mm) and staple through the joint. Overlap the ends of the blanket at least 6 in (150 mm) with the upgrade section on top and staple through the overlap.
Section 713—Organic and Synthetic Material Fiber Blanket

2. In Waterways

In waterways, ditches, flumes, and channels unroll the blanket with netting sewn on both sides and place in contact with the soil beginning at the downstream terminal and progressing upstream of the blanket according to the Construction Detail for Permanent Soil Reinforcing Mat.

Allow a longitudinal seam only if the blankets overlap at least 6 in (150 mm) and are securely stapled. Overlap ends of the blanket at least 6 in (150 mm) with the upgrade section on top.

Insert 12 in (300 mm) of the upslope end of the first row of blankets into a 6 in (150 mm) deep anchor slot. Staple the blanket in the slot bottom, backfill the slot, and solidly tamp.

B. Stapling

Drive staples vertically into the ground to anchor the plastic mesh. Place the staples approximately 2 yd (2 m) apart on each side of the blanket and add one row in the center alternately spaced between each side staple.

Where blankets lay side to side, place each staple so that half of the staple anchors mesh from each blanket.

At the beginning of a blanket, space staples approximately 12 in (300 mm) apart in a row.

C. Steep Slopes

The Engineer may specify additional staples or check slots in waterways where slopes are steep or large water volumes and/or velocities are anticipated.

713.3.06 Quality Acceptance

General Provisions 101 through 150.

713.3.07 Contractor Warranty and Maintenance

Maintain the blanket installation throughout the life of the Contract. If before Final Acceptance any staples become loose or lift up or if the blanket becomes loose, torn, or undermined, then fix the problem by reshaping, regrassing, refertilizing, or replacing damaged areas. Repairs are done without additional compensation.

713.4 Measurement

No measurement to be included for this item.

713.4.01 Limits

General Provisions 101 through 150.

713.5 Payment

The preliminary preparation of the areas on which the blanket is to be placed, including seeding or sodding, will be paid for under CONSTRUCTION COMPLETE.

Straw blanket excelsior blanket, coconut fiber blanket, Type I wood fiber blanket (slopes) or synthetic fiber blanket will be paid for under CONSTRUCTION COMPLETE. Wood Fiber Blanket Type I & Type II (shoulders) will be paid for under CONSTRUCTION COMPLETE. Includes construction of the Item including all laps, materials, equipment, tools, labor, incidentals, and maintenance.
Add the following:

936.1 General Description

This work includes furnishing and installing closed circuit television (CCTV) system, any specified type, which is a CCTV video surveillance field installation, including but not limited to color CCTV cameras, lens, housing, pan/tilt drive, camera system assembly, cabling, mounting hardware, interface panel, camera control receiver, and cabinet wiring. This CCTV system provides operator control from and video imaging to the Department’s NaviGAtor Advanced Transportation Management System (ATMS).

Provide all equipment and materials of like kind and function to be of the exact same manufacture, model, revision, firmware, etc.

Provide all equipment, materials, and work in accordance with all manufacturers’ recommendations, including but not limited to all mounting, wiring and cabling, power supply, surge suppression, and communications equipment and materials.

936.1.01 Definitions

CCTV System, Type B – The Type B CCTV System uses a self-contained camera system assembly with an analog NTSC video output and RS-232 serial data control interface.

Type A Cabinet – As specified in Section 939, the Type A cabinet housing is a standard Model 336 stretch (336S) housing with approximate exterior dimensions of 46 in (H) by 24 in (W) by 23 in (D).

Type B Cabinet – As specified in Section 939, the Type B cabinet housing is a standard Model 337 housing with approximate exterior dimensions of 35 in (H) x 20 in (W) x 17 in (D).

Type C Cabinet – As specified in Section 939, the Type C cabinet housing is a standard Model 332 housing with approximate exterior dimensions of 64 in (H) by 24 in (W) by 30 in (D).

Type D Cabinet – As specified in Section 939, the Type D cabinet housing is a standard Model 336 stretch (336S) housing with approximate exterior dimensions of 46 in (H) by 24 in (W) by 23 in (D).

Type E Cabinet – As specified in Section 939, the Type E cabinet housing is a standard Model 336 stretch (336S) housing with approximate exterior dimensions of 46 in (H) by 24 in (W) by 23 in (D).

936.1.02 Related References

A. Georgia Standard Specifications

Section 639 - Strain Poles for Overhead Sign and Signal Assemblies
Section 680 - Highway Lighting
Section 682 - Electrical Wire, Cable and Conduit
Section 922 - Electrical Wire and Cable
Section 923 - Electrical Conduit
Section 925 - Traffic Signal Equipment
Section 939 - Communication and Electronic Equipment

B. Referenced Documents

American National Standards Institute (ANSI)
American Society of Testing and Materials (ASTM)
Electronic Industries Association (EIA)
FCC Rules Part 15, Sub-part J
Insulated Cable Engineers Association (ICEA)
International Municipal Signal Association (IMSA)
MIL-HDBK-454A
MIL-STD-810F(3) Method 509 Procedure 1 – exterior salt atmospheres
National Electric Code (NEC)
National Electrical Manufacturers Association (NEMA)
NEMA-4
Underwriter’s Laboratory Incorporated (UL)
National Television Standards Committee (NTSC)

936.1.03 Submittals

This subsection and the following chart provide the Contractor with an outline of the submittal requirements for the equipment and components for all pay items in this Section 936. This chart is to be used as a guide and does not relieve the Contractor from submitting additional information to form a complete submittal package.

<table>
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<tr>
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<tr>
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<td>60 Days</td>
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</table>
Submit submittal data for all equipment, materials, test procedures, and routine maintenance procedures required for these items within sixty (60) calendar days after the Notice To Proceed and prior to any installation, unless noted otherwise in the Contract Documents.

Submit to the Engineer for approval, six (6) copies of the manufacturer’s descriptive literature (Catalog Cuts), Technical data, operational documentation, service and maintenance documentation and all other materials required within these specifications.

Provide submittal data that is neat, legible, and orderly. Neatly organize each package of submittal data and separate by hardware item. Use the “Materials Certification Package Index and Transmittal Form”, contained in Section 105.02 of the Special Provisions, for each pay item to document and list all material and components that are included in the submittal package. Any submittal data submitted without the Index/Transmittal form or that is incomplete will be rejected.

A. CCTV System, Type B; CCTV System, Type B, Retrofit Assembly; CCTV System, Type B, Retrofit Assembly (Furnish Only)

1. Camera System Assembly
Submit complete physical, performance, and operational materials submittal data for the camera system assembly and all associated components.

2. Camera System Assembly Mount
Submit complete physical, performance, and operational materials submittal data for the camera system assembly mount and all associated components and hardware.

3. Cabinet Interface Assembly
Submit complete physical, performance, and operational materials submittal data for the cabinet interface assembly and all associated components and hardware. Submit complete documentation for the CCTV system manufacturer’s Embedded Protocol as implemented between the camera system assembly and the user control interface, and submit the setup and operations procedure for changeover between the NaviGAtor Standard CCTV Control Protocol and the CCTV system manufacturer’s Embedded Protocol and vice versa. Submit the CCTV Embedded Protocol and control software application on CD-ROM with printed user documentation.

4. Cabling and Connectors
Submit complete physical, performance and operational materials submittal data for all cables, wire and connectors required for a complete and operational CCTV system. Submit cables and connectors as specified here and as recommended by the CCTV system manufacturer. Submit all CCTV system wiring diagrams and documentation as is required for as-built drawings in Subsection 936.3.05.B. Submit cabinet layout diagrams of all CCTV equipment. Submit CCTV manufacturer recommendations for all wiring and cabling, including but not limited to surge suppression and grounding/bonding. Submit CCTV manufacturer recommendations for the longest recommended cable distance between the Camera System Assembly and the CCTV interface equipment in the equipment cabinet; this distance shall be stipulated in consideration of all of the associated components and incidental materials, including but not limited to the system electronics design, cable materials, surge suppression, and power supply.

5. Submittal Review Demonstration Test Set
Submit demonstration test set(s) for Department evaluation after the Engineer approves the submittal materials for the equipment and materials listed below. The demonstration test sets shall be connected to and operated through the NaviGAtor system by the Engineer. Deliver the test set to the Department at the location specified by the Engineer. Request a delivery and test time a minimum of 30 days in advance. Provide demonstration test sets of the materials, types and quantities as shown below:

- CCTV System, Type B (quantity 2)
- CCTV System, Type B, Retrofit Assembly (quantity 2)
A demonstration test set shall include all materials, components, assemblies, control software and documentation of a CCTV system, Type B, (and/or Retrofit Assembly) and shall be a complete, fully functional CCTV system. The camera system assembly and the camera system assembly mount shall be attached to a desktop stand that securely holds the camera system assembly while the camera is being operated. The cabinet interface assembly may be mounted to the desktop stand or to separate panel. Cables C1, C2, C3, C4, C5, and C6 shall be connectorized and terminated as required in these Special Provisions. Provide TB18 and SS15. Provide a NEMA 5-15 cord that provides power to the cabinet interface assembly and communications equipment. Provide a terminal block or strip for connection of the RS232 communications signal to the communications equipment (e.g., video encoder serial port); label each terminal position for the conductor function. Provide a video decoder, a minimum 10 ft (3 m) coaxial video cable and a minimum 13 in color video monitor for viewing the video output. Serial (PTZ) communications to the CCTV System shall be provided through the serial port on the video encoder.

If the contract shows CCTV Systems to be installed in conjunction with video encoders/decoders and field switches, provide demonstration test sets in accordance with Section 939 for video encoders, video decoders, and field switches simultaneously with the CCTV System demonstration test sets.

Review of the demonstration test set submittal shall be conducted in two parts. The first part of the review shall be performed by the Contractor in the presence of the Engineer and shall include the setup and configuration of the demonstration test set on the NaviGAtor system. The first part of the review shall be conducted during normal Department weekday business hours and shall be conducted for the period of time necessary to the satisfaction of the Engineer. The second part of the review shall be a 60-day period during which the Engineer shall operate and evaluate the demonstration test set with the NaviGAtor system. The second part of the review shall commence only upon the Engineer’s approval of the first part of the review. Retrieve the demonstration test set upon completion of the second part of the review as notified by the Engineer.

For the first part of the demonstration test set review, the Contractor shall furnish and operate a serial data protocol analyzer to monitor and record the bi-directional RS232 communications session between the NaviGAtor session and the CCTV system. The protocol analyzer shall record all communications messages and shall time-stamp and record each byte of communications. Time-stamp resolution shall be no coarser than 0.10 millisecond. Provide recordings of all communications in the form of an MS-DOS-formatted ASCII text file. The protocol analyzer shall not be provided for the second part of the demonstration test set review, and the Contractor shall remove it at the end of the first part of the review.

B. Acceptance Testing

Submit acceptance test procedures and a desired acceptance test schedule.

C. Warranties and Guarantees

Submit materials submittal data providing complete example documentation on all manufacturers’ warranties or guarantees on all CCTV system equipment and hardware components furnished, as required in Subsection 936.3.07.

D. Training

Prior to training, submit resume and references of instructor(s). Obtain approval from the Engineer that the instructor is qualified in his/her respective field. Submit the Training Plan within 120 days of the notice to proceed. Include in the training plan an outline of the training course. Obtain approval of the Training Plan from the Engineer. The Training Plan shall explain in detail the contents of the course and the time schedule of when the training shall be given. Coordinate actual training with installation schedules as approved by the Engineer.

936.2 Materials

936.2.01 CCTV System, Type B

Ensure that the individual components and assemblies of the CCTV System, Type B, conform to the requirements specified in the following sections. Ensure that all equipment, materials, components and assemblies of the CCTV System Type B conform to the CCTV manufacturer’s requirements and recommendations.

A. Camera System Assembly

Follow these minimum requirements for a dome-enclosed camera system assembly including the camera, lens, pan/tilt drive, and control electronics.
Provide a downward-looking circular dome-shaped enclosure assembly. The enclosure shall have a maximum diameter of 14 in (356 mm) at its widest point and a maximum height of 22 in (559 mm) from the top of the housing assembly to the bottom point of the dome. The upper housing shall be constructed of a non-metallic UV-stabilized material of a light tan, gray, or white color, or constructed of an aluminum material with a heat-cured paint coating of an equivalent color. The lower housing shall be constructed of a UV-stabilized optically-correct acrylic material. The maximum weight of the complete and fully functional camera system assembly, including the camera, lens, pan/tilt drive, control electronics, environmental control components, housing assembly, and hub adapter shall be 25 lbs (11.4 kg).

Use an enclosure assembly that secures to the mounting bracket arm with a 1-1/2 in (37.5 mm) threaded pipe nipple. Hub adapters for the threaded pipe nipple on either the enclosure or the mounting arm, or both, are permitted.

All fastening and mounting hardware on or within the enclosure assembly shall be stainless steel.

Use a pressurized enclosure assembly that uses extra dry grade nitrogen. Provide a pressure relief valve and a Schrader valve for filling and evacuating the enclosure. An additional pressure relief screw is allowed. All mounting and wiring connections shall be pressure-tight. Ensure an operating pressure range of 3-7 psi.

Electrical power for the complete camera system assembly shall be per the manufacturer’s recommendations and between 12V to 120V DC or single-phase AC utilizing a two-wire (not counting ground) supply from the cabinet interface assembly in the equipment cabinet. Do not use a dual-voltage power supply. Maximum electrical load with all subsystems operational, including the heater, shall be no more than 130 VA.

Use an enclosure assembly with a heater and a circulating blower fan for environmental (temperature and defogging) control. Maximum electrical load for the heater shall be no more than 80 VA.

Use a camera unit with an integrated camera sensor and zoom lens assembly and an analog NTSC-compliant composite video output with a signal-to-noise ratio of 45dB or greater. The camera shall use an 1/4-inch interline transfer CCD image sensor, with a minimum of 768 horizontal by 492 vertical active picture elements. All elements shall remain operational. The camera shall have a minimum resolution of 460 horizontal TV lines by 350 vertical TV lines. The camera shall include on/off selectable automatic gain control and manual/automatic selectable white balance. The camera shall include an electronic shutter mode with user-selectable speeds of a minimum range from 1/60 second to 1/10,000 second. The camera unit shall provide an on/off selectable day/night function where the image sensing and output automatically switch between color and black-and-white imaging; fixed color or black-and-white imaging shall be user-controllable. The camera sensitivity shall be no less than 3.0 lux in color mode (1/60 second) and 0.5 lux in black-and-white mode (1/60 second, IR cut removed).

Provide a camera unit with an integrated zoom lens of a minimum of 22X optical zoom and a minimum of 4X digital zoom. The camera shall not employ any digital zoom functionality unless the lens is at the full limit of optical zoom and the zoom command continues to be applied, in which case the camera unit shall automatically switch from optical to digital zoom. The optical focal length shall be approximately 4 mm to 80 mm. The camera unit shall include on/off selectable automatic focus and manual/automatic selectable iris control.

Use a pan/tilt drive for the camera unit that is fully-contained within the enclosure assembly. The drive shall be capable of 360 degree panning and at least 0 degree horizontal to 90 degree vertical looking downward tilting. The camera unit and pan/tilt drive shall provide automatic 180-degree image output flip at the bottom of the tilt travel. The camera unit and pan/tilt drive shall provide a minimum of eight privacy blackout zones, each zone being individually programmable to be on/off by the user. The panning speed, when a pan-left or pan-right command is applied by the user, shall be between 10 and 18 degrees per second. The tilting speed, when a tilt-up or tilt-down command is applied by the user, shall be between 4 and 10 degrees per second.

Provide an system control interface to the camera system assembly that physically and logically supplies the user commands to and monitoring from the camera system assembly, including but not limited to pan, tilt, zoom, focus, position reporting, and configuration. The system control interface shall physically connect the camera system assembly to the cabinet interface assembly through cables C1 and C3 as specified below. Do not use a system control interface through cable C2. Provide a system control interface as RS422 or RS485 serial data communications of any messaging format that is in compliance with all of the physical and operational requirements specified for a CCTV System, Type B. Provide the capability to set the communications address used by the Standard Protocol (subsection 936.2.01.C) through the system control interface or through the user control interface in the cabinet; do not require the opening/disassembly of the camera system enclosure to set the communications address. Store all user configurable settings in non-volatile memory that is retained indefinitely upon loss of power.
Provide integral surge suppression features in the camera system assembly, within the dome enclosure, for all ungrounded conductors of the power, communications, and video signal lines. This surge suppression is secondary to the surge suppression in the equipment cabinet. Surge suppression technology shall as a minimum be low capacitance implementations of components such as gas tube, metal oxide varistors (MOV), isolation transformer, silicon avalanche/zener/transorb diode, positive temperature coefficient (PTC) resettable fuse, opto-isolator, or equivalent, or a hybrid of these components.

B. Camera System Assembly Mount

Provide a camera system assembly mount that includes a mounting bracket arm, camera enclosure mount and disconnect, mounting straps, and incidental fastening hardware. All fastening and mounting hardware shall be stainless steel.

The mounting bracket arm shall be suitable for pole-mounted applications using mounting straps or bolts. The bracket shall be fabricated to exactly mate with the camera enclosure mount/disconnect/pipe nipple and any needed pole-mount adapter with no drilling or welding required. The bracket shall be fabricated from aluminum alloy with an exterior polyurethane coating, stainless steel, or mild steel with a heat-cured paint coating. All bracket coatings shall be beige, tan, gray or off-white in color and corrosion resistant in accordance with MIL-STD-810F(3) Method 509 Procedure 1 for exterior salt atmospheres.

Use a mounting bracket arm that locates the vertical centerline of the camera enclosure from 14 in (356 mm) to 24 in (610 mm) from the exterior surface of the support pole. The mounting bracket arm shall provide for cable entrance through the base of the bracket directly from the support pole and from the exterior through a raintight opening on the underside of the bracket and adjacent to the support pole. Provide non-metallic cable protection grommets for both cable entrances.

Use a mounting bracket arm that fully encloses the cable connectors J1 and J2 and the manufacturer’s connector on the camera enclosure for cable C3. Provide a mounting bracket arm with a minimum interior cross-sectional area of 5 square in (32 square cm.) Provide a raintight access opening (minimum 2 in by 12 in (5 cm by 30 cm) to cable connectors J1 and J2 on the underside of the bracket with captive attachment hardware and access cover retaining lanyard. Provide a raintight access to the camera enclosure connector for cable C3 with captive attachment hardware and access cover retaining lanyard.

Include a camera enclosure mount and disconnect on the mounting bracket arm that secures to the camera enclosure with a 1-1/2 in (37.5 mm) threaded pipe nipple. Hub adapters for the threaded pipe nipple on either the enclosure or the mounting arm, or both, are permitted. When installed and fastened, the completed mounting mechanism assembly shall rigidly connect the camera enclosure to the mounting bracket arm with no movement relative between the enclosure and the arm. Provide a disconnect means for dismounting and remounting the camera enclosure from the mounting bracket arm with the use of threaded fasteners or locking pinned/slotted/keyed attachment mechanism and without the need to unthread the 1-1/2 in (37.5 mm) pipe nipple. The disconnecting means shall be compatible with the camera system manufacturer cable C3 and connector.

Unless otherwise shown in the Plans, mount the bracket arm to the support pole using a minimum of two 1/2 in (12.5 mm) or greater stainless steel mounting straps.

C. Cabinet Interface Assembly

Use a cabinet interface assembly that provides electrical service for the camera system assembly and provides the user control interface connection to the NaviGAtor system and/or user personnel. Install the cabinet interface assembly in the equipment cabinet. All fastening and mounting hardware shall be stainless steel. The cabinet interface panel assembly includes the following:

- CCTV Interface Enclosure
- Camera System Assembly Power Supply with surge suppression
- terminal blocks and video cable surge suppression for camera system assembly cabling
- user control interface to the NaviGAtor system and/or user personnel with surge suppression

Use the CCTV Interface Enclosure to hold the Camera System Assembly Power Supply and the user control interface unit for the camera electrical service and system control interface data path as shown in detail drawing 936.1a and 936.1b. The CCTV Interface Enclosure shall be rack mountable on a standard 19” rack and located in cabinet rack as shown in detail drawing 936.2a and 936.2b. The CCTV Interface Enclosure height shall not exceed 3 Rack Units, and it shall not exceed a depth of
15 inches. The CCTV Interface Enclosure shall provide the connector ports as defined below; unless otherwise specified, all connector ports shall be pluggable polarity-keyed terminal blocks with touch-safe design (Phoenix Contact Combicon blocks or approved equivalent) and rated for the voltage and current of the application. All connector ports shall be labeled as specified here and shall be located on the rear of the CCTV Interface Enclosure. The CCTV Interface Enclosure shall be labeled with the manufacturer model and part number of the camera system assembly with which the enclosure is supplied.

Provide a Camera System Assembly Power Supply in the CCTV Interface Enclosure that supports all electrical service needs for all components and subsystems of the camera system assembly and CCTV Interface Enclosure. Connect the line and neutral inputs of the CCTV Interface Enclosure power connector port labeled “CP Power” to the equipment cabinet’s 120VAC electrical supply on terminal block TB2 as shown in the CCTV system detail drawings, and use standard conductor insulation colors for 120VAC electrical service (black, white, and green). Provide a copper or copper alloy grounding lug on the CCTV Interface Enclosure and bond all internal components to the grounding lug; connect the grounding lug to the cabinet ground busbar. Connect the power supply output of the CCTV Interface Enclosure, labeled “CP1”, to the camera system assembly directly to the terminals of TB18, connecting to cable C1. Use only stranded copper wiring, with a minimum #12 AWG for the line and neutral conductors of the 120VAC circuit to TB2, and a minimum wire size recommended by the CCTV system manufacturer for the power supply circuit to TB18 but not less than #18 AWG. Ground the input and output sections of the power supply in accordance with the CCTV system manufacturer’s recommendations. The power supply shall not have any fusing for any circuit except for socket/holder mounted fusing. The power supply shall include a switch located on the front of the CCTV Interface Enclosure to disconnect power from the camera system assembly. The power supply and user control interface unit shall be sized so that they can fit inside the CCTV Interface Enclosure.

Provide a transient voltage surge suppressor for the power conductors to the camera system assembly. Use a surge suppressor that is independent of and external to the Camera System Assembly Power Supply. Use a surge suppressor rated for the voltage, current, and polarity of the Camera System Assembly with MOV/diode/thermal fusing hybrid technology and a -40C to +75C operating temperature.

Provide terminal block TB18 for the termination of cable C1 from the camera system assembly and the termination of wiring from the power supply and the user control interface unit. Mount TB18 on the equipment cabinet side panel as shown in the CCTV system detail drawings, not in the CCTV Interface Enclosure. Use a 14-position dual-screen barrier type terminal block with 7/16 in (11.11 mm) spacing using nickel-plated brass 6-32 philslot screw (Cinch 141 or approved equivalent) and quick-clamp type wire terminals (Cinch QC-1 or approved equivalent). Use a terminal block with voltage and current ratings greater than the voltage and current ratings of the wires, which are terminated on the block. Provide an EMI/RFI shield over the terminal block if recommended by the CCTV system manufacturer. Do not use compression-type, tubular clamp, or spade lug terminals. Provide a connector port on the CCTV Interface Enclosure for connection to TB18; label this port “CP2.”

Provide surge suppressor SS15 for the protection and termination of the coaxial video cables C2 and C4. Mount SS15 on the equipment cabinet side panel as shown in the CCTV system detail drawings, not in the CCTV Interface Enclosure. Use a shielded gas-tube and diode hybrid technology or equivalent with 6-volt line-to-ground clamping, 20kA peak surge current, -40C to +75C operating temperature, and BNC coaxial connectors. The maximum dimensions of SS15 are 4 in L x 2 in H x 1.5 in W (102 mm L x 51 mm H x 38 mm W.) Use a minimum #16 AWG stranded copper insulated green wire to ground SS15 directly to the cabinet ground bussbar.

Provide a user control interface on the CCTV Interface Enclosure to provide for control and monitoring communications between the camera system assembly and the NaviGAtor system user. The user control interface port shall be a DB9 connector labeled “CP4” that matches the RS-232 cable provided with the CCTV Embedded Protocol control software required below. The user control interface unit shall fully support control and monitoring communications from the NaviGAtor Standard CCTV Control Protocol and from the CCTV system manufacturer’s own embedded protocol; simultaneous operation of these two protocols is not required. The user control interface unit shall be a protocol switching or translation, port-sharing or port-coupling, or similar device that controls and monitors the camera system assembly through its system control interface.

The NaviGAtor Standard CCTV Control Protocol (hereinafter called the “CCTV Standard Protocol”) is specified below and shall connect to the user control interface unit through an RS-232 serial data interface directly from the NaviGAtor system. Connect the user control interface inside the CCTV Interface Enclosure to the CCTV Standard Protocol with cable C5 through the connector port labeled “CP3”. Provide the CCTV system manufacturer’s proprietary embedded protocol (hereinafter called the “CCTV Embedded Protocol”) as a Microsoft Windows-based PC software application with user interface. Provide the CCTV Embedded Protocol and control software application on CD-ROM with printed user documentation. The CCTV Embedded Protocol control software shall be capable of fully operating the camera system assembly and user control interface unit, including but not limited to control, monitoring, and configuration. Operate the
control software through a cable connection between the user control interface unit and an RS-232 serial data interface on a Windows-based PC. Provide the control software with a properly configured RS-232 cable (minimum length 6 ft) with a female DB-9 connector on the cable end attached to the computer and a pre-configured connector or termination on the end attached to the user control interface unit. Provide the control software with an unrestricted, non-cancelable user license for the Department’s use with any NaviGAtor equipment at any location. Furnish three copies each of the software, license, appropriate RS-232 cable, and user documentation per project.

Changeover between CCTV Standard Protocol and CCTV Embedded Protocol control and monitoring communications with the camera system assembly shall occur only through the user control interface unit. There shall be no requirement to physically access the camera system assembly or to reconfigure any wiring between the cabinet interface assembly and the camera system assembly. Use of different interface ports, or use of the same port, on the user control interface unit for the two protocols is permitted provided that the CCTV Embedded Protocol port is connectorized. Configuration changes to the user control interface unit to switch operation between the two protocols is not permitted, including but not limited to dipswitch or jumper settings.

Connect the user control interface unit, using cable C6, to TB18 for connection through C1 to the camera system assembly. Use a single one, two or three pair multi-pair cable of stranded copper wiring, minimum #18 AWG, twisted and/or shielded as recommended by the CCTV system manufacturer, and label this cable C6. Ground or bond any C6 cable or pair shielding and any unused conductors in accordance with the CCTV system manufacturer’s recommendations. Do not connect unused conductors of C1 to the user control interface unit unless recommended by the CCTV system manufacturer. Connect C6 directly to the terminating port on the user control interface unit; do not use any port adapters, gender changers, couplers, or similar components. Provide an automatically-resetting surge suppressor for each connected ungrounded conductor between the user control interface unit and TB18. Use a surge suppressor with low capacitance gas-tube and diode hybrid technology or equivalent with a 10kA peak surge current (8x20 waveform) and -40°C to +75°C operating temperature.

Connect the user control interface unit to the Video Encoder port with cable C5. Use a single multi-conductor or multi-pair cable with minimum stranded copper #18 AWG, with twisted pairs and/or shielded/bonded as recommended by the CCTV system manufacturer, and label this cable C5. Ground or bond any C5 cable or pair shielding and any unused conductors in accordance with the CCTV system manufacturer’s recommendations. Cable C5 shall be pre-connectorized on one or both ends as required for the user control interface and the Video Encoder port configurations and as required by the CCTV system manufacturer. Connect C5 directly to the terminating ports on either end; do not use any port adapters, gender changers, couplers, or similar components.

D. CCTV Standard Protocol

1. General

Use the NaviGAtor Standard CCTV Control Protocol (hereinafter called the “CCTV standard protocol” or “standard protocol”) for CCTV system control communications between the user control interface unit and the NaviGAtor central system. Implement the standard protocol as specified in this subsection for all CCTV equipment installed on this project. The CCTV standard protocol governs all control communications between the NaviGAtor central system (hereinafter called the “host”) and the CCTV system specified in this Subsection 936.2.01 (hereinafter called the “remote”). Do not implement any other control communications except as required and specified in this Subsection 936.2.01.C.

2. Interface

The remote shall communicate using the CCTV standard protocol over an RS-232 serial interface. Data is transmitted using 1 start bit, 8 data bits, and 1 stop bit (no parity) at a baud rate of 9600.

Each remote shall be identified by a unique integer address between 1 and 233. This address is set during installation and shall not be altered using the standard protocol. The address is included in messages to identify the intended recipient of commands from the host and responses from the remote.

3. Message Sequence

All communication between the host and the remote is initiated by the host. The remote shall not transmit except in response to a properly formatted message from the host addressed to the remote’s assigned address. The remote shall respond to such messages with an ACK (a single ASCII character 0x06) within 500 milliseconds (“ACK timeout”). The remote shall be ready
to receive and process another command in as little as 150 milliseconds after the ACK is transmitted (“Inter-command spacing”). Figure 1 below depicts this sequence.

![Diagram of Basic Message Sequence]

**Figure 1 - Basic Message Sequence**

Some messages from the host require a subsequent response message from the remote. In this case, the remote’s ACK shall be followed immediately by a response message. When the host receives the properly formatted response, it will transmit an ACK back to the remote. This sequence is depicted below in Figure 2. The timing constraints presented above apply. Additionally, the host will wait a maximum of 500 milliseconds for each byte in the response message (“Inter-byte timeout”).

![Diagram of Response Message Sequence]

**Figure 2 - Response Message Sequence**

4. Message Format

Command and response messages shall be formatted as shown below in Table 1.

<table>
<thead>
<tr>
<th>Size</th>
<th>Data</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>1 byte</td>
<td>0xF8</td>
<td>Message header</td>
</tr>
<tr>
<td>1 byte</td>
<td>Address</td>
<td>Controller address (0x01 to 0xDF)</td>
</tr>
<tr>
<td>2+ bytes</td>
<td>Message Data</td>
<td>(see Command and Response Message sections below)</td>
</tr>
<tr>
<td>1 byte</td>
<td>Checksum</td>
<td>0x80 to 0x8F (see below)</td>
</tr>
</tbody>
</table>
Messages start with a single-byte header followed by a single-byte address. Two or more message data bytes follow the address. The message is terminated with a single-byte checksum.

Checksum Calculations

The message checksum byte is defined as the least significant nibble of the exclusive-or of all previous bytes in the message (excluding the 0xF8 header) added to 0x80. The resulting byte will be between 0x80 and 0x8F inclusive.

Checksum Example:

• The bytes of a “Pan Left” command addressed to controller 1 would be:
  0xF8 0x01 0x50 0x4C 0x??

• The bitwise exclusive-or of the bytes between the 0xF8 header and the checksum comes out to 0x1D.
  0x01 ^ 0x50 ^ 0x4C = 0x1D

• Adding the least significant nibble to 0x80, we get 0x8D for the checksum byte.
  0x80 | (0x0F & 0x1D) = 0x8D

• The resulting 5-byte command would be
  0xF8 0x01 0x50 0x4C 0x8D

5. Command Messages

The tables below present the message data contained within command messages sent by the host to the remote. In most cases (exceptions will be noted), the “Data” column contains the ASCII characters of the message data to be sent. For example, the Pan Left command data is “PL”, an ASCII ‘P’ (0x50) followed by an ASCII ‘L’ (0x4C).

An “ACK” in the “Response” column indicates no response message is expected from the remote beyond the ACK itself as shown above in Figure 1. An “ACK + * Response” indicates the ACK from the remote is to be followed by the specified response message as shown above in Figure 2.

<table>
<thead>
<tr>
<th>Command</th>
<th>Data</th>
<th>Response</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pan Left</td>
<td>“PL”</td>
<td>ACK</td>
</tr>
<tr>
<td>Pan Right</td>
<td>“PR”</td>
<td>ACK</td>
</tr>
<tr>
<td>Pan Stop</td>
<td>“PS”</td>
<td>ACK</td>
</tr>
<tr>
<td>Tilt Up</td>
<td>“TU”</td>
<td>ACK</td>
</tr>
<tr>
<td>Tilt Down</td>
<td>“TD”</td>
<td>ACK</td>
</tr>
<tr>
<td>Tilt Stop</td>
<td>“TS”</td>
<td>ACK</td>
</tr>
<tr>
<td>Zoom In</td>
<td>“ZI”</td>
<td>ACK</td>
</tr>
<tr>
<td>Zoom Out</td>
<td>“ZO”</td>
<td>ACK</td>
</tr>
<tr>
<td>Zoom Stop</td>
<td>“ZS”</td>
<td>ACK</td>
</tr>
<tr>
<td>Focus Near</td>
<td>“FN”</td>
<td>ACK</td>
</tr>
<tr>
<td>Focus Far</td>
<td>“FF”</td>
<td>ACK</td>
</tr>
<tr>
<td>Focus Stop</td>
<td>“FS”</td>
<td>ACK</td>
</tr>
<tr>
<td>Iris Open</td>
<td>“IO”</td>
<td>ACK</td>
</tr>
<tr>
<td>Iris Close</td>
<td>“IC”</td>
<td>ACK</td>
</tr>
</tbody>
</table>
### Table 3 - Position Commands

<table>
<thead>
<tr>
<th>Command</th>
<th>Data</th>
<th>Response</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pan/Tilt Position Request</td>
<td>“P?”</td>
<td>ACK + Pan/Tilt Response</td>
</tr>
<tr>
<td>Pan/Tilt Goto Command</td>
<td>“pA2A1A0E2E1E0”</td>
<td>ACK</td>
</tr>
<tr>
<td>(see Position Encoding below)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Zoom/Focus Position Request</td>
<td>“V?”</td>
<td>ACK + Zoom/Focus Response</td>
</tr>
<tr>
<td>Zoom/Focus Goto Command</td>
<td>“vZ2Z1Z0F2F1F0”</td>
<td>ACK</td>
</tr>
<tr>
<td>(see Position Encoding below)</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

### Table 4 - Latch Commands

<table>
<thead>
<tr>
<th>Command</th>
<th>Data</th>
<th>Response</th>
</tr>
</thead>
<tbody>
<tr>
<td>Manual Iris Toggle</td>
<td>“LM”</td>
<td>ACK</td>
</tr>
<tr>
<td>Camera Power Toggle</td>
<td>“LP”</td>
<td>ACK</td>
</tr>
<tr>
<td>Lens Speed Toggle</td>
<td>“LL”</td>
<td>ACK</td>
</tr>
<tr>
<td>Latch Status Request</td>
<td>“L?”</td>
<td>ACK + Latch Response</td>
</tr>
</tbody>
</table>

### Table 5 – Preset Commands

<table>
<thead>
<tr>
<th>Command</th>
<th>Data</th>
<th>Response</th>
</tr>
</thead>
<tbody>
<tr>
<td>Goto Preset 0 – 9</td>
<td>“H0” – “H9”</td>
<td>ACK</td>
</tr>
<tr>
<td>Store Preset 0 – 9</td>
<td>“P0” – “P9”</td>
<td>ACK</td>
</tr>
<tr>
<td>Preset Status Request</td>
<td>“H?”</td>
<td>ACK + Preset Response</td>
</tr>
</tbody>
</table>

### Position Encoding

The message data for the “Pan/Tilt Goto Command” above begins with an ASCII ‘p’ followed by the azimuth ($A_2A_1A_0$) and elevation ($E_2E_1E_0$) positions. The positions are 12-bit values encoded four-bits each in to the least significant nibbles of the three bytes whose most significant nibbles are always 0x30. The subscript-2 indicates the byte containing the most significant bytes of the position and subscript-0 indicates the least significant.

**Position Encoding Example:** Encoding an azimuth position value of decimal 2748 or 0xABC, we break it up into three nibbles and add 0x30 to each so $A_2 = 0x3A, A_1 = 0x3B, \text{ and } A_0 = 0x3C$.

The same encoding scheme is used for the zoom and focus positions in the “Zoom/Focus Goto Command”.

### 6. Response Messages

The table below presents the message data contained within response messages sent by the remote to the host.

### Table 6 – Response Messages

<table>
<thead>
<tr>
<th>Message</th>
<th>Data</th>
<th>Comment</th>
</tr>
</thead>
</table>

Office of Traffic Operations
| Pan/Tilt Position Response | “PA₂A₁A₀E₂E₁₀”  
(see Position Encoding above) | using the same data encoding as Pan/Tilt Goto Command described earlier. (note: while the command uses a lower-case 'p', the response uses an upper-case 'P') |
|---------------------------|-----------------------------------------------------------------|
| Zoom/Focus Position Response | “VZ₂Z₁Z₀F₂F₁₀”  
(see Position Encoding above) | using the same data encoding as Zoom/Focus Goto Command described earlier. (note: while the command uses a lower-case 'v', the response uses an upper-case 'V') |
| Latch Response | “LD₁AD₀”  

\[ D₁ = 0x30 \text{ to } 0x38 \]
\[ D₀ = 0x30 \text{ to } 0x37 \] | The bits of \( D₁ \), the second byte in the data, indicate:
0: Iris Auto(0) or Manual(1)
1: Camera Power Off(0) or On(1)
2: Lens Speed Slow(0) or Fast(1)
3: Comm. Error No(0) or Yes(1)
4: (always 1)
5: (always 1)
6: (always 0)
7: (always 0)
The bits of \( D₀ \), the fourth byte in the data, indicate:
0: Aux-1 Off(0) or On(1)
1: Aux-1 Off(0) or On(1)
2: Aux-1 Off(0) or On(1)
3: (always 0)
4: (always 1)
5: (always 1)
6: (always 0)
7: (always 0) |
| Preset Response | “H D₀”  
\[ D₀ \] is a single ASCII character indicating:
'0'–'9': at the indicated preset
'A': active, going to a preset
'I': inactive, not at a preset
'E': error, unable to get to preset |

While the remote is going to a position as a result of a “Pan/Tilt Goto”, “Zoom/Focus Goto”, or “Preset Goto” command, “Preset Responses” will return ‘A’ in the D0 byte. If the remote is unable to get to the position in the “Goto” command (i.e. the pan/tilt assembly reaches the mechanical stops), the “Latch Response” will return ‘E’ in the D0 byte.

E. Cabling and Connectors

Provide cabling and connectors between the camera system assembly and the cabinet interface assembly as shown in the CCTV system detail drawings and in the Plans.

Provide coaxial video signal cables C2 and C4, with labels attached at both ends of each cable. Video signal cable C2 shall be double-shielded with tinned copper braid, #20 AWG solid copper center conductor, and polyethylene outer jacket approved for outdoor use (Belden 8281 or approved equivalent.) Video signal cable C4 shall be high-flexibility double-shielded with tinned copper braid, #22 AWG stranded copper center conductor, and PVC outer jacket (Belden 8281F or approved)
equivalent.) Use BNC connectors with gold-plated center pins on the video signal cables; use only connectors recommended by the cable manufacturer.

Provide control cable C1 with labels attached at both ends of the cable. Control cable C1 shall be a UL-listed six twisted-pair power limited tray cable with #18 AWG 19-strand tinned copper conductors with individual foil or braid shield on each pair and UV-resistant PVC or polyethylene outer jacket rated for 300 V, -20C to 105C operating range and suitable for aerial, outdoor, and underground conduit use. Terminate cable C1 in the equipment cabinet on TB18 as shown in the CCTV system detail drawings and as recommended by the CCTV system manufacturer. Ground or bond any pair shielding and any unused conductors in accordance with the CCTV system manufacturer’s recommendations.

Provide camera system assembly connector cable C3 to connect cables C1 and C2, through connectors J1 and J2 respectively, with the camera system assembly connector as provided by the CCTV system manufacturer. Cable C3 shall be manufactured and configured to mate with connectors J1 and J2 in accordance with the conductor assignments shown in the CCTV system detail drawings and the CCTV system manufacturer’s recommendations. Use stranded copper conductors, or a solid copper conductor for a coaxial cable, with minimum 75 degree C individual conductor and outer jacket insulation ratings on all materials. Cable C3 shall be configured with the minimum length necessary for connection within the mounting bracket arm to J1, J2 and the camera system assembly connector, while providing sufficient slack for a technician to disconnect and reconnect the cable at the camera system assembly connector. Cable C3 shall mate to the camera system assembly connector with a positive locking mechanism to prevent vibration from loosening the connection. The camera system assembly connector shall use gold-plated pins and sockets. Connector J1 on cable C3 shall be configured with male conductor pins. Connector J2 on cable C3 shall be a coaxial cable BNC connector as specified for cables C2 and C4 above. All connectors on cable C3 shall be strain-relieved. Label the cable as C3 and with the camera system manufacturer’s name and model number.

Terminate the aerial end of cable C1 with connector J1, which shall connect to mating connector J1 on C3. For J1 use a strain-relieved CPC 17-14 free-hanging plug and receptacle connector set (AMP 206043 / 206044 or compatible and approved equivalent) with gold-over-nickel spring-detent two-piece pins and sockets, with female conductor sockets terminated on cable C1. Do not use one-piece “formed” pins and sockets.

Terminate the aerial end of cable C2 as connector J2 with a BNC connector as specified for cables C2 and C4 above. Connect cable C2 to cable C3 through a BNC coupler with gold-plated center socket.

At the CCTV system manufacturer’s option and recommendation, submit for approval a single-jacketed composite cable that contains conductors and sub-cables with the equivalent function of cables C1 and C2. The composite cable shall have an UV-resistant PVC or polyethylene outer jacket rated for 300 V, -20C to 105C operating range and suitable for aerial, outdoor, and underground conduit use. The composite cable shall contain as a minimum: two (2) shielded-twisted pairs of #22AWG stranded conductors with drain wires; three (3) #16AWG stranded THHN-THWN conductors; and one (1) coaxial sub-cable exactly equivalent to cable C1 specified herein. Label the composite cable as C1/C2. Terminate cable C1/C2 in the equipment cabinet and in the mounting bracket arm by splitting out the separate conductors and sub-cables from the outer jacket; strain-relief with tie-wraps at the outer jacket opening. Terminate separated conductors in cable C1/C2 exactly as required for individual cables C1 and C2 at connectors J1 and J2, on TB18, and at SS15, following the exact pin-outs as shown in the CCTV system detail drawings. Ground or bond any pair shielding and any unused conductors in accordance with the CCTV system manufacturer’s recommendations.

936.2.02 CCTV System, Type B, Retrofit Assembly

Furnish a complete CCTV System, Type B, Retrofit Assembly to be installed at existing CCTV sites with existing support poles, equipment cabinets, power service, and communications systems. Ensure that the individual components and assemblies of the CCTV System, Type B, conform to all requirements specified in Subsection 936.2.01. The retrofit assembly includes all equipment and materials specified in Subsection 936.2.01 including but not limited to the camera system assembly, mount, cabinet interface assembly, communications and protocols, and cabling and connectors. Provide all new materials, including all surge suppressors and wiring to/from existing cabinet equipment such as C4, C5, SS15 and mounting plate, TB18 and mounting plate, 120VAC wiring to the cabinet interface assembly, and ground wiring to the cabinet interface assembly.

Determine and furnish the necessary CCTV System, Type B, cable lengths for all cables at each site shown in the Plans, including but not limited to cables C1 and C2 in the pole and all cables and wiring within the cabinet, as required by the equipment mounting locations. Determine and furnish the required C5 connector at the communications device end with proper pinout. Use the exact mating connector for the communications device port; do not use any port adapters, gender changers, couplers, or similar components.
Furnish a complete CCTV System, Type B, Retrofit Assembly (Furnish Only) to be delivered to the Department. Ensure that the individual components and assemblies of the CCTV System, Type B, conform to all requirements specified in Subsection 936.2.01. The retrofit assembly (furnish only) includes all equipment and materials specified in Subsection 936.2.01 including but not limited to the camera system assembly, mount, cabinet interface assembly, communications and protocols, and cabling and connectors. Provide all new materials, including all surge suppressors and wiring to/from existing cabinet equipment such as C4, C5, SS15 and mounting plate, TB18 and mounting plate, 120VAC wiring to the cabinet interface assembly, and ground wiring to the cabinet interface assembly. Ensure compliance with all fabrication and assembling requirements of Subsection 936.3.05, including but not limited to fastening hardware, wiring and mounting labeling, and wiring and cabling dressing, bundling, and strain relief.

Unless otherwise shown in the Plans, provide a cable length of 85 ft (26m) for C1 and fully terminate one end with connector J1. Provide a cable length of 85 ft (26m) for C2 and fully terminate both ends with male BNC connectors. Provide a cable length of 4 ft (1.2m) for cables C4, C5, C6, camera system power from TB18 to the camera interface assembly, 120VAC wiring to the cabinet interface assembly, and ground wiring to the cabinet interface assembly. Terminate both ends of cable C4 with male BNC connectors. Strip bare and tin the communications device end of cable C5. All other cable ends shall be terminated with the appropriate mating connector of the cabinet interface assembly; conductor ends intended for terminal blocks shall be stripped bare and tinned.

A. CCTV System, Type B, Retrofit Assembly (Furnish Only)

Provide all materials in protective packaging suitable for shipping and storage. Label all boxes with contents, including manufacturer name, model, serial numbers, and project number. Package each CCTV system in individual boxes as units of one complete CCTV System, Type B, Retrofit Assembly; multiple boxes for one assembly is acceptable, but multiple assemblies in the same box is not. Deliver assemblies to the Department at the location specified by the Engineer. Deliver at one time the full quantity of complete assemblies as shown in the Plans; multiple deliveries are not permitted.

936.3 Construction Requirements

Ensure that all construction for the equipment, materials, components and assemblies of the CCTV System Type B conform to the CCTV manufacturer’s requirements and recommendations.

A. General Requirements

Request that the Department establish the utility service(s) required for a CCTV installation as described in Section 939.
B. CCTV System, Type B (all)

1. Installation Requirements

Mount the camera system assembly and the mounting bracket arm at the cardinal direction and height as shown in the Plans.

Install cables C1 and C2 between the camera system assembly and the equipment cabinet inside new hollow metal or concrete support poles unless otherwise specified. Where devices are installed on existing wood poles, install cabling on the wood poles in rigid metal conduit risers of minimum 2 in (50.8 mm) diameter. Use weather heads on all nipple and conduit openings. Neatly install and route cabling to minimize movement in the wind and chafing against the pole, device or bracket. Form a drip loop at the weather head and route cabling to prevent water entry into the weatherhead or mounting bracket arm. Install the mounting bracket arm no more than 8 in (204 mm) above the weatherhead, and install a drip loop that is no more than 6 in (152 mm) below the weatherhead at the loop’s lowest point.

Install the cabinet interface assembly components in the equipment cabinet as shown in the CCTV system detail drawings. Neatly arrange and dress all wiring, firmly lace or bundle it, and mechanically secure the wiring without the use of adhesive fasteners. Route and secure all wiring and cabling to avoid sharp edges and to avoid conflicts with other equipment or cabling. Route all CCTV cables separate from any 120VAC power wiring or surge suppressor ground wiring. Neatly coil and dress between 3 ft (1 m) and 5 ft (1.5 m) of cables C1 and C2 in the bottom of the cabinet. Dress and route grounding wires separately from all other cabinet wiring and with the minimum length possible between the suppressor and the ground bussbar. Do not splice any cable, shield or conductor used for video, control, communications signaling, power supply, or grounding.

Fasten all components of the cabinet interface assembly to be mounted on the equipment cabinet side panel or on the CCTV Interface Enclosure with stainless steel hex-head or phillips-head machine screws. Install the screws into tapped and threaded holes in the panels. Fasten stud-mounted components to a mounting bracket providing complete access to the studs and mounting nuts. All fastener heads and nuts (when used) shall be fully accessible within the equipment cabinet, and any component shall be removable without requiring removal of other components, panels, or mounting rails. Do not use self-tapping or self-threading fasteners. Label TB18, each terminal position on TB18, and SS15 on the equipment cabinet side panel with silk-screened lettering on the panel.

For CCTV System, Type B, Retrofit Assembly, unless otherwise shown in the Plans, remove and dispose of all existing CCTV equipment including camera enclosures and mounting arms, pole-mount hardware, all cables from cabinet to camera, and all CCTV-specific wiring devices inside the cabinet including surge suppressors. Where shown in the Plans to salvage and return any existing CCTV equipment, remove and deliver the identified equipment to the location specified by the Engineer in the same condition as prior to the project. Repair or replace any equipment damaged by the Contractor’s operations. Do not remove or disturb any pole equipment, hardware or wiring, or existing cabinet equipment and wiring including those materials for other devices, cabinet power service, and communications systems. Mount new CCTV equipment and materials in the existing cabinet in locations as close as possible to the same locations required in new cabinet installations.

2. CCTV System Configuration

Program and configure the CCTV system in accordance with the procedure below. Provide all required documentation in writing with all data recorded in the format of the NaviGAtor Standard CCTV Control Protocol. Perform the CCTV system configuration in accordance with the acceptance procedures in subsection 936.3.06. It should be noted that configuration requirements specified herein are independent of the video encoder. CCTV System Configuration is not required for CCTV System, Type B, Retrofit Assembly (Furnish Only).

Configure each CCTV system with the communications address specified by the Department, prior to any acceptance testing at a given CCTV system site. Configure the communications address as “001” unless otherwise shown in the Plans or directed by the Engineer.

GDOT’s “ATMS Surveillance Camera Control Integration and Calibration Procedure” is as follows:

- Record the position status setting for the full pan left and pan right stops. The pan left and pan right stops are defined as the camera positions when the pole initially comes into view from either direction at maximum zoom.
• Record the position status setting and angle (degrees from horizon) for maximum tilt up and maximum tilt straight down (90 degrees down from horizontal).

• Record the position status settings for each end of maximum focus range.

• Record the position status settings for maximum zoom out and zoom in.

• Provide to the Department the following information from each field installation site:
  • Location (as shown in Plans)
  • Height of camera (ft) above travel lanes
  • Azimuth (compass heading in degrees at camera’s right stop as defined above)
  • Azimuth (compass heading in degrees at camera’s left stop as defined above)
  • Device ID as shown on the Plans (example: CAM001)
  • CCTV system communications address (example: 001)
  • IP address, port/socket of serial port on Encoder
  • Video switch input port (when connected)
  • Comments

3. As-Built Drawings

Furnish as-built CCTV system wiring diagrams, identified by location. Include all wiring, cabling, conductor function, connector type and connector pinouts. Included but not limited in the documentation requirements are cables C1, C3, C5, C6, the CCTV embedded protocol control software cable, and all connectors and terminations on the cables, the camera system assembly, TB18, the user control interface unit and power supply of the cabinet interface assembly, and the communications equipment. Place all documentation in a weatherproof sleeve and place in the cabinet drawer. As-built drawings are not required for CCTV System, Type B, Retrofit Assembly (Furnish Only).

936.3.06 Quality Acceptance

A. General

Acceptance testing of CCTV System, Type B, and CCTV System, Type B, Retrofit Assembly consists of three phases: 1) field installation testing; 2) CCTV system site testing; and 3) burn-in period. After the Engineer’s granting of burn-in period completion, obtain CCTV system acceptance. Acceptance testing of CCTV System, Type B, Retrofit Assembly (Furnish Only) consists of bench acceptance testing.

Perform acceptance testing for all equipment, hardware and work provided under this Contract, including each CCTV video surveillance field installation assembly and all associated communications hardware at a control center or communications hub. Perform all testing in the presence of the Engineer. Notify the Engineer of a desired acceptance test schedule no less than fourteen calendar days prior to beginning the testing except for testing using the NaviGAtor software and existing NaviGAtor control center and communications equipment. For acceptance testing using the NaviGAtor software and existing NaviGAtor control center and communications equipment, coordinate this testing with the Engineer no less than 30 days prior to the start of this testing.

Except as provided herein regarding the Department’s NaviGAtor software, develop, provide all equipment for, and perform all acceptance testing for all CCTV system equipment, hardware and work. Develop detailed and thorough test procedures with full test plan descriptions, test and measurement equipment listings, and test results data sheets. As part of the submittal data requirements, submit these test plans to the Engineer for approval. The Engineer will notify the Contractor of the approval or disapproval of the test procedures; only test procedures approved by the Engineer can be used. Provide all necessary testing and measurement equipment. Have a complete copy of all materials and equipment submissions and all documentary items on hand at all acceptance testing sessions.
Make the acceptance testing plan a detailed and thorough procedure for both the field installation test and the CCTV system site test. Demonstrate that the CCTV system equipment, hardware and work meet all requirements of the Contract. These requirements include but are not limited to all design, construction, materials, equipment, assembly, documentation of manufacturer’s certification of assembly and configuration, environmental, performance, communications, video and data communications signal strength and clarity, compatibility with the NaviGAtor software, and documentary requirements of the Contract.

Prior to the beginning of any acceptance testing at a given CCTV system site, complete all configuration and documentation associated with GDOT’s “ATMS Surveillance Camera Control Integration and Calibration Procedure,” described in Subsection 936.3.05.B. Be prepared to demonstrate such work.

**B. Field Installation Test**

Perform the Field Installation Test as an onsite test of the complete field installation assembly less the communications components; no acceptance testing at a given site can begin until all work associated with that site is complete, not including the communications components. For the field equipment installation test, use a PC system, CCTV Embedded Protocol control software, and a 13 in or larger color video monitor to demonstrate full operation of the CCTV site. Demonstrate operation to include pan, tilt, focus, zoom, iris, position feedback, and communications address configuration. Measure the video signal strength at the video connector of the communications equipment.

**C. CCTV System Site Test**

For the CCTV System Site Test, demonstrate proper CCTV system performance at the TMC or other control center determined by the Department. Perform the CCTV System Site Test only after successful completion of the field installation acceptance test. Demonstrate the complete video image, camera/lens control, and communications operation from each CCTV site to the TMC. Use the NaviGAtor software and existing NaviGAtor control center and communications system to demonstrate the compatibility of the CCTV equipment and installation in its permanent NaviGAtor configuration. Verify data communications (pan, tilt, focus, zoom, iris, position feedback) from the TMC as defined in the Department-approved test procedures.

**D. Burn-in Period**

1. General Requirements

   Provide a 30-day burn-in period for all work and equipment included in the Contract. The burn-in period shall consist of the field operation of the CCTV system in a manner that is in full accordance with the CCTV system requirements of the Plans and Specifications. An acceptance test procedure is not required for the system burn-in.

   Conduct only one (1) burn-in period on the entire Contract. Commence with the burn-in period only after meeting all of the following requirements:

   - All work required in all Contract documents for CCTV (may be combined with construction contract) (except this burn-in period) has been completed and inspected by the Engineer.

   - Successfully complete all Acceptance Testing.

   Commence with the burn-in period upon written authorization by the Department to commence. Terminate the burn-in period 30 consecutive days thereafter unless an equipment malfunction occurs. Stop the burn-in period for the length of time any equipment is defective. After repairing the equipment so that it functions properly, resume the burn-in period at the point it was stopped.

   Successful completion and acceptance of the burn-in period will be granted on the 30th day unless any equipment has malfunctioned during the 15th through 30th day of the burn-in period. If any equipment has failed during the 15th through 30th day, final acceptance will be withheld until all the equipment is functioning properly for 15 days after repair.

   When a specific piece of equipment has malfunctioned more than three times during the 30 day burn-in period, replace that equipment with a new unit and repeat the 30 day burn-in period.

2. Contractor Responsibilities
During the burn-in period, maintain all work under this Contract in accordance with the Specifications. Restore any work or equipment to proper operating condition within 12 hours after notification.

3. Department Responsibilities

Department responsibilities during the burn-in period will be as follows:

- Expeditious notification of Contractor upon failure or malfunction of equipment
- In the event that the Contractor does not provide the services enumerated above under his Contract responsibilities, the Department or its authorized agents may in the interest of public safety take emergency action to provide for adequate traffic control. Pay any costs incurred as a result of these emergency actions. Such action by the Department will not void any guaranties or warranties or other obligations set forth in the Contract.

4. Burn-In Period Acceptance

The Department will make burn-in period acceptance after satisfactory completion of the required burn-in period and on the basis of a comprehensive field inspection of the complete CCTV system in accordance with the Specifications. Upon burn-in period acceptance but prior to Final Acceptance of the entire Contract, maintain the complete CCTV system in accordance with the requirements of Subsection 936.3.07.

E. Bench Acceptance Test

Perform the bench acceptance test as an onsite test of the complete retrofit assembly for all assemblies furnished as shown in the Plans. Furnish a benchtop stand and associated hardware for the camera system assembly mount that securely holds the camera system assembly while the camera is being operated. For the bench acceptance test, use a PC system, CCTV Embedded Protocol control software, and a 13 in or larger color video monitor to demonstrate full operation of the CCTV site. Demonstrate operation to include pan, tilt, focus, zoom, iris, position feedback, and communications address configuration. Measure the video signal strength at the video connector of the communications equipment end of cable C4. At the successful completion of the bench acceptance test, repack all materials in the shipping packaging required in Subsection 936.2.04.

936.3.07 Contractor Warranty and Maintenance

Provide a manufacturer’s support (usual and customary warranties) period for all equipment and materials furnished and installed as part of the pay item for CCTV system equipment and materials. Include warranties or guarantees for system camera assembly and mount, cabinet interface assembly, and cabling/connector. Begin warranty upon successful completion of the CCTV System Site Test or Bench Acceptance Test.

Transfer Manufacturer’s and Contractor’s warranties or guarantees to the agency or user responsible for the CCTV system maintenance, The warranties and guarantees shall be continuous throughout their duration, and state that they are subject to such transfer.

936.3.08 Training

Provide installation, operations, and maintenance training on the CCTV equipment at a site in the project area. Personnel trained by the various equipment manufacturers and authorized by said manufacturers shall perform the training. Include in the cost of training all supplies, equipment, materials, handouts, travel, and subsistence necessary to conduct the training. Furnish a training notebook in a labeled 3-ring binder to each trainee.

Provide installation, operations and maintenance training for up to twelve (12) people. Include in this training both classroom training and hands-on training. Limit in-shop and in-field training to group sizes of four (4) people at a time. Conduct all training in half-day sessions. Two half-day sessions may be held on the same day. The total of the training shall consist of at least six (6) clock hours of training for each participant. Provide a course content of, at a minimum, the following:

- Installation of all CCTV equipment
- Operations of all CCTV equipment
• Explanation of video quality
• Maintenance of all CCTV components
• Use of the CCTV embedded protocol control software
• Measurement of video signals
• Discussion of all warrantee clauses
• Hands-on use of CCTV equipment in signal shop environment for each trainee
• In-field maintenance training

CCTV training shall be provided in conjunction with the digital video transport system training specified in Section 939. The total of the CCTV and video transport system training shall consist of at least eight (8) clock hours of training for each participant. Meet all video transport system training requirements of Section 939.

936.4 Measurement

936.4.01 CCTV System, Type B

CCTV systems, Type B, are not measured for payment separately. This shall include the Contractor furnishing and installing the following minimum items for a CCTV system, Type B:

• camera system assembly including the camera, lens, pan/tilt drive, control electronics and environmental enclosure.
• pole-mounting hardware.
• cabinet equipment, including but not limited to the cabinet interface assembly and all associated wiring, conductors, terminal blocks, and surge suppression.
• all weather heads, vertical conduit risers and conduit hardware on the CCTV support pole for power service, grounding, communications and control.
• all cables, connectors, hardware, interfaces, supplies, and any other items necessary for the proper operation and function of any CCTV system component with any other CCTV system component.

936.4.02 CCTV System, Type B, Retrofit Assembly

CCTV Systems, Type B, Retrofit Assembly are not measured for payment separately. This shall include the Contractor furnishing and installing the following minimum items for a CCTV System, Type B, Retrofit Assembly:

• removal and disposal, or removal, salvage and delivery, of all existing CCTV equipment.
• camera system assembly including the camera, lens, pan/tilt drive, control electronics and environmental enclosure.
• pole-mounting hardware.
• cabinet equipment, including but not limited to the cabinet interface assembly and all associated wiring, conductors, terminal blocks, and surge suppression.
• any weather heads, vertical conduit risers and/or conduit hardware on the CCTV support pole for grounding, communications and control cabling included in or affected by the installation of the retrofit assembly.
• all cables, connectors, hardware, interfaces, supplies, and any other items necessary for the proper operation and function of any CCTV system component with any other CCTV system component.
936.4.03 CCTV System, Type B, Retrofit Assembly (Furnish Only)

CCTV Systems, Type B, Retrofit Assembly (Furnish Only) are not measured for payment separately. This shall include the Contractor furnishing and installing the following minimum items for a CCTV System, Type B, Retrofit Assembly (Furnish Only):

- camera system assembly including the camera, lens, pan/tilt drive, control electronics and environmental enclosure.
- pole-mounting hardware including round pole adapter, attachment hardware, and mounting straps.
- cabinet equipment, including but not limited to the cabinet interface assembly and all associated wiring, conductors, terminal blocks, surge suppression, and fastening hardware.
- packaging for shipping and storage.
- all cables, connectors, hardware, interfaces, supplies, and any other items necessary for the proper operation and function of any CCTV system component with any other CCTV system component.

936.4.04 Testing

Testing is not measured separate for full delivery of testing and acceptance requirements.

936.4.05 Training

Training is not measured separately for all supplies, equipment, materials, handouts, travel, and subsistence necessary to conduct the training.

936.4.06 Limits

Not applicable

936.5 Payment

936.5.01 CCTV System

Payment for CCTV systems is made under CONSTRUCTION COMPLETE and shall include full compensation for furnishing and installing or delivering the CCTV system.

936.5.02 Testing

The Department will pay for testing performed as prescribed by this Item, measured as provided under Measurement at the Lump Sum Contract bid price for CONSTRUCTION COMPLETE.

936.5.03 Training

Payment for training is made under CONSTRUCTION COMPLETE.

936.5.04 Adjustments

Not applicable
CCTV System Detail Drawings
Detail Drawing 936.1a
System Block Diagram, CCTV System, Type B
(Ethernet over Fiber)
CCTV System Detail Drawings
Detail Drawing 936.1b
System Block Diagram, CCTV System, Type B
(Ethernet over VPN)
CCTV System Detail Drawings
Detail Drawing 936.2a
Type D Equipment Cabinet Assembly with
Type B CCTV Equipment.

Notes:
1. All dimension and scale are approximate.
2. FDC (Fiber Distribution Center) required and paid for in Section 935.
3. Support Panels shall be mounted inside the cabinet rack cage.
Notes:
1. All dimension and scale are approximate.
2. Support Panels shall be mounted inside the cabinet rack cage.

CCTV System Detail Drawings
Detail Drawing 936.2b
Type E Equipment Cabinet Assembly with Type B CCTV Equipment.
CCTV Field Termination Mounting Panel (TB18, SS15)

Note:
1. All linear dimensions ±0.005" tolerance.
2. Drawing not to scale
3. Mounting Panel shall be 0.125" 5052 Aluminum

CCTV System Detail Drawings
Detail Drawing 936.3
CCTV Field Termination Mounting Panel,
CCTV System Type B
1. Use C1 to J1 conductor and pin termination assignments as shown. Use camera system assembly power supply function and TB18 termination as shown. Use C1 pairs 2-6 and TB18 positions 4-14 for function assignments, including chassis, pair shield, or data ground, as per CCTV system manufacturer. Use only TB18 position 3 for power ground when required by CCTV system manufacturer. Do not use J1 pins 2 and 4.

2. Label 1018, each terminal position on TB18, and SS15 on the equipment cabinet side panel with silk screened lettering on the panel.
SECTION 999 – DESIGN-BUILD

999.1 GENERAL DESCRIPTION

A. Project Location: The location of the construction work included in this Project is shown in the Costing Plans. This Project is located in Cherokee County, with portions located in the limits of the City of Woodstock. The Project is located along Interstate 575 between the exits of Towne Lake Parkway (exit 8), and the Etowah River.

B. Design-Build Concept: The Contractor and a design consultant (or design consultant team) shall work together to design and build the Project. The design consultant shall either be acting as a subconsultant to the Contractor or as a joint-venture member with whom this agreement has been executed. In this document (Section 999), the words “design consultant” or “design consultant team” shall refer to the consultant firm or consultant team acting as a subcontractor or joint-venture team member to the Contractor. The design consultant or design consultant team will not be required to fill out Department subcontractor forms for Department use.

The words “Engineer” (with a capital “E”) shall refer to those personnel of the Department which are or are acting in the capacity of an engineer for the Department. When the word “engineer” is used it shall refer to those persons acting on behalf of the Contractor. The Department will have oversight responsibilities only, which include performing official reviews and granting acceptance of the design work.

The Contractor shall not begin land disturbing activities until the following have been accepted by the Engineer, and the Department provides written authorization that the plans are Released for Construction:
1. Basis of design
2. ROW recertification
3. NEPA recertification
4. Approved Permits (including but not limited to the Nationwide 14 USACE Section 404 permit)
5. Final construction plans
6. Erosion Sedimentation and Pollution Control Plans
7. Notice of Intent (NOI) submission to EPD along with the 14 day wait period
8. QC/QA Plan
9. Traffic Control Plan
10. Traffic Management Plan
11. Utility Agreements, Utility Encroachment Permits, Utility Relocation Plans, and/or Contractor Certification of “No-Conflict”
Bids on this project shall reflect designing and constructing the project as shown in the Scope (999.1.C) and applicable portions of the Costing Plans and specification package. No exceptions shall be assumed by the Contractor. However, alternative proposals on portions of the work will be entertained once the project is awarded. The VE Specification, including procedures, in which the Contractor and Department have a share in the savings, can be utilized as a part of this project. Alternative proposals shall not conflict with the overall completion date of the project or intermediate completion dates unless otherwise approved by the Department.

The Contractor may propose alternative methods/solutions to the Project Scope once the project is awarded, but shall provide the same, or better, facilities as shown in the Costing Plans Package and specifications and meet the following criteria: no additional or increased costs, no extension in overall schedule (or specified milestones), and no exceptions to specifications included in this contract. Alternatives proposed by the Contractor shall be submitted to the Engineer in writing and shall include clear documentation background, reason for the change, and any potential cost and/or time savings that may result. If acceptable, the Department will authorize the change in writing.

The Contractor shall use those entities prequalified in related disciplines (design, traffic analysis, geotechnical, etc.) as presented in the Statement of Qualifications. Revisions to the design and data-gathering elements of the team and/or the proposed assignments reflected in the Statement of Qualifications shall be approved by the Engineer. Approval of any replacements in the team shall occur prior to the letting of the project. All proposed changes to the team shall be received prior to letting. The Contractor shall send all requests for changes to:

- Transportation Services Procurement Administrator
- Georgia Department of Transportation
- Office of Transportation Services Procurement
- One Georgia Center,
- 600 West Peachtree Street, NW, Room 1920
- Atlanta, GA 30308

Additional disciplines needed to meet the requirements of the special provisions for this project not identified in the Statement of Qualifications shall meet GDOT prequalification as required and any applicable standards, policies or guidelines of the local, state or federal agencies or utility owners.

Any revisions to the team and/or the proposed assignments reflected in the Statement of Qualifications after award of the contract shall be approved by the Department. The Contractor shall send all requests to the Department’s project manager for review and further handling for approval.

Where specifications differ, this Special Provision 999 Design-Build, shall take precedence unless otherwise revised through the amendment process. Prescriptive provisions found in the Specifications shall be followed for this project.

The work covered under this Specification includes the furnishing of all materials, labor, tools, equipment, and other incidental items for the designing, detailing, and construction of the Project contained in the Project Scope. The Contractor shall make all the improvements for this Project within the right of way and easement limits that are shown in the approved Right of Way plans as included in the Costing Plans, (See also 999.1.D Right of Way).

Information related to “existing conditions”, as reflected in the Costing Plans Package, is for information only. The Contractor shall be aware that existing conditions found in the Costing Plans Package may have changed since the field survey work and associated design efforts were completed. The Costing Plans Package, along with the specifications, shall attempt to highlight areas of known changes in the existing conditions. These areas may or may not include all actual areas where existing conditions differ from those that currently exist in the field. The Contractor shall be responsible to verify all existing conditions. No claims will be considered due to decisions/assumptions made by the Contractor based on “existing conditions” reflected in the Costing Plans Package.
C. General Project Scope: The Project includes the construction of a full diamond interchange on a new location where Ashland Parkway/Ridgewalk Parkway currently overpasses Interstate 575 (I-575) approximately 1.6 miles north of the Towne Lake Parkway interchange and approximately 1.6 miles south of the Sixes Road interchange. The new exit will be known as Exit 10. The existing Ashland Parkway/Ridgewalk Parkway bridge over I-575 (Project no I575-1(6)07, contract 5, bridge #6) shall be removed and a new bridge constructed to accommodate the proposed typical section. The Project shall also realign the portion of existing Old Rope Mill Road located north of Ridgewalk Parkway approximately 400 feet to the east of its current location to form a four-way intersection with Woodstock Parkway and Ridgewalk Parkway. Also, two 12-foot auxiliary lanes, one in each direction, will be added/extended on I-575 between the new interchange and the existing I-575/Towne Lake Parkway interchange. The total project length along Interstate 575 is approximately 9,300 feet (1.76 miles). The following is a list of the major items of the project:

1. Relocation of Old Rope Mill Road, from the current location to the intersection of Woodstock Parkway and Ridgewalk Parkway, for approximately 3,000 ft to tie-in to the existing roadway. The typical section will be an urban two lane section with turn lanes at its relocated intersection with Woodstock Parkway.

2. Widening and reconstruction of Ridgewalk Parkway/Ashland Parkway, from its current intersection with Woodstock Parkway, to approximately 1,000 ft northwest of the proposed new bridge over I-575. The approximate length of this reconstruction is 2,000 ft. (not including the length of bridge over I-575). The typical section will be one travel lane in both directions, a center turn lane, paved shoulders in conjunction with curb and gutter, along with turn lanes at the proposed ramps and at the intersection of Woodstock Parkway/Old Rope Mill Road.

3. Construction of a new bridge over Interstate 575 in the same general location as the existing bridge over I-575. The bridge will be 224 ft long and 74' - 5" wide. The implementation of the new bridge will require the existing bridge to remain in service. After an alternative access to the property on the west side of I-575 is available, by the construction of a portion of the new bridge, the existing bridge over I-575 is to be removed and the remaining portions of the bridge are to be constructed.

4. Construction of four new ramps for exit and entrances onto I-575. The total length of all four ramps is approximately 10,600 ft, not including tapers.

5. Construction of an auxiliary lane from the end of the proposed south-bound on-ramp to the existing south-bound off ramp at Towne Lake Parkway. The approximate length is 5,350 ft.

6. Relocation/reconstruction of the existing Cul-de-Sac on Freelon Lane in order to construct the relocation of Old Rope Mill Road.

7. Sound barrier walls shall be constructed along the west side of Interstate 575 at the existing or proposed right of way. In one location, the sound barrier is to be constructed adjacent to the shoulder of ramp D (south bound on-ramp) on top of a proposed retaining wall. Early construction of the sound barriers is an environmental commitment.

8. Sound barrier type B or type C shall be used on this project.

9. Ramp meters are not a part of the project.

10. A Value Engineering Study was performed for the project and those recommendations and approved items for implementation are to be adhered to in the final design/construction of the project.

11. Retaining walls are anticipated at four locations, 1) along relocated Old Rope Mill Road to provide for the relocation of Freelon Lane, 2) along the southbound on-ramp to minimize impacts to the existing recreation facilities of Deer Run Subdivision, 3) along the southbound auxiliary lane to avoid purchase of right of way from residential areas of Deer Run Subdivision, 4) between the southbound off-ramp and the southbound travel lane of I-575.
12. Limited access fencing along the limits of access shall be maintained or installed as necessary to delineate and secure the existing and proposed limits of access. The existing woven wire fence along the old right of way line shall be replaced in the areas where it is impacted. The new fence shall be placed along the required Right of Way lines where possible or along construction limits in those areas where Right of Way is outside of construction limits. The new fence shall tie to the existing Right of Way fence and shall be installed after grading activities are complete.

13. Overhead signs along Interstate 575 shall be installed in accordance with Department and MUTCD design guidelines, as well as MUTCD “Notice of Proposed Amendments.” In addition, Interstate exit signs for the new interchange shall be installed at the 1 mile and ½ mile locations for the south bound off-ramp.

14. Strain poles for construction of traffic signals and overhead wire spans shall be pre-stressed concrete or steel.

15. The Contractor shall install Right of Way monuments on this project.

16. Preformed Plastic Pavement Marking, 8 inch Contrast, shall be used on all concrete roadways, specifically on ramps A, B, C, D, and the Bridge.

17. The existing truck climbing lane on I-575 southbound shall be striped out, and all signage related to the truck climbing lane shall be removed.

18. Access to the construction of I-575 through the Deer Run Community ball field via any temporary easement obtained for construction shall be minimized and/or avoided to the greatest extent possible.

19. The Contractor shall install 4 – 2” fiberglass conduits on the new Bridge for the Department’s use. The conduits shall terminate past the bridge apron and into a type IV pull box in the shoulder.

20. The speed designs for the respective roadways are as follows:
   a. Interstate 575 75 mph
   b. Old Rope Mill Road 35 mph
   c. Ridgewalk Parkway 35 mph
   d. Freelon Lane 25 mph

21. The Contractor shall restore or replace existing facilities in kind or better as specified herein. Possible affected, existing facilities include, though not limited to the following: cameras, signing and marking, traffic signals, conduit, electrical wiring, utilities, water distribution, and sanitary sewer. If applicable on this project, lighting on the interstate under the bridge shall conform to RP-22-96 IESNA Recommended Practice for Tunnel Lighting dated 1996. Design and construct any required lighting.

22. Upon the Department’s authorization to begin land disturbing activities the Contractor may permanently close the I-575 SB outside lane (truck climbing lane) from station 647+70 (+/-) to station 687+70 (+/-). This permanent lane closure will not conflict with Special Provision 150.11. Once closed this lane shall not be reopened to traffic. This lane shall be permanently striped out in the project’s final configuration.

23. The Contractor shall install two pre-stressed concrete strain poles, and to each attach a CCTV camera with a wireless cellular communications and video connection to GDOT NaviGAtor. CCTV installation shall include cabinet, conduit, and all necessary hardware/software for wireless connection in accordance with applicable GDOT specifications. The Contractor shall coordinate and engineer power and wireless connections to these poles. One pole shall be located in the southeast quadrant of the Ridgewalk Pkwy/SB Ramp intersection. Another pole shall be located in the northwest quadrant of the Ridgewalk Pkwy/NB Ramp intersection.
24. 6’ chain link fence shall be installed on top of any and all retaining walls, unless otherwise noted in the costing plans as handrail.

25. When asphaltic curb is installed along shoulder behind guardrail the Contractor shall use concrete flumes and spillways to drain.

26. Existing pavement inside the construction limits that will no longer be used shall be obliterated, graded to drain and grassed.

27. Cherokee County will be responsible for relocating their utilities within conflict on this project. The Contractor shall coordinate utility relocations with Cherokee County.

28. One traffic signal (traffic signal upgrade) shall be installed at the intersection of Woodstock Parkway/Ridgewalk Parkway. A traffic signal permit will not be required for this location since it is off-system.

29. Stop condition intersections shall be installed at the ramps and Ridgewalk Parkway and shall accommodate design year turning volumes.

30. The Contractor shall install a driveway at parcel 11 for access to the existing cell tower. As indicated in the general notes shown in the Costing Plans, an asphalt driveway shall be installed to the radius return, and Aggregate Surface Course shall be installed beyond the radius return to the driveway tie-in. A temporary driveway easement will be obtained by the Department, and the revised Right-of-Way plans will be provided to the awarded design-build team.

31. The Contractor shall install detention facilities if the roadway construction produces a 10 percent or greater increase in peak flow volume for the design year storm event at any outfall, in areas where existing storm drain systems are downstream of the project have been determined not to have sufficient capacity to carry the required design event, and/or at outfall locations where the receiving channel is located in or leads to an environmentally sensitive area (wetlands, habitats, etc.).

32. All permanent asphaltic concrete shall include H-Lime.

D. Right of Way: All construction shall occur within the existing or proposed Right of Way and easements per the approved Right of Way Plans. The City of Woodstock is acquiring the Right of Way, and the acquisition is scheduled to be complete prior to letting. The Department will issue a Right of Way recertification upon completion of the acquisition.

The Contractor is responsible designing and constructing the Project within the Right of Way and easements. If elements of the proposed construction are shown extending beyond this area then the Contractor is responsible for designing and constructing measures necessary for the construction limits to remain within the Right of Way and easements.

The Contractor shall prepare revised Right of Way plans in conjunction with the final As-Built plans. The revised Right of Way plans are to reflect the final constructed roadway elements on the project. The Contractor shall use the original Right of Way centerlines/baselines as the survey centerlines/baselines and shall only include the roadway constructed elements in the revised Right of Way plans. Right of Way data table sheets will remain as shown in the previously approved Right of Way plans.

E. Environmental: The Contractor shall provide all material, labor, equipment and other incidentals required to adhere to the “Commitments/ Requirements” that apply to the Contractor, design or construction of the project. Key words such as “construction,” “contractor,” “work,” etc., point to the areas of responsibility by the Contractor. Other general items include:

1. The NEPA document for this project is an EA/FONSI which has been approved. The Department is currently performing a reevaluation of the document based on the construction limits in the Costing Plans. This reevaluation is expected to be complete prior to letting.
2. A Section 404 Nationwide Permit application has been drafted by the Department, and is based on those impacts in the Costing Plans.

3. The Contractor shall provide the preliminary plans, preliminary utility relocation plans and anticipated construction schedule to the Department as per Table 4-1. The Department will use this information to re-quantify impacts, prepare the permit documentation, submit permit documentation to the appropriate agencies and secure all necessary environmental permits. It is anticipated that approximately 90 days will be required to obtain permit approvals from receipt of the accepted preliminary plans. If more impacts result from the Contractor’s proposed design versus those in the Costing Plans then the Department will facilitate special studies being performed which will require 60 additional days prior to the Department’s submittal of permit documentation to the appropriate agency and will require the Department to complete a reevaluation.

4. Once the Department receives an approved permit from the US Army Corps of Engineers, the Department will issue written notification to the Contractor that the Contractor shall then acquire all mitigation credits as required under the approved permit. All mitigation credits obtained by the Contractor and applied to the project shall be approved by the US Army Corps of Engineers. Upon satisfactory receipt of the Contractor’s credit purchase, the Department will provide written authorization to work in jurisdictional Waters of the US in accordance with the permit conditions.

5. The Contractor is responsible for identifying the need for any Stream Buffer Variances on this project. The Contractor shall also be responsible for the necessary design and construction needed to mitigate for the Stream Buffer impact. If a Stream Buffer Variance is identified then the Contractor is responsible for notifying the Department at the time of the preliminary plans submittal. The Department will be responsible for preparing the Stream Buffer Variance application, and the Contractor shall be responsible for providing the necessary documentation to the Department. It is anticipated that approximately 120 days will be required to obtain a Stream Buffer Variance from receipt of the Section 404 Nationwide Permit (and any other necessary documentation needed for the application), if a Stream Buffer Variance is determined to be required.

6. Upon adequate completion of NEPA related tasks (including receipt of the purchase of the mitigation credits), the Department will issue a NEPA recertification. No land disturbing activities shall take place until the Department issues the NEPA recertification.

The following pages include environmental commitments:
ENVIRONMENTAL COMMITMENTS TABLE

<table>
<thead>
<tr>
<th>Project Information</th>
<th>Project Manager Review</th>
<th>Specialist Review</th>
</tr>
</thead>
<tbody>
<tr>
<td>Project No.: CSNHS-0006-00(043)</td>
<td>☑ I have reviewed these commitments and verified their feasibility.</td>
<td>Air/Noise A.P./XKR</td>
</tr>
<tr>
<td>County: Cherokee</td>
<td>☐ All delineations are marked on the plans.</td>
<td>Archaeology To</td>
</tr>
<tr>
<td>PI No.: 0005043</td>
<td></td>
<td>EcoLogic/404 Y</td>
</tr>
<tr>
<td>Status: Construction</td>
<td>Michael D. Rawe 12/11/2009</td>
<td>History 6</td>
</tr>
<tr>
<td>Date Updated: December 11, 2009</td>
<td>PM Signature</td>
<td>NEPA Y</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>NO.</th>
<th>COMMITMENT/REQUIREMENT (Separate out commitments by PI No.)</th>
<th>DOCUMENT STIPULATED IN</th>
<th>RESPONSIBLE PARTY</th>
<th>ESTIMATED COST*</th>
<th>PLACE ON PLANS (Yes or No)</th>
<th>REQUIRES A SPECIAL PROVISION (Yes or No)</th>
<th>STATUS (Pre- and Post Construction - Complete or Incomplete; During Construction - Signature Required)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>A detailed barrier analysis will be conducted to further determine whether or not the proposed noise barriers would be effective.</td>
<td>Noise Impact Assessment dated 03-19-07</td>
<td>Office of Environment and Location (OEL) &amp; Office of Innovative Program Delivery (IPD)</td>
<td>$19,133.40</td>
<td>Yes</td>
<td>No</td>
<td>Complete</td>
</tr>
<tr>
<td>2</td>
<td>All waters of the US will be delineated on the plans.</td>
<td>Ecology Report dated July 2008</td>
<td>IPD</td>
<td>$300.00</td>
<td>Yes</td>
<td>No</td>
<td>Complete</td>
</tr>
<tr>
<td>3</td>
<td>Public outreach regarding the noise barrier locations &amp; effectiveness will be conducted once barrier design is complete.</td>
<td>Environmental Assessment (EA)</td>
<td>OEL and IPD</td>
<td>$480.00</td>
<td>No</td>
<td>No</td>
<td>Complete</td>
</tr>
<tr>
<td>4</td>
<td>Staging will be conducted so that the grading &amp; construction of the noise walls would occur as early as possible during project implementation so as to minimize construction noise impacts to residences in the area. Where feasible, construction would take place primarily during the less noise</td>
<td>EA</td>
<td>IPD</td>
<td>$0.00</td>
<td>Yes, staging</td>
<td>Yes (SP 999)</td>
<td>Complete</td>
</tr>
</tbody>
</table>

*Estimated Cost for planning purposes only; in current dollars as of Date Updated

Page 1 of 5
<table>
<thead>
<tr>
<th>NO.</th>
<th>COMMITMENT/REQUIREMENT (Separate out commitments by PI No.)</th>
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<th>STATUS (Pre- and Post Construction – Complete or Incomplete; During Construction - Signature Required)</th>
</tr>
</thead>
<tbody>
<tr>
<td>5</td>
<td>Sensitive daylight hours to avoid impact during the hours associated with sleep [see Special Provision (SP) 999 attached].</td>
<td>Ecology Report dated July 2008</td>
<td>OEL</td>
<td>$5,000.00</td>
<td>No</td>
<td>No</td>
<td>Incomplete</td>
</tr>
<tr>
<td>6</td>
<td>A Nationwide 14 USACE Section 404 permit with a PCN will be obtained for impacts to waters of the US.</td>
<td>Ecology Report dated July 2008</td>
<td>OEL</td>
<td>$5,000.00</td>
<td>Yes</td>
<td>Yes (SP 107.23G)</td>
<td>Complete</td>
</tr>
<tr>
<td>7</td>
<td>SP 107.23G for the protection of the Cherokee darter would be adhered to during project implementation (see SP 107.23G attached).</td>
<td>Ecology Report dated July 2008</td>
<td>IPD</td>
<td>$180.00</td>
<td>Yes</td>
<td>No</td>
<td>Complete</td>
</tr>
<tr>
<td>8</td>
<td>Stream buffers will be delineated on the plans.</td>
<td>Ecology Report dated July 2008</td>
<td>IPD</td>
<td>$180.00</td>
<td>Yes</td>
<td>No</td>
<td>Complete</td>
</tr>
<tr>
<td>9</td>
<td>The culvert at Stream 5 will be designed to retain the natural stream substrate and allow for long term aquatic species passage (see SP 999 attached).</td>
<td>Ecology Report dated July 2008</td>
<td>OEL and IPD</td>
<td>$5,000.00</td>
<td>Yes</td>
<td>Yes (SP 999)</td>
<td>Incomplete – To be included in the Design/Build contract</td>
</tr>
<tr>
<td>10</td>
<td>Survey information in the area of the ball field will be provided to Michael Douglas, Douglas Property Management.</td>
<td>EA</td>
<td>IPD</td>
<td>$120.00</td>
<td>No</td>
<td>No</td>
<td>Complete</td>
</tr>
<tr>
<td>11</td>
<td>Access to construction on I-575 via the remainder of the ball field parcel, through temporary construction easements, would</td>
<td>EA</td>
<td>IPD</td>
<td>$0.00</td>
<td>Yes, staging</td>
<td>Yes (SP 999)</td>
<td>Complete</td>
</tr>
</tbody>
</table>

*Estimated Cost for planning purposes only; in current dollars as of Date Updated*
### ENVIRONMENTAL COMMITMENTS TABLE

**Project No. CSNHS-0006-00(043), Cherokee County**  
**Date Updated: December 11, 2009**

| NO. | COMMITMENT/REQUIREMENT  
(Separate out commitments by P1 No.) | DOCUMENT STIPULATED IN | RESPONSIBLE PARTY | ESTIMATED COST | PLACE ON PLANS  
(Yes or No) | REQUIRES A SPECIAL PROVISION  
(Yes or No) | STATUS  
(Pre- and Post Construction  
Complete or Incomplete:  
During Construction - Signature Required) |
|-----|---------------------------------|----------------------|------------------|----------------|---------------|---------------|--------------------------------------------------|
| 11  | be avoided or minimized to the  
greatest extent possible (see  
SP 999 attached). |  
Ecology Addendum  
dated December 2009 | OEL and IPO | $180.00 | Yes | Yes  
(SP 999) | Complete |

**During Construction Commitments**  
Construction or Area Engineer signature required upon the completion of all During Construction Commitments.

| NO. | COMMITMENT/REQUIREMENT  
(Separate out commitments by P1 No.) | DOCUMENT STIPULATED IN | RESPONSIBLE PARTY | ESTIMATED COST | PLACE ON PLANS  
(Yes or No) | REQUIRES A SPECIAL PROVISION  
(Yes or No) | STATUS  
(Pre- and Post Construction  
Complete or Incomplete:  
During Construction - Signature Required) |
|-----|---------------------------------|----------------------|------------------|----------------|---------------|---------------|--------------------------------------------------|
| 12  | A NPDES permit will be obtained  
after the project is let to  
design-build contract and  
prior to land disturbing  
activities (see SP 999 attached). |  
Ecology Report dated  
July 2008 | IPD/GDOT Office of  
Construction | $1,000.00 | No | Yes  
(SP 999) | During Construction – Signature  
Required |
| 13  | SP 107.23G for the protection  
of the Cherokee darter would  
be adhered to during project  
implementation (see SP 107.23G  
attached). Per the conditions  
of the SP, an ESA sign will be  
posted in the area of Stream 5. |  
Ecology Report dated  
July 2008 | GDOT Office of  
Construction | $500.00 | Yes | Yes  
(SP 107.23G) | During Construction – Signature  
Required |
| 14  | Staging will be conducted so  
that the grading and  
construction of the noise walls  
would occur as early as  
possible during project |  
EA | GDOT Office of  
Construction | $0.00 | Yes, staging | Yes  
(SP 999) | During Construction – Signature  
Required |

*Estimated Cost for planning purposes only; in current dollars as of Date Updated
### ENVIRONMENTAL COMMITMENTS TABLE

**Project No. CSNHS-0006-00(043), Cherokee County**  
**Date Updated: December 11, 2009**

<table>
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<tr>
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<th>ESTIMATED COST</th>
<th>PLACE ON PLANS (Yes or No)</th>
<th>REQUIRES A SPECIAL PROVISION (Yes or No)</th>
<th>STATUS (Pre- and Post Construction – Complete or Incomplete; During Construction - Signature Required)</th>
</tr>
</thead>
<tbody>
<tr>
<td>15</td>
<td>Implementation so as to minimize construction noise impacts to residences in the area (see SP 999 attached).</td>
<td>EA</td>
<td>GDOT Office of Construction</td>
<td>$500.00</td>
<td>Yes</td>
<td>Yes (SP 999)</td>
<td>During Construction – Signature Required</td>
</tr>
<tr>
<td>16</td>
<td>A &quot;No Outlet&quot; sign will be provided at the entrance of the Brookshire subdivision (see SP 999 attached).</td>
<td>Ecology Addendum dated December 2009</td>
<td>GDOT Office of Construction</td>
<td>$180.00</td>
<td>Yes</td>
<td>Yes (SP 999)</td>
<td>During Construction – Signature Required</td>
</tr>
</tbody>
</table>

### Post Construction Commitments

<table>
<thead>
<tr>
<th>NO.</th>
<th>COMMITMENT/REQUIREMENT (Separate out commitments by P1 No.)</th>
<th>DOCUMENT STIPULATED IN</th>
<th>RESPONSIBLE PARTY</th>
<th>ESTIMATED COST</th>
<th>PLACE ON PLANS (Yes or No)</th>
<th>REQUIRES A SPECIAL PROVISION (Yes or No)</th>
<th>STATUS (Pre- and Post Construction – Complete or Incomplete; During Construction - Signature Required)</th>
</tr>
</thead>
<tbody>
<tr>
<td>17</td>
<td>In accordance with SP 107.23G for the protection of the Cherokee darter, no herbicides, pesticides or fertilizers will be used within the established ESA for Stream 5 (see SP 107.23G attached).</td>
<td>Ecology Report dated July 2008</td>
<td>Maintenance</td>
<td>$0.00</td>
<td>Yes</td>
<td>Yes (SP 107.23G)</td>
<td>Not applicable, perpetual commitment.</td>
</tr>
</tbody>
</table>

**Total Estimated Cost* for all Project Commitments:** $37,393.40

*Estimated Cost for planning purposes only; in current dollars as of Date Updated
999.2 PLANS

A. General: The Costing Plans Package prepared on behalf of the Department includes multiple resources listed below. They will be made available to the short listed Design-Build Teams via a read only GDOT FTP site and shall be considered for information only. These resources are to be used in preparing the bid and corresponding technical proposal (refer to Special Provision Section 102—Bidding Requirements and Conditions) for this project. The Contractor shall make the Department aware of any resource that is in error or would cause the design (as presented in the Costing Plans Package) to not be constructible.

The Georgia Department of Transportation, in making this information available to Contractors, assumes no responsibility for its accuracy. No claim will be considered if the Contractor relies on this “For Information Only” data in its bidding or in its construction operations and finds that it is inaccurate. The Contractor’s attention is directed to Specifications 101.16 – CONTRACT and 102.05 – EXAMINATIONS OF PLANS, SPECIFICATIONS, SPECIAL PROVISIONS, AND SITE OF THE WORK. Available information includes:

1. Concept Report – For Information Only
2. Location and Design Report and Notice – For Information Only
3. Interchange Justification Report (IJR)
4. Project Framework Agreement (PFA) – For Information Only
5. Environmental Document
6. Value Engineering Study and Responses to Study
7. Survey Control Information – For Information Only
8. CAICE Data – For Information Only
9. Microstation J Files – For Information Only
10. Bridge Foundation Investigation (BFI) Report – For Information Only
11. Bridge Layout – For Information Only
12. Soils Survey Report – For Information Only
13. Wall Foundation Investigation (WFI) Report – For Information Only
14. Pavement Structures
15. Overhead/Subsurface Utility Engineering Investigation Plans for quality level “B” (See Section 999.3.D.3.c for details) – For Information Only
17. Traffic Engineering Studies
18. Traffic Management Plan – For Information Only
19. Previously prepared highway/bridge plans within the project area.
   a. Original Bridge design plans for the existing bridge over I-575, PI No 620354, Contract 5, bridge No 6.
   b. Original Roadway design plans for I-575
   c. PI No. 620352, I-575, North of Rope Mill Road
   d. PI No. 620353, I-575, Roadway Plans
   e. PI No. 620355, I-575, Signing and Marking Plans

The above items are available for download at the GDOT FTP site which shall be accessed two ways:

<table>
<thead>
<tr>
<th>1st Access to the FTP site through the internet:</th>
<th>2nd Access to the FTP site through the FTP program:</th>
</tr>
</thead>
<tbody>
<tr>
<td>ftp://dotpublic:<a href="mailto:dotoutside02@ftp.dot.state.ga.us">dotoutside02@ftp.dot.state.ga.us</a>/</td>
<td>Username: dotpublic</td>
</tr>
<tr>
<td></td>
<td>Password: dotoutside02</td>
</tr>
<tr>
<td></td>
<td>Host Name: ftp.dot.state.ga.us</td>
</tr>
</tbody>
</table>

The files are located at /DOTFTP/InnovativeProgramDelivery/0006043/. The Contractor shall check this site weekly for possible updates.
999.3 DESIGN

A. General: The Contractor shall be responsible for the design of the complete project. The design shall be based on their specific knowledge, and engineering judgment in the preparation of the design for the project.

1. Measuring Units: The project shall be designed in English units of measurement.

2. Design Software: Microstation/J and CaiCE software are required.


Current Department design manuals and guidelines may be found at: http://www.dot.ga.gov/doingbusiness/PoliciesManuals/roads/Pages/default.aspx. Project designers shall adequately consider all elements of the design, including but not limited to roadway geometry, drainage requirements, traffic control during construction, erosion control, structural design, utility conflicts, signing and marking, and future maintenance requirements.

Roadway lighting will not be required.

4. Design Reviews: The design is to be prepared under the direct supervision of licensed design professionals. A Professional Engineer licensed to practice engineering in the State of Georgia on the design team shall seal the final plans. Their seal on the drawing shall represent certification that the design meets all applicable codes and is of good engineering practice and standards. It shall be the responsibility of the Contractor to check and certify the design.

The Department will establish dates and times for cursory reviews and will comment on design work, but will not require hold points on the design, review periods, or comment responses, except as noted otherwise. If at any time the Department determines that the design work is not in conformance with the Department’s standards, details, specifications, or good engineering practice, the Department reserves the right to stop work, at the Contractor’s expense until a resolution of the issue(s) has occurred.

Construction documents (plans and specifications) shown in Table 4-1 shall be submitted to the Department for review and acceptance. Acceptance, disapprovals, or comments made by the Department will be provided in writing to the Contractor within the appropriate timeframes shown in Table 4-1.

No construction is to begin on any phase of the work prior to the Department authorizing the various component(s) of the plans as Released for Construction. Other items shall be submitted to the Department by the Contractor, if requested. After the Department has accepted the plans and has authorized them as Released for Construction then the Contractor shall submit to the Department a request for any subsequent plan/design changes and include necessary documentation which supports the reasoning behind the change request. The Department must approve the requested change with written notice prior to its implementation as a plan revision and subsequent construction activity.
The Contractor shall facilitate monthly progress meetings at a venue and time that is determined convenient to the Department. The general purpose of these meetings are to update the Department staff on the status of design, current activities, issues, activities that the Department is currently performing, and other related matters that impact scope, schedule and budget. The Contractor shall provide the Engineer an agenda of items one week in advance of the meeting so that the Engineer may arrange for the various GDOT Office reviewer(s) to attend, if necessary. Other attendees shall include the Contractor, design consultant, the Department’s Project Engineer and Project Manager. The Contractor shall provide a call in number and conferencing capabilities to allow others to participate at the Department’s discretion. The Contractor shall publish meeting notes of those discussions within two weeks of their occurrence. The first of these monthly meetings shall occur at the conclusion of the initial preconstruction conference.
## TABLE 4-1: REVIEWS

<table>
<thead>
<tr>
<th>Submittal Description</th>
<th>Format</th>
<th>Quantity</th>
<th>Delivery Date*</th>
<th>Review Period*</th>
<th>Review Type</th>
<th>Comment</th>
</tr>
</thead>
<tbody>
<tr>
<td>Basis of Design</td>
<td>HC, PDF</td>
<td>3, 1</td>
<td>NTP+7</td>
<td>14</td>
<td>Accepted by Engineer</td>
<td></td>
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<tr>
<td>Schedule – including review times</td>
<td>HC, PDF</td>
<td>3, 1</td>
<td>NTP+14</td>
<td>14</td>
<td>Accepted by Engineer</td>
<td></td>
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<tr>
<td>QC/QA Plan</td>
<td>HC, PDF</td>
<td>3, 1</td>
<td>NTP+14</td>
<td>14</td>
<td>Accepted by Engineer</td>
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<tr>
<td>Worksite Utility Control Supervisor</td>
<td>HC, PDF</td>
<td>3, 1</td>
<td>PAS</td>
<td>21</td>
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<tr>
<td>Worksite Erosion Control Supervisor Qualifications</td>
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<td>3, 1</td>
<td>PAS</td>
<td>21</td>
<td>Accepted by Engineer</td>
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<td>Traffic Control Supervisor Qualifications</td>
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<td>PAS</td>
<td>21</td>
<td>Accepted by Engineer</td>
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<tr>
<td>Construction Traffic Control Plan</td>
<td>FS, HS, PDF</td>
<td>3,3</td>
<td>PAS</td>
<td>21</td>
<td>See Specification 150</td>
<td></td>
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<tr>
<td>Traffic Management Plan</td>
<td>HC, PDF</td>
<td>3, 1 PDF</td>
<td>PAS</td>
<td>21</td>
<td>Accepted by Engineer</td>
<td>FHWA to perform concurrent review.</td>
</tr>
<tr>
<td>Preliminary Plans (including all roadway plan components, erosion control plans, signing and marking, ITS, signal plans)</td>
<td>HS, PDF</td>
<td>6, 1</td>
<td>NTP+90</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td>Incl overhead sign details/clearance diagrams. FHWA to perform concurrent review of plans. Department to review, quantify environmental impacts and amend permit.</td>
</tr>
<tr>
<td>Final Plans (including all roadway plan components, erosion control plans, signing and marking, ITS and signal plans)</td>
<td>HS, PDF</td>
<td>6, 1</td>
<td>PAS</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td>FHWA to perform concurrent review of plans.</td>
</tr>
<tr>
<td>Notice of Intent (NOI) with final/signed Erosion Control Plans</td>
<td>HS, PDF</td>
<td>3,1</td>
<td>PAS</td>
<td>NA</td>
<td>EPD letter stating plans do not contain deficiencies.</td>
<td>The Department will submit NOI Package to EPD. The Contractor shall be responsible for addressing any plan changes required by EPD and to EPD's satisfaction.</td>
</tr>
<tr>
<td>Submittal Description</td>
<td>Format</td>
<td>Quantity</td>
<td>Delivery Date*</td>
<td>Review Period*</td>
<td>Review Type</td>
<td>Comment</td>
</tr>
<tr>
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<td>------------------------</td>
<td>-------------------------------------------------------------------------</td>
</tr>
<tr>
<td>Preliminary Structures</td>
<td>FS, HS, PDF</td>
<td>2, 3, 1</td>
<td>PAS</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td>Acceptance of plans in plan package or submission of Alternate Plan; see 999.3.C.3.4.a.3.</td>
</tr>
<tr>
<td>• Bridge Layouts</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Preliminary Structures</td>
<td>FS, HS, PDF</td>
<td>2, 3, 1</td>
<td>PAS</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td></td>
</tr>
<tr>
<td>• Wall Layouts</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>50% Structures</td>
<td>FS, HS, PDF</td>
<td>2, 3, 1</td>
<td>PAS</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td>Do not proceed with the final design of bridge plans until the preliminary bridge plans have been approved by the Department and FHWA.</td>
</tr>
<tr>
<td>• Bridge Plans</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>100% Structures</td>
<td>FS, HS, PDF</td>
<td>2, 3, 1</td>
<td>PAS</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td>FHWA to perform concurrent review.</td>
</tr>
<tr>
<td>• Bridge Plans</td>
<td></td>
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<tr>
<td>• Wall Plans</td>
<td></td>
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<td></td>
<td></td>
</tr>
<tr>
<td>Shop Drawings</td>
<td>FS</td>
<td>6</td>
<td>PAS</td>
<td>30</td>
<td>Accepted by Engineer</td>
<td></td>
</tr>
<tr>
<td>Released for Construction Plans</td>
<td>FS, HS, PDF</td>
<td>9, 8, 1</td>
<td>PAS</td>
<td>NA</td>
<td>NA</td>
<td>Contractor shall provide the Engineer with one HS set for a backcheck prior to Release for Construction authorization.</td>
</tr>
<tr>
<td>Plan Revisions During Construction</td>
<td>FS, HS, PDF</td>
<td>1, 3, 1</td>
<td>Per Occurrence</td>
<td>14</td>
<td>Accepted by Engineer</td>
<td>Clear and concise supporting documentation justifying reason for proposed revision to be included with submittal.</td>
</tr>
<tr>
<td>As-Built Plans</td>
<td>See 999.3.A.8</td>
<td></td>
<td></td>
<td>Project Completion (+30)</td>
<td>Accepted by Engineer</td>
<td></td>
</tr>
<tr>
<td>Final Revised Right of Way Plans</td>
<td>See 999.3.A.9</td>
<td></td>
<td></td>
<td></td>
<td>Accepted by Engineer</td>
<td>See 999.1.D and 999.3.A.9</td>
</tr>
<tr>
<td>Utility Submittal Description</td>
<td>Format</td>
<td>Quantity</td>
<td>Delivery Date*</td>
<td>Review Period*</td>
<td>Review Type</td>
<td></td>
</tr>
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<td></td>
</tr>
<tr>
<td>Supplemental verification of Overhead/Subsurface Utility Engineering (SUE) Investigations - QL-B</td>
<td>MS, HS</td>
<td>Electronic SUE files, mapping files and proposed design files 1 to Engineer</td>
<td>NTP + 45 Calendar Days (Or as Determined by State Subsurface Utilities Engineer at SUE Kick-Off meeting)</td>
<td>21</td>
<td>Accepted by State Subsurface Utilities Engineer</td>
<td></td>
</tr>
<tr>
<td>SUE Utility Impact Analysis “UIA”</td>
<td>AR, PDF</td>
<td>3</td>
<td>NTP + 90 Calendar Days (Or as Determined by State Subsurface Utilities Engineer at SUE Kick-Off meeting)</td>
<td>30</td>
<td>Accepted by State Subsurface Utilities Engineer</td>
<td></td>
</tr>
<tr>
<td>Overhead/Subsurface Utility Engineering (SUE) Investigations - QL-A</td>
<td>AR, MS</td>
<td>Electronic SUE files, 1 to Engineer</td>
<td>Concurrently w/Approved UIA + 45</td>
<td>21</td>
<td>Accepted by State Subsurface Utilities Engineer</td>
<td></td>
</tr>
<tr>
<td>Subsurface Utilities Engineering Information to Utilities for Review (URPN Letter 1a - SUE Submit to Utility Companies Revise)</td>
<td>FS, HS, PDF, MS</td>
<td>Plans: 2 for each Utility Owner +3 for Dept. and Microstation files</td>
<td>NTP + 5 Calendar Days (Or as Determined by District Utilities Engineer at SUE Kick-Off meeting)</td>
<td>5 days for Dept. + 30 days for each Utility Owner</td>
<td>Reviewed by District Utilities Office (DUO) and Accepted by Engineer (According to the details contained in the MOUs)</td>
<td></td>
</tr>
<tr>
<td>Relocated Utility Plans (URPN Letter 2 - 2nd Submission Letter (Existing and Proposed))</td>
<td>FS, HS, PDF, MS</td>
<td>Plans: 2 for each Utility Owner +3 for Dept. and Microstation files</td>
<td>Concurrently w/ Accepted SUE Verification by Utility Owner</td>
<td>5 days for Dept. + 90 days for each Utility Owner</td>
<td>Reviewed by District Utilities Office (DUO) and Accepted by Engineer (According to the details contained in the MOUs)</td>
<td></td>
</tr>
<tr>
<td>Preliminary Utility Status Report (URPN Letter 6 - Notice to Proceed with Permit)</td>
<td>HC, PDF</td>
<td>3, 1</td>
<td>NTP + 160 Calendar Days (Or as Determined by State Utilities Preconstruction Engineer)</td>
<td>10 days + 5 days</td>
<td>Reviewed by District Utilities Office (DUO) and Accepted by Engineer (According to the details contained in the MOUs)</td>
<td></td>
</tr>
<tr>
<td>Utility Plans/Agreements (Utility NTP Letter)</td>
<td>Plans/Agreements HS, PDF, MS</td>
<td>Agreements: 3 hard copy, 1 electronic pdf Plans: 2 for each Utility Owner +3 for Dept. and MicroStation files</td>
<td>Concurrently w/ accepted Relocated Utility Plans</td>
<td>Agreements: 30 days for Dept. + 60 days for each Utility Owner Plans: 30 days</td>
<td>Relocated Plans and Agreements reviewed by Department Utilities Office. Agreements also reviewed by Utility Owner (According to the details contained in the MOUs)</td>
<td></td>
</tr>
</tbody>
</table>
TABLE 4-1: REVIEWS (Continued)

*All days are “Calendar Days.”, as defined in section 101, Standard specifications

**All Submittals** shall be made directly to the Engineer. The Engineer shall provide submittals to the applicable GDOT Office Reviewer and/or other applicable entities (including FHWA) as directed by the Engineer, unless otherwise noted or discussed with the Contractor. As accepted by the Engineer the Contractor may provide submittals to applicable offices for a concurrent review. The Contractor shall hand-deliver submittals, track and regularly update the Engineer on review status. In the event that concurrent submittals are required, the “receipt” date shall be the date the last recipient receives the submittal and shall be the contractual begin date for the review. Unless a different review time is specified elsewhere in the contract, a period of **thirty (30) calendar days** from receipt to release of the submittal by the Department shall be allowed for the Department’s review. Engineer’s (Department’s) acceptance as to completeness is required for all reviews. All Contractors’ schedules shall reflect the review times contained within the specifications and contract. Engineer’s receipt of submittals will mark the beginning of the review period. All submittals by the Contractor shall be required to contain a statement certifying that no unapproved design-exceptions have been incorporated in the submittal. Up to date half-size sets of plans with the most current design and construction plans shall be made available to a distribution list made up of up to 20 individuals/offices at all times during this project. Errors and omissions are the responsibility of the Contractor to correct and shall be at the Contractor’s expense.

Any submittal received by the Engineer after 12 PM (noon) shall be considered as being received the following business day.

<table>
<thead>
<tr>
<th>AR</th>
<th>As Required</th>
</tr>
</thead>
<tbody>
<tr>
<td>FS</td>
<td>Full-size paper – meets GDOT Plan Presentation Guide</td>
</tr>
<tr>
<td>HC</td>
<td>Hard Copy – 8 ½ x 11 unless otherwise noted</td>
</tr>
<tr>
<td>HS</td>
<td>Half-size paper – meets GDOT Plan Presentation Guide</td>
</tr>
<tr>
<td>MS</td>
<td>Microstation File – Electronic</td>
</tr>
<tr>
<td>NTP</td>
<td>Notice to Proceed</td>
</tr>
<tr>
<td>PAS</td>
<td>Per Approved Schedule</td>
</tr>
<tr>
<td>PDF</td>
<td>Adobe PDF – One complete file</td>
</tr>
</tbody>
</table>
5. **Field Surveys:** The Contractor shall verify all provided survey data. The Contractor is to provide terrain and drainage cross sections, pavement elevations, and drainage structure information for this project. All survey data shall be noted in English units. The following is only a guideline for data collection and is not intended to be comprehensive:

a. Provide cross sections of the terrain and pavement at stations as follows:
   
   1) These cross sections shall be provided at intervals adequate enough to accurately design and construct the Project, but not to exceed 50 feet along centerlines. Cross sections are required along the relocated driveway on the west side of I-575.
   
   2) The cross sections are to extend from the centerline to the limits of construction, plus 20 feet, or to the existing or proposed right of way, whichever is greater.
   
   3) In addition to all terrain breaks, the cross sections shall include all applicable edges of pavement (emergency, outside edges of travel lanes, and curb and gutter sections).

b. The Department feature codes shall be used when collecting the data in accordance with CAiCE Survey Data Guidelines, current version at time of advertising.

c. Locate all existing mainline drainage structures (X, Y, and Z) within the right of way and provide their size, type, condition, and flow line elevations at each end.

d. Locate inlet elevations for all drop inlets and catch basins.

e. Develop terrain profile at each drainage structure showing the skew of the structure.

f. Develop terrain profile of the drainage outfall from the end of each structure to the appropriate limits of influence.

g. Provide any additional necessary survey control.

h. Stake centerlines of all alignments, (mainline, sideroads, ramps, and driveways).

i. Verify the Survey control Packet.

j. Perform sign surveys

k. Perform bridge surveys

l. Perform surface utility surveys

m. Perform supplemental topo surveys

n. Perform right of way surveys

o. Perform stream surveys

p. Perform surveys of ITS items, if located within the limits of construction

q. The accuracy for all survey data shall be as follows:

   1) Horizontal: 1:10,000

   2) Topography: 0.4’

   3) Vertical: NOAA 3rd Order

   4) Pavement: 0.03’

   5) Ground Terrain: 0.25’
As per the Environmental Commitments, survey information shall be provided as early as possible to Michael Douglas, Douglas Property Management, in order to facilitate the relocation of the existing ball field within the Deer Run Community.

6. Quality Control/Quality Assurance for Design: The Department, except where noted otherwise, will have oversight responsibilities only and will not perform official reviews and approvals of design work. The Department will not take any approval or formal review actions on design issues except as noted herein or for deviations from the intended scope of the project.

The Contractor is to employ only persons duly registered in Georgia in the appropriate category in responsible charge of supervision and design of the work; and further, shall employ only qualified, State of Georgia registered land surveyors in responsible charge of any survey work.

The Contractor shall use only a consultant design team that is prequalified by the Department in all applicable area classes as described in the SOQ. Should a member of the Contractor’s Team need to be replaced, the Department must approve of the change prior to the project letting. Failure to secure approval of the replacements prior to letting may result in the disqualification of the Contractor’s bid.

The Contractor shall endorse all final reports, contract plans and survey data. These endorsements shall be made by a person(s) duly registered in the appropriate category by the Georgia State Board of Registration for Professional Engineers and Land Surveyors, being in the full employ of the Contractor and responsible for the work prescribed in the contract.

Authorized representatives of the Department and Federal Highway Administration (FHWA) may review and inspect the Project activities and data collected at all times. All reports, drawings, studies, specifications, estimates, maps and computations prepared by or for the Contractor shall be available to authorized representatives of both the Department and the FHWA for inspection and review. The Department’s review comments are to be incorporated into the plans by the Contractor or as agreed. These changes shall not result in an increase in cost.

Before the start of the contracted design effort, the Contractor shall develop and acquire the Department’s approval for a QC/QA Plan to ensure that all design documents are prepared in accordance with the Department’s Plan Presentation Guide (PPG) using good, prudent and generally accepted design and engineering practice. Also see the Department’s Manual of Quality Standards for Consultant Services.

a. The QC/QA Plan shall include the following, which shall be considered minimum requirements:

1) Quality control and quality assurance procedures for design documents shall specify measures to be taken by the Contractor to (A) ensure that appropriate quality standards are specified and included in the design documents and to control deviations from such standards, being understood and agreed that no deviations from such standards shall be made unless they have been previously accepted by the Department, and (B) for the selection of suitable materials and elements of the Work that are included in the Project.

2) Quality control and quality assurance procedures for preparing and checking all plans, calculations, drawings and other items submitted to ensure that they are independently checked and back-checked in accordance with generally accepted engineering practices, by experienced engineers. The originator, checker and back-checker shall be clearly identified on the cover of all submittals. Plans, reports and other documents shall be stamped, signed and
dated by the responsible Georgia Registered Engineer where required under the contract documents, generally accepted engineering practices or by applicable laws. It is required that the Contractor also submit a statement that all reviews have been completed.

3) Procedures for coordinating work performed by different persons within the same area, in an adjacent area or in related tasks shall ensure that conflicts, omissions or misalignments do not occur between drawings or between the drawing and specifications. These procedures shall also allow for the coordination of the review, approval, release, distribution and revision of documents involving such persons.

4) All the persons proposed to be responsible for Quality Control and Quality Assurance procedures are to be listed as follows: Discipline, Name, Qualifications, Duties, Responsibilities and Authorities.

5) All key personnel performing Quality Control and Quality Assurance functions shall be designated as such and shall not be assigned to perform conflicting duties.

All plan related documents produced during the contract period are to be maintained by the Contractor for the duration of the Contract and shall be organized, indexed and delivered to the Department (1) upon Final Acceptance of the project or (2) even if incomplete, within seven (7) days of receipt of request from the Department. These documents shall include, but not be limited to, the following items: design criteria, reports and notes, calculations, drawings, schematics, supporting materials, statement regarding accomplishment of reviews and others.

7. **Released for Construction:** Upon the Contractor’s satisfactory completion of the items listed in 999.1.B, and upon written authorization from the Department that the plans are Released for Construction, the Contractor shall stamp each plan sheet with “Released for Construction” and shall include the authorization date. The Released for Construction plans shall be the official plans used for construction of the Project.

8. **As-Built Plans:** Upon completion of the Project construction, a complete As-Built set of plans shall be provided to the Department in the following formats:

   a. Two (2) CD-ROMs or DVDs containing:
      1) all electronic design files, electronic calculations, etc.
      2) .tiff images of each plan sheet – one sheet per file
      3) .pdf containing the entire plan set
   b. One (1) hard copy of the design databook, and drainage calculations
   c. Two (2) full-size set of bond prints
   d. Two (2) half-size set of bond prints
   e. An estimated summary of quantities and detailed estimate shall be provided in the final As-Built plans

The Contractor shall be responsible for all production and delivery of materials needed for Department review. Both a member of the design team, who is a Professional Engineer, and a member who is a Registered Surveyor, licensed to practice engineering in the State of Georgia shall seal the As-Built plans.

All files are to conform to the criteria found in the Department’s Electronic Data Guidelines (EDG) found at: [http://www.dot.state.ga.us/doingbusiness/PoliciesManuals/roads/Pages/default.aspx](http://www.dot.state.ga.us/doingbusiness/PoliciesManuals/roads/Pages/default.aspx).
9. **Revised Right of Way Plans**: Upon completion of the Project construction, a complete set of Revised Right of Way plans shall be provided to the Department in the following formats:

a. Two (2) full-size set of bond prints

b. Two (2) half-size set of bond prints

10. **Ownership of Documents**: The Contractor agrees that all reports, drawings, studies, specifications, survey notes, estimates, maps, computations, computer files and other data, prepared by or for it under the terms of this Agreement shall be delivered to the Department to become and remain the property of the Department upon termination or completion of the work. The Department will have the right to use this information without restriction or limitation and without compensation to the Contractor other than that provided for in this agreement.

Any use of these documents by the Department on any project other than this one will be done without warranty by the Contractor/Design Consultant Team.

11. **Insurance**: In addition to the insurance requirements covered elsewhere, the Contractor shall have insurance coverage of the following types and amounts:

a. Valuable Papers: Insurance in an amount sufficient to assure the restoration of any plans, drawings, field notes or other similar data relating to the work covered by the project is required. Insurance is to be maintained in full force and effect during the life of this Agreement.

b. Professional Liability (Errors and Omissions): Insurance in an amount not less than one million dollars ($1,000,000) per claim (with a maximum of $250,000 deductible per claim) during the agreement term and for a period of at least five (5) years after this Agreement is closed is required. Such a policy is to cover all of the Contractor’s professional liabilities, whether occasioned by the Contractor, his employees, subcontractors or other agents, arising out of services performed under or in accordance with this Agreement.

12. **Publication and Publicity**: Articles, papers, bulletins, reports or other materials reporting the plans, progress, analyses or results and findings of the work conducted under this Agreement shall not be presented publicly or published without prior approval in writing from the Department. All releases of information, findings and recommendations shall include a disclaimer provision to be included in all published reports on the cover and title page in the following form:

“The opinions, findings and conclusions in the publication are those of the author(s) and not necessarily those of the Department of Transportation, State of Georgia or the Federal Highway Administration.”

Any information concerning the project, including conduct, results or data gathered or processed, released by the Contractor without prior approval from the Department will constitute grounds for termination without indemnity to the Contractor. Information released by the Department or by the Contractor with prior written approval is to be regarded as public information and no longer subject to the restrictions of this Agreement. Information required to be released by the Department under the Georgia Open Records Act, Section 50-18-70, et seq., O.C.G.A., the restrictions and penalties mentioned set forth herein shall not apply. Any request for information directed to the Contractor, pursuant to the Georgia Open Records Act, is to be redirected to the Department for further action.

13. **Copyrighting**: The Contractor and the Department agree that any papers, interim reports, forms and other material which are a part of work under this Agreement are to be deemed a “work made for hire”, as such term is defined in the Copyright Laws of the
United States. As a “work made for hire”, all copyright interests in said works shall vest in the Department upon creation of the copyrightable work. If any papers, interim reports, forms or other material which are a part of work under the Agreement are deemed by law not to be a “work made for hire”, any copyright interests of the Contractor are hereby assigned completely and solely to the Department. Publication rights to any works produced under this Agreement are reserved by the Department.

14. Patent Rights: If patentable discoveries or inventions shall result from work described herein, all rights accruing from such discoveries or inventions shall be the sole property of the Contractor. However, the Contractor agrees to and does hereby grant to the Department, an irrevocable, non-exclusive, non-transferable and royalty-free license to practice each invention in the manufacture, use and disposition according to law of any article or material and in use of any method that may be developed as a part of the work under this Agreement.

B. Roadway

1. Preparation of Construction Plans

a. General Criteria: The Contractor shall be familiar with and use the most current design criteria at the time of letting, as determined by the Department, American Association of State Highway and Transportation Officials (AASHTO) Design Manuals for Arterial Streets, Rural, Urban and Interstate Highways, including those standards adopted by AASHTO and approved by the Secretary of Commerce, as provided by Title 23, United States Code, Section 109 (b), with the Department’s Standards, Procedures, Plans, Specifications and Methods, with Federal Highway Administration procedures relating to plan review and approval, and shall produce plans in accordance therewith.


Design work for inside interstate rights of way shall conform to the interstate standards. Design for work outside interstate right of way shall conform to AASHTO design standards for the appropriate classification and speed design. Any deviation from this design criteria shall require a written design exception or variance, depending on the violation, to be approved prior to incorporating it into the work. The Contractor shall prepare the required design exception request for approval by the Department and the FHWA. All design exception requests shall justify why the guideline cannot be reasonably met considering such items as right of way impacts, cost, mitigation measures taken, and accident history and shall include a recommended alternative practice or design alternative. In addition to the references listed above, the following references shall be used as a minimum in the development of this project:

1) Electronic Data Guidelines (EDG) – current version at time of letting
2) Plan Presentation Guide (PPG) – current version at time of letting
3) GDOT Design Policy Manual – current version at time of letting
4) Manual on Uniform Traffic Control Devices (MUTCD) by the U.S. Department of Transportation, Federal Highway Administration “FHWA” and the current MUTCD Notice of Proposed Amendments– current version at time of letting

5) Manual on Drainage Design for Highways by the Georgia Department of Transportation - current version at time of letting

6) Roadway and Bridge Standard Plans as of July, 2006 by the GDOT Road and Airport Design Office. Design and plan preparation shall also be in accordance with the Certification Acceptance authorized by 23 USC 117(a) for Administering Federal Aid Projects Not On Interstate System, dated June 1, 1990.

7) Guidelines for Processing Design Data in CAiCE – http://www.dot.state.ga.us/doingbusiness/PoliciesManuals/roads/Pages/default.aspx - current version at time of letting.

8) GDOT Construction Standards and Details - current versions at time of letting

9) Pay Item Index by the GDOT State Transportation Office Engineer - current version at time of letting

10) Utility Accommodation Policy and Standards by the GDOT Utilities Office - current version at time of letting

11) GDOT Signing and Marking Design Guidelines – current version at time of letting

12) Traffic Signal Design Guidelines – current versions at time of letting

13) Other manuals of guidance which are standard procedures of the Department, (signal design, signing and markings, etc).

The above list is not intended to be all-inclusive. All references to the “current editions” shall mean those in effect at time of letting. Any current editions that are written in metric units shall be “soft converted” to U.S. Standards Units. Any rounding shall be to the dimension that shall increase safety.

c. Plan Sizes: Plans for roadway, drainage and utilities shall be reproducible quality drawings on bond paper. They shall have outside dimensions of 36" by 24" with a 2" margin on the left and a \( \frac{1}{2} \)" margin elsewhere and be produced by a Microstation CADD system.


d. Construction Plan Requirements and Scale: The Plans shall be fully dimensioned in English units; all elevations necessary for construction shall be shown similar to the Department’s normal practice. All plans are to be prepared on the scales listed below, unless otherwise accepted by the Department. Drawings and lettering shall be such as to produce clear and legible reproductions when reduced to half-size. The scale of sheets are to be as follows:

1) \( 1" = 10' \)
   
   • Roadway cross sections \( 1" = 10' \) horizontal and \( 1" = 10' \) vertical (NOTE: Cross sections may have to be plotted lengthwise on the sheet to avoid folded sections)
   
   • Staging cross sections \( 1" = 10' \) horizontal and \( 1" = 10' \) vertical (NOTE: Cross sections may have to be plotted lengthwise on the sheet to avoid folded sections)

2) \( 1" = 20' \)
   
   • Roadway plan sheets for the interstate portion shall be at a scale of \( 1"=20' \) (or \( 1"=50' \) upon Contractor request and Department written authorization).
• Roadway plans sheets for the non-interstate portions shall be at a scale of 1"=20'.
• Roadway profile sheets shall be at the same horizontal scale as their corresponding plan sheets, and the vertical scale shall be 1"=10'.
• Drainage profile sheets shall be at the same horizontal scale as the plan sheets and the vertical scale shall be 1"=10'.
• Staging plans for the project will be at the same scale and same sheet layout as the plan sheets.
• Bridge plans shall be per the Bridge Department’s Standards.
• Utility plans sheets shall be at the same scale and same sheet layout as the plan sheets.
• Drainage cross-sections shall be at the same scale as the roadway cross section sheets.

3) 1" = 100’
   • Stake out sheet

4) 1" = 400’ or 500’
   • Cover sheet
   • Drainage area map

The Contractor shall check all details and dimensions shown on the plans before they are submitted to the Department for review. Topography shall remain fully legible when plans are reduced in size, but shall be less prominent and readily distinguishable from the proposed work. Profile sheets shall have the existing ground line dashed and the required profile in a solid line. All other plan sheets (utility, erosion control, signing & marking, etc.) shall be the same scale and sheet layout as its corresponding roadway plan sheet.

e. **Construction Plans Organization and Sheet Index:** Construction plans shall be assembled according to the Department’s Plan Presentation Guide (PPG), current version at time of letting.

The total sheets shown in the Index shall be the total number of sheets in the plans. Plans shall be assigned temporary sheet numbers by using the sequence prefix followed by a three-digit number per the PPG. These numbers are to be placed in small blocks in the lower right corner of the sheet.

f. **Computations:** All design computations and computer printouts shall be neatly recorded on 8 ½” by 11”, fully titled, numbered, indexed, dated and signed by the designer/project manager and checker. The computer files and two copies of the computations fully checked and appropriately bound, shall be submitted to the Department with the plans. A complete tabulation of the drainage analysis along with the calculations used to determine the size of drainage structures shall be submitted to the Department.

g. **Plan Print Requirements:** The Contractor shall furnish all the prints necessary for the development of the preliminary and final construction plans and specifications. All prints shall be clear and legible.

h. **Supplementary Information on Construction Plan Preparation:** All of the sheet descriptions and others required for completeness of the plans shall conform to the Department’s Plan Presentation Guide.

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i. Traffic Flow Diagrams: The Contractor shall update these sheets to provide the current design and opening year traffic data information needed to determine design criteria. The sheets are not required to be to a scale, but the drawing shall show and represent the alignment of the overall project. Two sets of diagram shall be prepared, one which shows the Average Daily Traffic (ADT) and the other showing the peak Design Hourly Volumes (DHV).

j. Typical Sections:

1) Typical sections shall show dimensions (medians, travel-lanes, shoulders, slopes, ditches, etc.) from the construction centerline. Locate and label the roadway profile grade line for both existing and proposed. Label appropriate items as to type and thickness. All slope controls shall be specified on each typical section. Typical sections are being provided by the Department in the Costing Plans.

2) Typical sections shall indicate the spread rates for Asphaltic Concrete and thickness for Graded Aggregate Base to be used on the project, and/or PCC.

3) Any special conditions shall be shown as details on the typical section sheets. However, if these items are covered by a Georgia Standard or a construction detail, then a note shall be included referring to the standard or detail.

4) The scale of each typical section may and should differ between the horizontal and the vertical in order to more clearly show the division between separate layers of the structure of the pavement. However the typical sections shall be drawn to a scale and thus are proportionally accurate.

5) Roadway plans shall meet the established speed design within the limits of this project as shown in the current versions, at time of letting, of the AASHTO Roadside Design Guide and the MUTCD.

6) Any substandard guardrail within the limits of construction is to be replaced under this contract. Where construction exists only on one side, only the guardrail on the construction side shall adhere to this requirement.

k. Construction Plan Sheets: Construction plan sheets shall be in accordance with the Plan Presentation Guide including, but not limited to, the following: existing topography, construction centerline, curve data, edge of pavements, medians, drainage, bridges, and project limits.

l. Roadway Profile Sheets: The roadway profiles shall be in accordance with the Plan Presentation Guide including, but not limited to, the following: existing ground line, existing elevations, proposed ground line, proposed elevations, PVC, PVT, PVI, LVC, K Value, high points, low points, existing structures, and proposed structures.

m. Cross Section Sheets: Cross section sheets shall be developed in accordance with the Plan Presentation Guide.

n. Staging Plan Sheets: Staging plan sheets shall be in accordance with the Plan Presentation Guide including, but not limited to, the following: existing topography, construction centerline, curve data, edge of pavements, medians, drainage, bridges, and project limits.

o. Staging Profile Sheets: The staging profiles shall in accordance with the Plan Presentation Guide including, but not limited to, the following: existing ground line, existing elevations, proposed ground line, proposed elevations, PVC, PVT, PVI, LVC, K Value, high points, low points, existing structures, and proposed structures.
p. **Staging Cross Section Sheets:** Cross sections are to be developed to show and correspond to the Staging Plan Sheets. The cross sections are to show the travel paths of the vehicles, and status of the grading associated with that stage of construction. These cross sections are for the purpose of identifying conflicts with utilities, grading, and staging plan sheet sequencing.

q. **Drainage Profile Sheets:** Drainage profiles shall be shown for all proposed drainage structures. Existing drainage profiles shall be shown if pipe and structures are to be retained and when a proposed drainage system connects to it. Drainage structures shall be fully detailed and dimensioned.

All drainage structures located in a designated floodway shall be sized to comply with FEMA regulations. FEMA structures require the use of a computer analysis approved by FEMA, usually HEC-2 analysis, to remodel the flood plain and in order to document that the 100-year storm does not rise more than 1.0 foot total. If the published floodway is altered, all the necessary maps and computer printouts shall be included in the drainage analysis and the Contractor shall ensure that all FEMA and Local Government requirements are satisfied. All other guidelines and computation sheets are in the “Manual on Drainage Design for Highways”. The Contractor shall submit all final drainage computations.

r. **Sound Barrier Envelopes and Plans:** Sound barrier envelopes and plans sheets shall be in accordance with the Plan Presentation Guide (PPG) and current Department practices.

s. **Erosion and Sediment Control Sheets:** The Contractor shall not begin any land disturbing activities until the Control of Soil Erosion and Sedimentation Plan has been accepted by the Engineer; the NOI has been successfully submitted to EPD by the Department; EPD has issued a letter to the Department indicating the plan “does meet” current NPDES requirements; and the required waiting period of 14 days is observed.

The Erosion Sedimentation and Pollution Control Plans (ESPCP) shall be prepared in accordance with current Department practice, and in accordance with the requirements set forth in the NPDES General Permit No. GAR1000002 [August 2008]. NPDES General Permit Guidance may be found at: http://www.dot.state.ga.us/doingbusiness/PoliciesManuals/roads/Pages/DesignPolicies.aspx.

In addition, the plans shall be designed in accordance with the current version of Georgia Soil and Water Conservation Commission’s Manual for Erosion and Sediment Control in Georgia (Green Book).

Erosion and Sediment Control Plans detail the erosion control devices to be used. These devices include, but are not limited to, sediment traps, silt control gates, floating silt retention barriers, check dams, silt fence (types A, B & C), bailed straw ditch checks, brush barriers and slope drains. Additional plan sheets are required for each stage of construction. Additional plan sheets are also required to illustrate phased installation of erosion measures. All required sediment and erosion control items, including but not limited to installation and maintenance, shall be paid for under CONSTRUCTION COMPLETE.

t. **Signing and Marking and Signalization Requirements:** Prepare signing, signalization and marking plans in accordance with the Manual of Uniform Traffic Control Devices (MUTCD), MUTCD “Notice of Proposed Amendments” and any applicable AASHTO or Department standards and guidelines that are current at time of letting. Prepare plan sheets to show all permanent roadway signs and pavement markings as they appear upon completion of the project. Place emphasis on
designing clear directional signage and coordinating sign placement with roadway features, structures, sight distances and driver awareness. All signs are to be replaced within the project limits, unless otherwise approved by the Engineer.

As per the Environmental Commitments, the Contractor shall install a “No Outlet” sign at the entrance to the Brookshire sub-division.

Signing and marking plan sheets are not included in the costing plans, however they were reviewed and comments have been added to the Costing Plan Review Report. Microstation J files for the signing and marking, as well as the Costing Plan Review Report can be obtained through the ftp site described in Section 999.2.A.

Signalization plans are not included in the Costing Plan Package; however, signalization plans are required. All work shall comply with the Department’s Specification Sections for traffic signal equipment, and IVDS intersection video detection. The Contractor shall be responsible for all fees and permits necessary for establishing power to the traffic signal installations. The contractor shall be responsible for all charges associated with monthly utility service to the device until the device has satisfactorily completed a test period of uninterrupted operation, of at least 30 days. Prior to activating new signal equipment, the contractor shall contact the District Six Traffic Signal shop at least 10 working days prior to activation to allow for preliminary inspection of the installation and development of signal timing. The Contractor shall be responsible for providing variable message signs to be placed in advance of each signal installation with text advising motorists of traffic signal activation prior to placing any new traffic signal into flashing operation.
C. Bridges and Structures

1. Design Specifications and Guidelines:
   c. Use “Basic Drawings” where possible. Basic drawings and cells can be downloaded at the following internet address:
   e. Use MicroStation/J to prepare plans in accordance with the Office of Bridge and Structural Design’s MicroStation Customization. These files include a folder structure that is required to be on C:\Drive along with the “Bentley” folder. Access the Bridge MicroStation Customization files at the internet address: http://www.dot.ga.gov/doingbusiness/PoliciesManuals/roads/software/Pages/MicroStation.aspx.

2. Bridge foundation investigation: The Department will supply bridge and wall foundation investigations for information only. If the Contractor proposes a new location for these structures from those shown in the Costing Plans, and the Department determines that additional investigations are required as a result of the change, then the investigation and reporting shall be prepared in accordance with the following:
   a. General:
      1) Perform field and laboratory testing and analysis, and prepare a report with foundation recommendations for the bridges and walls. Work is to be performed by qualified and experienced firms that are pre-qualified with the Georgia DOT in Area Class 6.02.
      2) Perform work in accordance with AASHTO Standards and in general conformance with the Department’s Geotechnical Engineering Bureau Foundation Drilling and Sampling Guidelines. Comply with all applicable Federal and State requirements.
   b. Field Investigation:
      1) Drill a minimum of one boring at each bent line and at each wall. Drill additional borings as necessary. Perform the following, as applicable:
         (a) Notify property owners prior to accessing their properties.
         (b) Obtain locations and clearance for all utilities within the area of the borings.
         (c) Provide traffic control and lane closures in accordance with the Georgia DOT Specifications.
         (d) Clearing and preparation of the boring site.
         (e) Obtaining and transporting water to the site.
         (f) Foundation drilling and sampling of soil and rock.
(g) Obtaining accurate survey elevations.
(h) Site cleanup, erosion control, and restoration.

2) Fill portions of all drill holes with drill cuttings after completion of drilling that are not subject to excavation for construction. Top off all drill holes through pavements with cold mix asphalt (unless subject to excavation) to the same depth as the existing pavement. Remove all drill cuttings, muddy water, slurry, and other debris deposited on pavements, paved shoulders, and other travel ways immediately when the areas shall be subject to traffic after the completion of drilling. Calculate elevations to an accuracy of one tenth (0.1) of a foot.

3) Do not provide copies of boring logs, plans, or field test reports to property owners or other parties without the permission of the State Geotechnical Engineer.

c. Laboratory Testing:

1) Perform laboratory testing on samples obtained from the field in accordance with applicable methods of AASHTO, ASTM, or GDT test procedures. Use a laboratory that possesses current AASHTO certification.

2) Furnish laboratory results as part of the Final Report.

d. Final Analysis and Report:

1) Perform a geotechnical analysis for this project and prepare geotechnical recommendations in the form of a final report to the Department’s State Geotechnical Engineer for review, prior to foundation construction. Base the final report on the information collected from the field investigation, the plans, specifications, results of laboratory tests, and the analysis of all other available information.

2) Stamp and sign the final reports by a Professional Engineer registered in the State of Georgia. Provide copies of the final report to the State Geotechnical Engineer.

3) Prepare the reports in general conformance to the Department’s Geotechnical Engineering Bureau Report Preparation Guidelines, Georgia DOT Specifications, and in conformance with good engineering practice. Incorporate the following recommendations and additional recommendations as applicable

(a) Foundation types and allowable loads.
(b) Footing elevations.
(c) Pile minimum and estimated tip elevations.
(d) Drilled caisson tip elevations.
(e) Foundation installations in rock.
(f) Embankment construction, settlement, and slope angles.
(g) Treatment of groundwater conditions.
(h) Treatment of poor soil conditions.
(i) Construction effects on adjacent structures and remedies for any potential problems.

4) In the Final Report, include (as applicable) copies of boring logs, field notes, laboratory and field test results or summaries, photographs, special provisions, details and drawings, and other related information. Correct final reports with
errors and omissions, as determined by the State Geotechnical Engineer. Resubmit the corrected report at no additional cost to the Department.

5) Acceptance of the work by the Department will not relieve the Contractor of the responsibility for subsequent correction of errors or for the costs associated with work caused by negligent errors or omissions from work performed by the Contractor.

3. Plan Submittals
   a. Preliminary Plans: Preliminary Layout (if required, see below) and Preliminary Wall Plans
   b. Construction Plans: Submit complete bridge plans and complete wall plans
   c. Shop Drawings.
   d. Submit one (1) copy of the design calculations for each scheduled submittal.
   e. Do not proceed with the final design of bridge plans until the preliminary plans have been approved by the Department and FHWA.

4. Preliminary Bridge and Wall Plans
   a. Preliminary Bridge Plans
      The existing bridge carrying Ridgewalk Parkway (Rope Mill) over I-575 shall be replaced. The following information is to be used in the development of the final plans:
      1) The approved Preliminary Layout for the Ridgewalk Parkway (Rope Mill) Bridge over I-575 is included in the contract documents.
      2) The Contractor shall check the Preliminary Layout supplied by the Department in the costing plans. This check shall verify all dimensions and clearances based on field measurements. Notify the Department of any discrepancies that may be present in the Preliminary Layout. The Contractor can accept the Preliminary Layout as provided in the costing plans by notifying the Department in writing. Upon the Contractor’s written acceptance of the Preliminary Layout, the Department will authorize the Contractor to proceed with final design of the bridge.
      3) Should the Contractor choose to change the Preliminary Layout provided in the costing plans and provide an alternate design, the Contractor shall prepare a Preliminary Layout for the Department’s review and approval in accordance with the following guidelines:
         (a) The Contractor shall verify all dimensions and elevations in the field prior to preparing plans, ordering materials or building forms.
         (b) Design the bridge replacement using prestressed concrete beams only. Steel beams will not be allowed.
         (c) Do not increase stresses on existing bridge elements during staging.
         (d) Design the substructure end bents and intermediate bents with concrete columns, caps, or walls with footings. Tops of footings shall be a minimum of two feet below existing grade and possible grade of I-575.
         (e) Provide a minimum vertical clearance of 17'-0" between the bottom of the Ridgewalk Parkway (Rope Mill) bridge superstructure and all current and future lanes on I-575. Contractor shall field survey the existing clearance
over all travel lanes and submit the survey results to the Bridge Office along with the Preliminary Layout.

(f) Provide a typical section which indicates the following information:

- The center to center spacing of girders provided in the costing plans may be adjusted to optimize the beam design. The maximum center to center spacing of the girders is limited to 9'-0".
- Overhang or distance from outside edge of slab to center of exterior girder: This distance (overhang) shall meet AASHTO requirements, but shall not exceed 4'-6" or one half of the adjacent beam spacing, whichever is less.
- Cross slope of the deck.
- Deck thickness between girders and deck thickness at the centerline of girder measured from the top surface of deck to top of the flange.
- Provide a slab with a minimum thickness determined by the Georgia DOT computer program, BRSLAB07, Service Load Design of Concrete Bridge Slabs proportioned to provide 2.75 inches of concrete cover over the top mat of reinforcing and 1 inch cover to the bottom mat of reinforcement (minimum deck thickness is 7 inches). Use the slab thickness determined for the portion of the bridge supporting the highway loading at all locations.
- Parapet location, height and width.
- Gutter to gutter and out-to-out dimensions.
- Location of the profile grade.

(g) In addition to the requirements above, provide the following:

- A plan view of the proposed structure indicating beginning and end bridge stations, construction centerline, profile grade line, bent skew angles, joint locations, station and skew of roadways crossing under the structure, width of roadways beneath the structure, gutter to gutter width of the bridge, out to out width of the bridge, distance from gutter to outside edge of deck, taper control stations, location of point of minimum vertical clearance, and location and magnitude of the horizontal clearances from edge of travel way beneath the structure to the face of intermediate bents.
- Stations and elevations along the centerline of construction at the intersection of the centerline of construction and the back face paving rest and centerline of bents. Provide profile grade elevations corresponding to the above stations.
- An elevation view of the proposed structure indicating the span length, location of fixed and expansion joints, profile of roadways beneath structures, vertical clearance from bottom of structure to roadway beneath, proposed bent locations, and existing ground profile.
- All horizontal and vertical curve data for the bridge and the roadway beneath the bridge.
- The location and elevation of the nearest bench mark. The nearest benchmark shall be within 300 feet of the bridge.
• A brief description of the proposed structure indicating span lengths, and type of end bents.
• Any drawing and/or narrative description of the construction scheme necessary to indicate how the bridge is to be built, including traffic handling sketches and temporary barrier locations.

b. Preliminary Wall Plans

Retaining wall envelopes have been provided in the costing plans. Prepare Preliminary Wall Plans in accordance with the following guidelines:

1) The wall types are as follows:
   (a) Wall 1 - Ramp C (Station 505+80 to Station 507+20) - Cast-in-place concrete, Type 2-C Side Barrier (Ga. STD. 4948B)
   (b) Wall 2 - Ramp D (Station 716+00 to Station 724+00) - MSE (Mechanically Stabilized Earth)
   (c) Wall 3 - Old Rope Mill Road (Station 200+38.58 to Station 203+00) - MSE (Mechanically Stabilized Earth)
   (d) Wall 4 - Ramp D (Station 704+40 to Station 706+20) - Cast-in-place concrete, Type 2-C Side Barrier (Ga. STD. 4948B)
   (e) East (End Bent 1) and West (End Bent 3) Walls - MSE (Mechanically Stabilized Earth)

2) Alternate wall types are permissible as approved by the Department. Soil-nail type walls and modular block type walls will not be allowed.

3) An elevation view or wall envelope of the proposed wall drawn to a scale of 1:10 and indicating the following data:
   (a) Beginning and end wall stations.
   (b) Elevations on top of wall parapet, coping, or traffic barrier at the beginning and end of wall, at profile break points, and at least every 50 feet along the wall.
   (c) Bottom of wall (top of footing) elevation necessary to maintain minimum berm requirements.
   (d) Original ground profile.
   (e) Proposed ground profile.
   (f) Stations and offsets to ends of walls and locations where wall changes direction
   (g) Stations and elevations along top and bottom of wall

4) Roadway cross-sections in the vicinity of the wall that will indicate the existing and final slope behind the wall.

5) Typical sections for MSE walls shall include:
   (a) Limit of special backfill (1'-0" beyond end of reinforcement)
   (b) Reinforcement
   (c) Facing
   (d) Coping, parapet or barrier
5. Final Bridge and Wall Plans

a. Additional Bridge Design Criteria

1) The Contractor’s design professionals, in designing the bridge in this project, shall utilize the Department Bridge Geometric and Design Software to the maximum extent possible. Upon prior written approval by the Department, the Contractor’s design professionals may be authorized to utilize its computer capabilities. The contractor’s design professionals are required to verify results to obtain final design accuracy.

2) Design the bridge replacement for seismic performance category “B”.

3) Use ASTM A 615 Grade 60 reinforcement.

4) Use Class AA Concrete with a minimum 28-day concrete strength of 3,500 psi for superstructure and substructure concrete.

5) Include 30 pounds per square foot in the design loads to allow for future paving.

6) If metal deck forms are used, include 16 pounds per square foot in the non-composite design loads.

7) Design and detail 1’-0” wide edge beams where the deck is to be discontinuous. Detail edge beams as provided in the Bridge Design cell library for the appropriate prestressed beam.

8) Concrete decks shall be made continuous at intermediate bents; no expansion joints at the intermediate bents will be allowed. Continuity shall not be dependent on dead or live load. See Bridge Design memos for details.

9) Place slab transverse and longitudinal reinforcing steel 2 inches from the edge of the slab and place the top mat of edge beam bars below the top mat of the deck.
steel. Do not use truss shaped bars in the edge beam. Extend stirrups from the edge beam into the slab.

10) Use protective platforms in accordance with Section 510 of the GDOT Specifications. Maintain a minimum of 17'-0” vertical clearance over I-575.

11) Use the following in the design and construction of the bridge foundations:
   (a) Foundation Type:
      - Bents 1 & 3: Steel H-pile, Pile Bent
      - Bents 2: Steel H-pile, Pile Footing
   (b) Maximum Design Loads:
      - 10 BP 42 = 55 Tons
      - 12 BP 53 = 70 Tons
      - 14 BP 73 = 96 Tons
   (c) Plan Driving Objective – At each bent, drive all piles to the design driving resistance after achieving the minimum pile tip elevation as follows:

<table>
<thead>
<tr>
<th>Bent Number</th>
<th>Minimum Pile Tip Elevation</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>963</td>
</tr>
<tr>
<td>2 Lt</td>
<td>958</td>
</tr>
<tr>
<td>2 Rt</td>
<td>943</td>
</tr>
<tr>
<td>3 Lt</td>
<td>936</td>
</tr>
<tr>
<td>3 Rt</td>
<td>960</td>
</tr>
</tbody>
</table>

   (d) Use a minimum of one pile per beam location at end bents. Use a minimum of one pile at each wingwall and pile size equivalent to piles supporting beams within the end bent.

   (e) Reinforce pile tips at Bent 3 in accordance with Sections 520 and 855 of the GDOT Specifications.

   (f) Protect piles driven within the limits of MSE walls from negative skin friction, see Section 551 of the Georgia DOT Specifications. Drive these piles before wall leveling pads are constructed.

12) For prestressed beams, meet the following criteria:
   (a) Design prestressed concrete beams with conventional strength concrete up to a maximum 28 day compressive strength of 9,000 psi.

   (b) Design prestressed concrete beams with high performance concrete (HPC) for a maximum 56 day compressive strength over 9,000 psi up to 10,000 psi. The maximum design compressive strengths shall not exceed 10,000 psi.

   (c) Design prestressed beams as simple spans.

   (d) In calculation of prestressed girder section properties, do not utilize transformed area of bonded reinforcement.
(e) Use neoprene bearing pads at each end of the prestressed beams. Design the pads to account for transverse and longitudinal expansion and contraction.

(f) Use anchorage beds set for horizontal and vertical strand patterns of two inches center to center. Detail all straight and draped strands utilizing two inch spacings.

(g) Provide the minimum amount of reinforcing steel at beam ends as required by AASHTO specifications, Article 9.22.

(h) Detail beam lengths to 1/16 inch increments.

(i) Provide prestressed beam sheets with all the applicable details as shown on the “basic drawings.”

(j) Require the use of 10 inch wide concrete diaphragms.

b. Additional Wall Design Criteria

1) MSE Walls are to be constructed in accordance with Section 627 of the GDOT Specifications.

2) Tie Back Walls are to be constructed in accordance with Section 617 of the GDOT Specifications.

3) Concrete Retaining Walls are to be constructed in accordance with Section 500 of the GDOT Specifications.

4) Use the following in the design and construction of the walls:

(a) Wall 1 - Ramp C (Station 505+80 to Station 507+20) – MSE Wall
   • Soil Unit Weight, $\gamma = 120$ pcf
   • Angle of effective internal friction, $\varphi = 32$ degrees
   • Cohesion, $C = 0$ psf
   • Coefficient of sliding friction, $\mu = 0.45$
   • Maximum allowable bearing pressure = 3500 psf
   • For an MSE Wall, if the calculated bearing pressure exceeds the maximum allowable design pressure, build wall to a height equivalent to the maximum allowable design pressure and wait a period of 30 days. After this waiting period, the construct wall to final height.

(b) Wall 2 - Ramp D (Station 716+00 to Station 724+00) – MSE Wall
   • Soil Unit Weight, $\gamma = 120$ pcf
   • Angle of effective internal friction, $\varphi = 32$ degrees
   • Cohesion, $C = 0$ psf
   • Coefficient of sliding friction, $\mu = 0.45$
   • Maximum allowable bearing pressure = 3500 psf (for MSE wall)
   • For an MSE wall, if the calculated bearing pressure exceeds the maximum allowable design pressure, build wall to a height equivalent to the maximum allowable design pressure and wait a period of 30 days. After this waiting period, the construct wall to final height.
(c) Wall 3 - Old Rope Mill Road (Station 200+38.58 to Station 203+00) – MSE Wall
- Soil Unit Weight, $\gamma = 120$ pcf
- Angle of effective internal friction, $\varphi = 32$ degrees
- Cohesion, $C = 0$ psf
- Coefficient of sliding friction, $\mu = 0.45$
- Maximum allowable bearing pressure = 3500 psf (for MSE wall)
- For an MSE Wall, if the calculated bearing pressure exceeds the maximum allowable design pressure, build wall to a height equivalent to the maximum allowable design pressure and wait a period of 30 days. After this waiting period, the construct wall to final height.

(d) Wall 4 - Ramp D (Station 704+40 to Station 706+20) – Conc Side Barrier – TP-2A
- Soil Unit Weight, $\gamma = 120$ pcf
- Angle of effective internal friction, $\varphi = 30$ degrees
- Cohesion, $C = 0$ psf
- Coefficient of sliding friction, $\mu = 0.40$
- Maximum allowable bearing pressure = 2500 psf

(e) End Bent 1 Wall – MSE Wall
- Soil Unit Weight, $\gamma = 120$ pcf
- Angle of effective internal friction, $\varphi = 32$ degrees
- Cohesion, $C = 0$ psf
- Coefficient of sliding friction, $\mu = 0.45$
- Maximum allowable bearing pressure = 3000 psf (for MSE wall)
- For an MSE Wall, if the calculated bearing pressure exceeds the maximum allowable design pressure, build wall to a height equivalent to the maximum allowable design pressure and wait a period of 30 days. After this waiting period, the construct wall to final height.

(f) End Bent 3 Wall – MSE Wall
- Soil Unit Weight, $\gamma = 120$ pcf
- Angle of effective internal friction, $\varphi = 32$ degrees
- Cohesion, $C = 0$ psf
- Coefficient of sliding friction, $\mu = 0.45$
- Maximum allowable bearing pressure = 3500 psf (for MSE wall)
- For an MSE Wall, if the calculated bearing pressure exceeds the maximum allowable design pressure, build wall to a height equivalent to the maximum allowable design pressure and wait a period of 30 days. After this waiting period, the construct wall to final height.

c. Bridge and Wall Construction Plans
The Contractor shall arrange a meeting with the Department to specifically discuss how the plans will be prepared prior to beginning plan preparation on the project.

1) Prepare construction plans with all dimensions, notes and details necessary to construct the structure. As a minimum, include the following sheets:

(a) Plan and Elevation sheets that include:
   - Plan view of the bridge,
   - Elevation view of the bridge,
   - Beginning and ending stations,
   - North arrow,
   - Location of fixed and expansion bearings,
   - Location of the minimum vertical clearance above I-575,
   - Existing Bridge Serial No., Existing Bridge ID No., Project No. Project PI No., and construction ID No. supplied by the Department.

(b) General Notes sheets that include:
   - Notes for the following; Specifications, Reinforcing Steel, Chamfer, Existing Bridge Plans, Welding, Salvage Material, and others as necessary,
   - Bridge Design Data,
   - A summary of Bridge Consists Of,
   - A summary of Traffic Data,
   - A summary of Quantities (for information only)
   - A list of Existing Utilities (if applicable),
   - A list of Utilities

(c) Deck Plan sheets,
(d) Deck Cross-Section sheets,
(e) Bearing assembly sheets,
(f) Beam sheets,
(g) Miscellaneous sheets,
(h) Framing Plan and Substructure Layout sheets,
(i) End Bent/Abutment sheets,
(j) Intermediate Bent sheets,
(k) As Built Foundation sheets, and
(l) Bar Reinforcing Detail sheets.

Additional sheets may be necessary to show the details required for construction. Provide additional sheets at no additional cost when deemed necessary by the Department.

2) Provide the following details:
(a) On deck section sheets, provide one full-width section across the structure which indicates, at least, all the horizontal dimensions necessary to construct the bridge. Provide sufficient deck cross-sections to indicate the staging, location of the existing structure and location of any temporary barriers on the structure.

(b) Show as many sections as are necessary to detail the placement of reinforcing in the deck and barrier. Also, draw deck sections indicating edge beams, back walls, diaphragms, and end walls. Cut sections radially across the structure.

(c) Detail deck plan sheets with all longitudinal and transverse dimensions necessary to construct the bridge, including edge beam width, expansion joint widths, back wall or end wall locations, location of construction and expansion joints, and any other items that are necessary to construct the structure.

(d) All views, sections and details, except those in GDOT’s standard bridge cell library, are to be drawn to scale. Draw deck cross-sections and intermediate bent sheets “Looking Ahead”. If the end bents or abutments are drawn separately, draw bent/abutment one “Looking Back”, and draw the other end bent/abutment “Looking Ahead”.

(e) All details on the Plans shall be clear and legible. The Department will have the final say as to how a Project is to be drawn and will have the right to require additional drawings at no increase in Contract cost. Fully check the plans for completeness of content and accuracy before submittal to the Department for review.

3) Maintain and protect all utilities supported and in the area of the bridge during construction. Consider the installation of utilities in staging the construction of the bridge.

4) Groove the bridge deck in accordance with Section 500 of the Georgia Specifications.

6. Shop Drawings

Provide shop drawings in accordance with Georgia DOT Specifications. The Contractor’s engineer shall review and stamp approved all shop drawings as the Engineer of Record. After being stamped by the Contractor’s design engineer, the Department will review the shop drawings for conformance with the plans and specifications. Allow the Department a 30 day review period upon receipt of the shop drawings for each submittal.

7. Bridge Removal

No materials removed from the existing structure are to be salvaged for use by the Georgia DOT. The Contractor is responsible for the removal and disposal of all material removed from the existing bridge. There is no suitable location within project limits for the disposal of the existing bridge. The Contractor shall be responsible for locating and obtaining a suitable location for the disposal of the existing bridge. The Contractor shall be responsible for obtaining all necessary permits associated with the disposal.
D. Utilities

1. The Contractor shall have the responsibility of coordinating the project construction with all utilities that may be affected. Coordinating responsibilities shall include but not be limited to the following:

   a. The Contractor shall initiate early coordination with all Utility Owners located within the project limits. All Utility Coordination shall be performed to GDOT standards by a prequalified firm in Area Class 3.10 - Utility Coordination. Refer to the following website for a list of current prequalified firms: http://www.dot.state.ga.us/doingbusiness/consultants/Pages/default.aspx

   b. The Contractor shall be responsible for the cost of Utility Coordination. Coordination shall include, but shall not be limited to, contacting each Utility Owner to advise of the proposed project; supplemental verification of the locations of existing utility facilities (including the employment of additional Overhead/Underground Subsurface Utility Engineering investigations (SUE) as described in section 999.3.D.3.c of this specification); and determining requirements for the relocation or adjustment of facilities.

   b. The Department and/or the Utility Owner shall be responsible for the cost of utility relocation (this may change according to the details contained in the MOUs), where they hold a property interest, and in accordance with the Department's "Utility Accommodation Policy and Standards Manual". Details are provided in the attached Memorandum of Understanding (MOU) executed between the Department and each Utility Owner.

   c. The Contractor shall endeavor to design the project to avoid conflicts with utilities when feasible, and minimize impacts where conflicts cannot be avoided (See Section 999.3.D.2.c). The Contractor shall submit to the Department a SUE Utility Impact Analysis (UIA) in the Department’s prescribed format within 90 days of notice to proceed (see TABLE 4-1: REVIEWS).

   d. The Contractor shall coordinate and conduct a preliminary review meeting with the Utility Owners to assess and explain the impact of the project. The Department's Project Manager, District Construction Engineer (or designee), and District Utilities Engineer (or designee) shall be included in this meeting. Knowledge of the project environmental “Commitments/ Requirements” (Green Sheets) is essential for Utility Owners during their design phase. The Contractor shall provide the Environmental Commitments table, and any re-evaluation with all Utility Owners. Also, during the preliminary review meeting Utility Owners are particularly interested in the status of Right of Way acquisition and its direct effect on their relocation design. The Contractor shall develop a status report of the Right of Way acquisition process, for Utility Owners use in planning for relocations. The Contractor shall record the minutes for this meeting and distribute to all attendees for their review and concurrence.

   e. The Contractor shall research the property interests of each Utility Owner's facilities. If there is a dispute over property interests with a Utility Owner, the Contractor shall be responsible for resolving the dispute. The Contractor shall meet with the Department's District Utilities Engineer (or designee) to present the property interests information gathered. This information must be sufficient for the District Utilities Engineer (or designee) to certify the extent of the Utility Owner's property interests. The Department shall have final approval authority as to the Contractor's determination of whether the Utility Owner has property interests.

   f. The Contractor shall prepare and submit to the Department a Preliminary Utility Status Report within 160 days after the Notice to Proceed has been given for the
contract (See TABLE 4-1: REVIEWS). This report shall include a listing of all Utility Owners located within the project limits and a recommendation as to the extent of each Utility Owner's property interests. This report shall include copies of easements, plans, or other supporting documentation that substantiates any property interests of the Utility Owners. The report shall also include a preliminary assessment of the impact to each Utility Owner.

g. Depending on the provisions stipulated in the Memorandum of Understanding (MOU – See Attached) between the Department and each Utility Owner the Contractor shall be responsible for one of the following Design Activities:

1) The Contractor shall provide Utility Owners with design plans and Preliminary Utility Plans as soon as the plans have reached a level of completeness adequate to allow them to fully understand the project impacts. The Utility Owner will use the Contractor’s design plan for preparing Utility Relocation Plans, cost estimates, and respective Utility Adjustment Schedules (UAS). If a party other than the Utility Owner prepares Utility Relocation Plans, there shall be a concurrence box on the plans where the Utility Owner signs and accepts the Utility Relocation Plans as shown.

2) The Contractor shall prepare all engineering design, plans, technical specifications, cost estimates, and utility adjustment schedules required to perform the necessary utility relocations. The Contractor shall certify to the Department that the design package listed above has been reviewed and accepted by the each respective Utility Owner.

h. The Contractor shall be responsible for collecting the following from each Utility Owner that is located within the project limits: Certified Utility Relocation Plans including a letter of "no cost" where the Utility Owner does not have a prior right; Utility Agreements, certificates of eligibility, including cost estimate and Utility Relocation plans where the Utility Owner has a property interest; Letters of "no conflict" where the Utility Owner's facilities will not be impacted by the Project. The Contractor shall prepare and submit to the Department a Utility Retention Request for any utility which is to remain under the roadway within the construction limits.

i. The Contractor shall be responsible for determining if the Department has agreed to pay for in-kind relocations according to any approved Utility-Aid assistance package for publicly (government) owned utilities found within the project’s limits (See the Department’s (Policies & Procedures) TOPPS Policy #6863-11 for additional information regarding Utility-Aid). If the Department has approved Utility-Aid; it is the Contractor's responsibility to assemble the necessary information including any Utility Agreements in a final and complete form and in such a manner that the Department may approve the submittals with minimal review. Failure to submit such required Utility Agreements prior to the beginning of construction shall fully transfer the utility owner's obligations, as stated in the subject Utility-Aid assistance package, to the Contractor. Deductions to reimburse the Department for such obligations may be made from any current partial payment of the Lump Sum price.

j. The Contractor shall review all Utility Relocation Plans and Utility Agreements, Utility Estimates and certificates of eligibility to ensure that relocations comply with the Departments “Utility Accommodation Policy and Standards Manual”. The Contractor shall review the utility plans to identify that there are no conflicts with the proposed highway improvements, and ensure that there are no conflicts between each of the Utility Owner's relocation plans. The Contractor shall show all existing and proposed utilities on the cross sections and drainage profiles.
k. The Contractor shall compile, and submit to the Department all SUE deliverables, Utility Relocation Plans, SUE Utility Impact Analysis, Utility Adjustment Schedules, Utility Agreements, Utility Estimates (if estimates are provided by the utility owners), and Letters of "no conflict," as set forth above for the project. The Contractor is expected to assemble the information included in the Utility Agreements and Utility Relocation Plans in a final and complete form and in such a manner that the Department may approve the submittals with minimal review. The Contractor shall schedule a meeting with the Department's District Utilities Office and the State Subsurface Utilities Engineer (or designee) for a SUE Kick-Off meeting within 15 days of the Notice to Proceed to gain a full understanding of what is required with each submittal. The Utility Owners shall not begin their Utility Relocation work until authorized in writing by the Department.

l. Each Utility Agreement and Utility Relocation Plan submitted shall be accompanied by a certification from the Contractor and the Utility Owner stating that the proposed relocation will not conflict with the proposed highway improvement and will not conflict with another Utility Owner’s relocation plan.

m. Depending on the provisions stipulated in the Memorandum of Understanding (MOU – See Attached) between the Department and each Utility Owner the Contractor shall be responsible for one of the following construction activities:

1) The Contractor shall be responsible for coordinating the work of its subcontractors and the various Utility Owners. The resolution of any conflicts between Utilities and the construction of the project shall be the responsibility of the Contractor. No additional compensation will be allowed for any delays, inconveniences, or damage sustained by the Contractor or its subcontractors due to interference from utilities or the operation of relocating utilities.

2) The Contractor shall be responsible for performing all utility removal, relocation, and adjustments required to accommodate the proposed project. This shall include any required inspection, permitting, testing and monitoring to ensure that the work is properly performed to the certified design package. The resolution of any conflicts between Utilities and the construction of the project shall be the responsibility of the Contractor. No additional compensation will be allowed for any delays, inconveniences, or damage sustained by the Contractor or its subcontractors due to interference from utilities or the operation of relocating utilities.

n. During the construction of the project, the Contractor shall designate, prior to beginning any work, a Worksite Utility Coordination Supervisor (WUCS) who shall be responsible for initiating and conducting utility coordination meetings and accurately recording and reporting the progress of utility relocations and adjustment work. Also, the WUCS shall prepare an Emergency Response Plan for the purpose of planning, training, and communicating among the agencies responding to the emergency. The WUCS shall be the primary point of contact between all of the Utility companies, the Contractor and the Department. The WUCS shall recommend the rate of reoccurrence for utility coordination meetings and the Engineer will have the final decision on the regularity for utility coordination meetings. In no case will utility coordination meetings occur less than monthly until controlling items of utility relocations and adjustment milestones are completed. The WUCS shall contact each of the utility companies for the purpose of obtaining information including, but not limited to, a Utility Adjustment Schedule for the controlling items of utility relocations and adjustments. The WUCS shall notify the appropriate utility company and/or utility subcontractors and the Department of the status of controlling items of relocations and adjustment milestones as they are completed. The WUCS shall furnish the
Engineer, for approval, a Progress Schedule Chart, prior to beginning Construction unless otherwise specified, which includes the utility companies controlling items of work and other information in accordance with Section 108.03 or elsewhere in the Contract documents. Duties and Responsibility of the Worksite Utility Coordination Supervisor, (WUCS):

1) **Qualifications:** The WUCS shall be an employee of the Prime Contractor, shall have at least one year experience directly related to highway and utility construction in a supervisory capacity and have a complete understanding of the Georgia Utilities Protection Center operations, and shall be knowledgeable of the High-voltage Safety Act and shall be trained on the Georgia Utility Facility Protection Act (GUFPA). The Department does not provide any training on GUFPA but will maintain a list of the Georgia Public Service Commission certified training programs developed by other agencies. Currently the following companies offer approved GUFPA training programs:

   Associated Damage Consultants  
   Phone: 706.234.8218 or 706.853.1362

   Georgia Utility Contractors Association  
   Phone: 404.362.9995

   Georgia Utilities Protection Center  
   Phone: 678.291.0631 or 404.375.6209

   H B Training & Consulting  
   Phone: 706.619.1669 or 877.442.4282 (Toll Free)

   The Prime Contractor is responsible for obtaining the GUFPA training for their employees.

   Questions concerning the Georgia Public Service Commission GUFPA training program shall be directed to:

   Georgia Public Service Commission  
   244 Washington St. SW  
   Atlanta, GA 30334-5701  
   404.463.9784

2) **Ticket Status:** During the utility coordination meetings the WUCS shall collect and maintain the Ticket Status information to determine the status of all locate requests within the project limits. This information will be used to assure those planning to use mechanized equipment to excavate or to work within the project limits are prepared to begin work when they have reported or estimated beginning work. At points where the Contractor’s or utility company’s operations are adjacent to or conflict with overhead or underground utility facilities, or are adjacent to other property, damage to which might result in considerable expense, loss, or inconvenience, work shall not commence until all arrangements necessary for the protection thereof have been made.

3) **Notice:** The names of known utility companies and the location of known utility facilities shall be shown on the Plans, or listed in the Subsurface Utility Engineering Investigation if performed or in the Special Provisions; and the WUCS shall give 24-hour notice to such utility companies before commencing work adjacent to said utility facilities which may result in damage thereto. The WUCS shall further notify utility companies of any changes in the Contractor’s work schedules affecting required action by the utility company to protect or
adjust their facilities. Notice to the utility companies by the Department of the Award of Contract, under Subsection 105.06, shall not be deemed to satisfy the notice required by this paragraph. Furthermore, this 24-hour notice shall not satisfy or fulfill the requirements of the Contractor as stated in Chapter 9 of Title 25 of the Official Code of Georgia Annotated, known as the “Georgia Utility Facility Protection Act”.

4) **Agenda:** The WUCS shall cooperate with the companies of any underground or overhead utility facilities in their removal and relocations or adjustment work in order that these operations may progress in a reasonable manner, that duplication of their removal and relocations or adjustment work may be reduced to a minimum, and services rendered by those parties will not be unnecessarily interrupted. To promote this effort the WUCS shall prepare an agenda for the utility coordination meetings and circulate same in advance of the meeting to encourage input and participation from all of the utility companies. The agenda will be prepared by an examination of the project site and may include photographs of potential/actual utility conflicts.

5) **Emergency Response Plan:** The WUCS shall prepare and submit to the Department an Emergency Response Plan no later than 30 days prior to beginning construction. The WUCS shall clearly mark and highlight the gas, water and other pressurized pipeline shut-off valves and other utility services including overhead switch locations on the utility plans; and prepare a chart to indicate the location of each site (Street address or intersections), the utility company or operator of the facility with emergency contact information and the working condition of the device to facilitate prompt shut-off. The WUCS shall post the Emergency Response Plan in an area readily accessible by the Department. In the event of interruption to gas, water or other utility services as a result of accidental breakage or as a result of being exposed or unsupported, the WUCS shall promptly notify the appropriate emergency officials, the Georgia Utilities Protection Center and the appropriate utility facility company or operator, if known. Until such time as the damage has been repaired, no person shall engage in excavating or blasting activities that may cause further damage to the utility facility.

6) **Submission:** Provisions for reporting all utility coordination meetings, the progress of utility relocation and adjustment work milestones and ticket status information shall be reported on a form developed by the WUCS and will be distributed by the WUCS to all of the utility companies as milestones are met and shall be included as part of the project records. These reports shall be delivered to the Engineer for review, on a monthly basis. The WUCS shall immediately report to the Engineer any delay between the utility relocation and adjustment work, the existing Utility Adjustment Schedule, or the proposed Utility Adjustment Schedule so that these differences can be reconciled.

7) **Utility Adjustment Schedule:** The purpose of the Utility Adjustment Schedule is to provide the Contractor with the pertinent information, including any utility staging required, dependent activities, or joint-use coordination that is required for the creation of a progress schedule chart that is feasible. A suitable Utility Adjustment Schedule form is available from the Department for the WUCS to circulate to utility companies for any proposed project construction staging. The WUCS shall submit the Progress Schedule Chart in accordance with Section 108.03 and the proposed Utility Adjustment Schedules from all utility companies to the Engineer for review and approval.
o. At the time the Contractor notifies the Department that the Contractor deems the Project to have reached Final Completion, the Contractor shall certify to the Department that all Utilities have been identified and that those Utility Owners with property interests or other claims related to relocation or coordination with the project have been relocated or their claims otherwise satisfied or shall be satisfied by the Contractor.

p. The Contractor shall show the final location of all utilities on the As-Built drawings for the project as stated in Section 999.3.A.8.

q. In addition to the above, the Contractor shall comply with all provisions set forth under subsection 107.21 of the Georgia Department of Transportation’s Specifications, Construction of Transportation Systems, current edition.

r. The Contractor shall be responsible for determining if the Department has agreed to a Project Framework Agreement (PFA) with Local Government or, additional Specific Activity Agreements (SAA) within the project’s limits (See the Department’s (Policies & Procedures) TOPPS Policy #7120-3 for additional information). If the Department has approved a PFA or SAA; it is the Contractor’s responsibility to assemble the necessary information including any Utility Agreements in a final and complete form and in such manner that the Department may approve the submittals with minimal review. Failure to submit such required Utility Agreements prior to the beginning of construction shall fully transfer the obligations, as stated in the subject PFA or SAA package, to the Contractor. Deductions to reimburse the Department for such obligations may be made from any current partial payment of the Lump Sum price.

2. General

a. By Georgia Statues, utilities whether public or privately owned, aerial or underground, are permitted by the Department and local governments to be accommodated within the public right of way. To this end, the Contractor needs to make every effort to design/build a project that will accommodate (and minimize impacts to) all existing utilities and new utilities to be constructed concurrently with the project. The selection of typical section features, horizontal alignment, and location of storm sewer lines are design elements that can sometimes be varied without violating safety standards, and accepted design principles. Design/construction techniques that minimize or avoid utility conflicts may involve increased upfront costs; however, those costs are offset by savings during construction, in addition to the total cost savings for the project (the Department or local government) and the respective utility owners.


c. The Utility Plans are used as the primary tool to identify and resolve utility related conflicts/issues prior to beginning the construction of a project. Also, when these plans are properly prepared as indicated in this Special Provision; they will support the vital coordination required between the Contractor and the Utility Owner during construction. Existing utility information shown on the utility plans for this project have been obtained from an Overhead / Subsurface Utility Engineering (SUE) Investigation (please refer to Section 3.C. for more information on SUE). This existing utility information has been provided by the Department for the Contractor’s use in the design and construction of this project. However, the Contractor shall be responsible for supplementing this utility information for utilities that have been installed after the Overhead / Subsurface Utility Engineering (SUE) Investigation was
performed. Known utilities and contacts are shown in the Costing Plans Package. This information shall be verified by the Contractor.

d. Utility plan sheets are comprised of completed roadway plan sheets but shall contain more detailed information featuring existing and proposed utility facilities. Specific requirements for Utility Plans are detailed below.

3. Required Information

a. Preliminary Utility Plans

1) Preliminary Utility Plan sheets are typically comprised of preliminary roadway plan sheets with the inclusion of all existing utility facility locations (overhead & underground) found within a project’s limits. Determining the location of the existing utilities shall be accomplished through an Overhead/Subsurface Utility Engineering Investigation. The “degree of effort” exerted on the part of the Department and the Utility Owner varies with the type and location of the utility. The Department has classified these “degrees of effort” into different Quality Levels of information. Please refer to Section 2.C. for definitions of these Quality Levels.

2) Preliminary Utility Plans shall be produced and used by the Contractor in the utility coordination/relocation design activities outlined here and under Section 999.1.D. The following minimum information shall be shown on the Preliminary Utility Plans:

(a) Construction centerline with project stations and begin/end project limits.
(b) Curb and gutter or edge of pavement (proposed and existing)
(c) Road and street names
(d) Existing and Required Right of Way limits, property lines, environmentally sensitive area limits, and property owners.
(e) All proposed and existing easements (including existing utility easements)
(f) Proposed and existing drainage structures/features (excluding drainage text)
(g) Proposed construction limits (C/F lines)
(h) Topographical planimetrics (i.e. existing buildings / structures, existing tree/vegetation limits)
(i) All proposed bridges, walls, other structures and landscape hardscapes.
(j) All proposed and existing strain poles (signal, sign, lighting)
(k) Utilities Legend
(l) Miscellaneous General Notes
(m) Existing overhead and underground utilities found within the project’s limits, including size and material if known.
(n) Sanitary sewer manhole top, and invert elevations. Sanitary Sewer pipe flow directions
(o) Railroad mainline and spur tracks with their respective property/easement limits
(p) Project Survey control point locations
(q) SUE specific General Notes
(r) Utility Pole Data Table
(s) SUE investigation Limit of study
(t) SUE Quality Level A information

b. Final Utility Plans

1) Final Utility Plans consist of all the elements provided for in the Preliminary Utility Plans, but also show all proposed utility adjustments required to accommodate the project.

2) The proposed utility information shall either be provided to the Contractor by each of the respective Utility Owners. Refer to this section to determine how proposed utility relocation design information is to be provided. In either case the Contractor shall compile and incorporate this information into the project’s Final Utility Plans.

3) The proposed utility work for this project shall either be performed by the Utility Owner or their designated contractor, or included as part of the project’s construction contract. Refer to Section 999.1.D or to the Memorandum of Understanding (MOU) to determine who is responsible for the proposed utility relocation work for this project.

4) In either case, the Final Utility Plans shall clearly show all existing, proposed, temporary, and relocated utilities on the plans and clearly indicate the disposition of all existing utilities: for example, “To be removed”, “To be Adjusted”, “To be Abandoned”, “To Remain”, “To be Relocated”, etc. The plans shall also clearly define utility work as to which is to be done by the Contractor and which is to be done by others. Utilities to be relocated (or removed, or installed) prior to construction shall be labeled on the plans as “To be relocated (or removed or installed) by others prior to project construction”.

5) When proposed utility work is included as part of the project’s contract, it is necessary for a Summary of Quantities to be included within the Final Utility Plans. The Summary of Quantities shown in the Final Utility plans shall be prepared in the same basic format as indicated in Section 999.2.B.1.

6) Where extensive or complex utility work is proposed to be performed, separate Utility Relocation Plan Sheets for that specific utility may be required to ensure plan legibility/constructability. The Contractor shall determine whether separate Utility Relocation Plans are needed. However, after review of the plans, the Engineer may require these additional sheets or drawing inserts to be included in the project plan package.

7) In addition to the information required for the Preliminary Utility Plans, the Final Utility Plans shall include the following:

(a) All proposed and temporary utility facilities with annotation describing nature of work.

(b) Miscellaneous General Notes required for coordination of utility facilities with roadway construction.

(c) Proposed water and sanitary sewer plan/profiles.

(d) Summary of Quantities for contract items (if applicable).

(e) Any proposed utility easements.

(f) Any miscellaneous proposed utility details.

c. Overhead/Subsurface Utility Engineering (SUE) Investigations
Employ an established engineering technology that can provide precise horizontal and vertical locations of underground and overhead utilities to produce an accurate picture of the underground and overhead utility infrastructure. The existing utility information provided in these investigations includes a description of what “degree of confidence” there is in its accuracy. The Department has classified these “degrees of confidence” into different Quality Levels of information:

1) Quality Level "D" Information - Information obtained solely from a review of utility records and field verification. The comprehensiveness and accuracy of such information is highly limited. Even when existing information for a utility in a particular area is accurate, there are often other underground systems that are not shown on any records. Quality Level “D” may be appropriately used early in the development of a project to determine the presence of utilities.

2) Quality Level "C" Information - Information obtained to augment Quality Level “D” information. This involves topographic surveying of visible, above-ground utility features (e.g., poles, hydrants, valve boxes, circuit breakers, etc.) and entering the topographic data into the CADD system. Since aerial utility lines are not surveyed, information provided for these facilities is considered Quality Level “C” also. Quality Level “C” may be appropriately used early in the development of a project and shall provide better data than Quality Level “D” information alone. Designers shall be very cautious when working on projects using information for underground utilities that is based only on Quality Levels “D” and “C” locates.

3) Quality Level "B" Information - Information obtained through the use of designating technologies (e.g., geophysical prospecting technologies). This is an application using scanning technologies, most of which have very specific capabilities. Applying a variety of techniques is essential to the process of preparing a comprehensive horizontal map of utilities and other underground structures on the site. Designating technologies are capable of providing good horizontal information.

4) Quality Level "A" (Test Hole) Information - Provides the highest level of accuracy of utility locations in three dimensions. This level may apply manual, mechanical or nondestructive (e.g., vacuum excavation) methods to physically expose utilities for measurement and data recording. Quality Levels “B”, “C”, and “D” locates are incorporated in Quality Level “A” locates.

5) The Contractor shall identify all utility conflict points where verified existing utility information is necessary to avoid/minimize/identify the respective utility conflict. The Contractor shall obtain Quality Level “A” locates at these project/utility conflict points, and shall coordinate with the Utility Owners and make every effort to avoid existing utility facilities and thereby reduce utility relocations.

6) All Overhead/Subsurface Utility Engineering (SUE) shall be performed to GDOT standards by a prequalified firm in Area Class 5.08. Refer to the following website for a list of current prequalified firms: http://www.dot.state.ga.us/doingbusiness/consultants/Pages/default.aspx

4. Sheet Layout

a. The Contractor needs to ensure that any information and graphic data that is not necessary to depict the disposition of utilities found within the project’s limits is removed by turning off the appropriate CADD levels(s) on which the data is stored. This will help ensure that information pertinent to utility facilities can be clearly seen in the Utility Plan sheets. Examples of extraneous information would be items such as horizontal curve data, superelevation data, roadway dimensions, misc. text, etc. All background information such as pavement limits, existing structures, etc. shall be
screened back. Also, the Contractor shall ensure all text, line work, details, and symbols are clear and legible when plans are reduced to ½ size.

b. In order to maintain plan clarity all applicable general notes, tables, and the Utility Legend shall be placed separately from the Utility Plan sheets. A Utility Plan “Cover Sheet” shall be provided for both preliminary and final Utility Plans. A recommended example utility sheet schedule is provided below:

1) Utility Sheet 1 (Cover Sheet) – Utility General Notes, Utility Legend, Miscellaneous Details
2) Utility Sheet 2 (required as needed) – Additional Miscellaneous Details, Pole Data Table
3) Utility Plan Sheets – Utilities shown in plan view with respect to project.
4) Utility Profile and Cross Sections Sheets - Proposed Utility facility profiles and cross sections (as required)
5) Miscellaneous Utilities Sheets – Miscellaneous proposed utility details (as required).

The above sheet schedule shall also be generally followed for all separate utility relocation plans (i.e. water & sewer plans) included in the project plans.

5. Miscellaneous Notes and Other Information

a. Note on the Utility Plans whose responsibility it is for utility adjustment. For bridge plans required, the Contractor is to make sure the plans have made accommodations for utility crossings and attachments, if applicable. Any new utility crossings requests shall include the size, weight, and type of utility. In addition, the method of attachment to the bridge shall be fully detailed. Such requests shall be reviewed by the Contractor to ensure adequacy and constructability and final approval shall be obtained by the Contractor from the Department. The Contractor shall follow the approval process within this specification. The Contractor is responsible to ensure that all proposed and existing utilities are coordinated with the respective project's Construction Staging Plans and Erosion Control Plans.

b. Upon completion of the Utility Relocation Plans, the Contractor needs to ensure that any additional environmental impacts due to utilities are addressed in the project's environmental document/permit.
999.4 CONSTRUCTION

The Contractor shall construct the project as per the project scope and as per the accepted Released for Construction plans in accordance with the Specifications. No construction shall begin on any phase of the work prior to the Department providing written authorization to the Contractor to begin land disturbing activities. Two (2) full size and four (4) half size sets of the Released for Construction plans shall be delivered to the Department’s Area Office at least 1 (one) week prior to the Contractor performing initial land disturbing activities. In addition, the Contractor shall deliver all subsequent Released for Construction plans at least 24 (twenty four) hours before commencing land disturbing activities. All plans submitted to the Area Office for use on construction shall include all applicable Standards and Details required in the Work.

Construction includes, but is not limited to, the following:

A. All clearing and grubbing and grading required in accordance with Sections 201, 202, 205, 206, 208 and 209.

B. All necessary grading and drainage (All proposed pipes shall be concrete) to construct the subgrades, including the removal and replacement of unsuitable material, shoulders and incidental work to include furnishing borrow pits, waste disposal areas and hauling borrow and waste materials as required. The removal and replacement of unsuitable material is the responsibility of the Contractor.

C. All necessary culvert extensions including the removal and replacements of headwalls, aprons and rip rap. Existing culverts shall be analyzed for structural sufficiency for new fills. Existing portions of culvert shall be removed and replaced with appropriate excavation and shoring as needed.

D. All necessary base construction, milling, leveling, asphalt paving and concrete paving to construct the pavement structure.

E. Removal of all curbs, drainage structures, pavements, bases and sub-bases, or other obstructions within the rights of way as necessary to construct the roadway section.

F. All signing, Interstate signage including sign structures, signalization, pavement marking, raised pavement markers, and guardrail.

G. All equipment and materials stored on the project shall be stored outside of the active clear zone.

H. Errors and omissions are the responsibility of the Contractor to correct and at the expense of the Contractor.

I. No items are anticipated to be salvaged by the Department. All salvage material shall be disposed of properly by the Contractor in accordance with all Local, State and Federal laws.

J. Preparation of As-Built Construction Plans.

M. As stipulated within the Environmental Commitments table and described within Special Provision 107.23g, protection of the Cherokee Darter, no herbicides, pesticides or fertilizers shall be used within the established Environmentally Sensitive Area (ESA) for Stream #5.
MEASUREMENT AND PAYMENT

The Work required under the Specification shall not be measured separately for payment unless otherwise specified. Payment for the items listed below, complete and accepted, shall be made at the Lump Sum price bid. Payment shall be full compensation for furnishing all materials, labor, tools, equipment, superintendence, mailing charges, removal and replacement of unsuitable material and other incidentals. It shall also be made for performing all work specified, including but not limited to, designing, detailing, producing construction plans (preliminary and final, electronic and hard copy), meeting with the Department, processing the NOI and complete construction. For all asphaltic concrete, when materials or construction are not within the tolerances specified in Sections 400 and 402, deductions shall be made in accordance with the applicable requirements of Sections 106, 400 and 402. The deduction will be determined by the following formula:

\[ \text{Deduction (per ton)} = (1 - \text{Pay Factor}) \times \frac{\text{Assumed Unit Price/Ton}}{} \]

(See Chart Below)

<table>
<thead>
<tr>
<th>Material</th>
<th>Assumed Unit Price/Ton</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphaltic Concrete 12.5 mm Superpave</td>
<td>$63.00</td>
</tr>
<tr>
<td>Asphaltic Concrete 12.5 mm Superpave with Polymer</td>
<td>$74.00</td>
</tr>
<tr>
<td>Asphaltic Concrete 19 mm Superpave</td>
<td>$60.00</td>
</tr>
<tr>
<td>Asphaltic Concrete 25 mm Superpave</td>
<td>$62.00</td>
</tr>
<tr>
<td>Asphaltic Concrete 12.5 mm SMA</td>
<td>$101.00</td>
</tr>
<tr>
<td>Asphaltic Concrete 12.5 mm leveling</td>
<td>$67.00</td>
</tr>
</tbody>
</table>

Partial payments of the Lump Sum price shall be made on monthly statements based on an accepted schedule of values. The Contractor shall develop a schedule of values with sufficient breakdown for each of the following items:

- DESIGN COMPLETE
- CONSTRUCTION COMPLETE

The schedule for values shall include a rational basis for partial payments of the Lump Sum bid based on the completed portion of the item and definitive activities. The schedule for values shall be submitted to the Engineer and no payments shall be made until the schedule of values is accepted. The schedule of values shall also include items for the design and construction of the City of Woodstock’s utility relocations.

Contractor shall work with the Engineer to establish estimated earthwork, asphalt, and concrete quantities, as this will determine the frequency of required testing by the Department.

At the end of each calendar month, the Contractor shall provide the Department with a certification showing the percent complete for each item of work. The Contractor shall include a breakdown and supporting documentation, to include the Design Consultant’s monthly invoice, in sufficient detail to substantiate the percent complete certified.

Payment shall be made under:

- Item 999, DESIGN COMPLETE ................................................................. per Lump Sum
- Item 999, CONSTRUCTION COMPLETE ..................................................... per Lump Sum
AMENDMENT TO ADVERTISED CONTRACT

CONTRACT I.D. NUMBER: B13668-10-000-0
GEORGIA PROJECT NUMBER: CSNHS-0006-00(043)
PCN: 0006043.01000
COUNTY: CHEROKEE
AMENDMENT NUMBER: 1
LETTING DATE: MARCH 19, 2010
LETTING NUMBER: 001

THE FOLLOWING CHANGES ARE HEREBY MADE TO THIS CONTRACT. THE BIDDER IS RESPONSIBLE FOR MAKING ANY NECESSARY CHANGES IN INK IN THE PROPOSAL. BIDDER SHALL ACKNOWLEDGE THIS AMENDMENT BY CHECKING THE APPROPRIATE SPACE ON THE PROPOSAL SIGNATURE PAGE.

1. Proposal Page 563, Special Provision Section 999 Design-Build, Subsection 999.1.C; Add the following items:

33. The typical pavement sections shall be as follows:

<table>
<thead>
<tr>
<th>I-575 Auxiliary Lane</th>
<th>Outside Shoulders</th>
</tr>
</thead>
<tbody>
<tr>
<td>Mainline</td>
<td>Material</td>
</tr>
<tr>
<td>12.5 mm OGFC</td>
<td>12.5 mm OGFC</td>
</tr>
<tr>
<td>12.5 mm SMA</td>
<td>12.5 mm Superpave</td>
</tr>
<tr>
<td>19 mm Superpave</td>
<td>19 mm Superpave</td>
</tr>
<tr>
<td>25 mm Superpave</td>
<td>25 mm Superpave</td>
</tr>
<tr>
<td>Graded Aggregate Base - 12” Depth</td>
<td>Graded Aggregate Base - 12” Depth</td>
</tr>
</tbody>
</table>

Old Rope Mill Road (Relocation)

<table>
<thead>
<tr>
<th>Mainline</th>
<th>Material</th>
<th>Rate</th>
<th>Outside Shoulders</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Material</td>
<td>Rate</td>
<td>Curb and gutter with graded aggregate base at 10” depth.</td>
</tr>
<tr>
<td>12.5 mm Superpave</td>
<td>12.5 mm Superpave</td>
<td>165 lb/sy</td>
<td></td>
</tr>
<tr>
<td>19 mm Superpave</td>
<td>19 mm Superpave</td>
<td>220 lb/sy</td>
<td></td>
</tr>
<tr>
<td>25 mm Superpave</td>
<td>25 mm Superpave</td>
<td>330 lb/sy</td>
<td></td>
</tr>
<tr>
<td>Graded Aggregate Base - 10” Depth</td>
<td>Graded Aggregate Base - 10” Depth</td>
<td>-</td>
<td></td>
</tr>
</tbody>
</table>
### AMENDMENT TO ADVERTISED CONTRACT (continued):

<table>
<thead>
<tr>
<th>Material</th>
<th>Rate</th>
<th>Material</th>
<th>Rate</th>
</tr>
</thead>
<tbody>
<tr>
<td>12.5 mm Superpave</td>
<td>165 lb/sy</td>
<td>Curb and gutter with graded</td>
<td>aggregate base at 12&quot; depth.</td>
</tr>
<tr>
<td>19 mm Superpave</td>
<td>220 lb/sy</td>
<td></td>
<td></td>
</tr>
<tr>
<td>25 mm Superpave</td>
<td>440 lb/sy</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Graded Aggregate Base</td>
<td>-</td>
<td></td>
<td></td>
</tr>
<tr>
<td>- 12&quot; Depth</td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Material</th>
<th>Rate</th>
<th>Material</th>
<th>Rate</th>
</tr>
</thead>
<tbody>
<tr>
<td>Plain PC Conc Pvmnt</td>
<td>-</td>
<td>Plain PC Conc Pvmnt</td>
<td>9&quot;</td>
</tr>
<tr>
<td>- 9&quot; Depth (Class 3)</td>
<td></td>
<td>- 9&quot; Depth</td>
<td>(Class 3)</td>
</tr>
<tr>
<td>19 mm Superpave</td>
<td>330 lb/sy</td>
<td>19 mm Superpave</td>
<td>330 lb/sy</td>
</tr>
<tr>
<td>Graded Aggregate Base</td>
<td>-</td>
<td>Graded Aggregate Base</td>
<td>-</td>
</tr>
<tr>
<td>- 12&quot; Depth</td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

34. The Contractor shall revise and submit for GDOT’s review and acceptance a revised Wall Foundation Investigation report based on GDOT’s Office of Materials and Research comments dated January 22, 2010. The wall parameters included in this Special Provision are provided for information only.

35. The proposed typical section dimensions for the I-575 Auxiliary Lane normal shoulder condition shall be as follows: 12 feet (10 feet paved, 2 feet grassed), a minimum of 16 feet foreslope with a 4 foot wide ditch.

2. Proposal Page 560, Special Provision Section 999 Design-Build, Subsection 999.1.B; Revise the first paragraph as follows:

“Bids on this project shall reflect close conformity to designing and constructing the project as shown in the Scope (999.1.C) and applicable portions of the Costing Plans and specification package. No design exceptions and no design variances shall be assumed by the Contractor. However, alternative proposals on portions of the work will be entertained once the project is awarded. The VE Specification, including procedures, in which the Contractor and Department have a share in the savings, can be utilized as a part of this project. Alternative proposals shall not conflict with the overall completion date of the project or intermediate completion dates unless otherwise approved by the Department.”

3. Add the attached Cherokee County Utility Memorandum of Understanding (MOU) to the proposal.
AMENDMENT TO ADVERTISED CONTRACT (continued):

4. Proposal Page 561, Special Provision Section 999 Design-Build, Subsection 999.1.C.10; Revise to read as follows:

“A Value Engineering Study was performed for the project and those recommendations and approved items for implementation are to be adhered to in the final design/construction of the project. The typical Bridge section and length shown in the Bridge layout provided is consistent with the VE Response noted under “New Alternate” in Alternate D-3A section of the VE Study Implementation Letter, and is consistent with what the Department intends to be constructed.”

5. Proposal Page 562, Special Provision Section 999 Design-Build, Subsection 999.1.C.20.c; Revise to read as follows:

“Ridgewalk Parkway 45 mph”

6. Proposal Page 584, Special Provision Section 999 Design-Build, Subsection 999.3.B.1.s; Add the following:

“As contained within the Department’s standard ESPCP General Notes (dated 8-26-2008 or more current), the Contractor shall remove all references to the following statement: “The Erosion Sedimentation and Pollution Control Plan (ESPCP) is provided by the Department.””

DAVID E. HOGE
STATE TRANSPORTATION OFFICE ENGINEER
MEMORANDUM OF UNDERSTANDING

between the
Georgia Department of Transportation (hereafter the DEPARTMENT)
and
Cherokee Co. Water & Sewer (hereafter the OWNER)

Whereas the DEPARTMENT proposes to undertake a design-build project to construct a new interchange on I-575 @ Rope Mill Connector within the existing right of way, in the City of Woodstock, Cherokee County, Georgia by contract through competitive bidding procedures; and,

Whereas the DEPARTMENT will accomplish the design-build project through a Design Consultant. Design Consultant Team and/or Contractor hereafter referred to as CONTRACTOR; and,

Whereas the OWNER has the following utility facilities which may need to be adjusted or relocated as a result of the proposed contract:

Type of facility or facilities of the OWNER:

- [ ] Domestic water mains and distribution lines and associated appurtenances.
- [X] Sanitary Sewer facilities and/or Storm Drainage System
- [ ] Electrical Distribution (overhead and underground) wires, poles, etc.
- [X] Electrical Transmission (overhead and underground) wires, poles, etc.
- [ ] Natural Gas Distribution Facilities (underground)
- [ ] Telecommunications facilities and equipment
- [ ] Cable TV facilities.
- [X] Street Lighting
- [ ] Internet Data Service

Whereas the OWNER desires the following to be installed as new, additional facilities during the proposed contract:

Insert detailed description of proposed new additional utility installations:

10" DIP along new proposed roadway near back of P/L
Design Responsibilities for adjusted, relocated, and new additional utility facilities:

\(\text{No}\) (1a) Whereas the OWNER does not have adequate staff to perform the design functions for the adjustment or relocation of its facilities and is willing to have this design work included in the contract to be let by the DEPARTMENT.

\(\text{No}\) (2a) Whereas the OWNER is willing to have this design work included in the contract to be let by the DEPARTMENT.

\(\checkmark\) (3a) Whereas the OWNER will perform its own relocation or adjustment design.

If method (1a) or (2a) for the adjustment, relocation or installation of new facilities is selected, all design necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

_____ None.

Excluded Items: ____________________________________________________________

Construction Responsibilities for adjusted, relocated, and new additional utility facilities:

\(\text{No}\) (1b) Whereas the OWNER does not have adequate equipment and staff to adjust its facilities and is willing to have this work included in the contract to be let by the DEPARTMENT.

\(\text{No}\) (2b) Whereas the OWNER is willing to have this work included in the contract to be let by the DEPARTMENT.

\(\checkmark\) (3b) Whereas the OWNER will perform its own relocations or adjustments.

If method (1b) or (2b) for the adjustment, relocation or installation of new facilities is selected, all work necessary for the adjustment or relocation of the described utilities in accordance with the plans when approved shall be included in the project contract and accomplished by the CONTRACTOR except as follows (check none or list any work items to be performed by the OWNER):

_____ None.

Excluded Items: ____________________________________________________________

472B
The following is hereby mutually agreed to and understood by both parties:

1. The identification of existing facilities including preparation of base maps plans will be accomplished by the DEPARTMENT prior to award of the project contract and thereafter supplemented by the CONTRACTOR.

2. The CONTRACTOR shall coordinate reviews of the information and obtain acceptance from the OWNER and DEPARTMENT when required. However; the OWNER shall apply for and obtain any required permits from the DEPARTMENT and perform any final design or proprietary design needed to administer its own relocation work if the work will not be included in the project contract (list any work not included in the project in space provided above). If the preliminary plans indicate that no conflict exists, and the OWNER accepts this information, the OWNER shall provide a letter of “no conflict” to the CONTRACTOR.

3. The CONTRACTOR will research the property interest of each OWNER and present the findings to the DEPARTMENT and OWNER for approval. The CONTRACTOR will coordinate resolution of any disputed items. The plans and estimate for the utility work shall be subject to approval of both the DEPARTMENT and the OWNER prior to construction. If the OWNER chooses to include the subject utility work in the project construction and the research indicates that no property interest exists, and the OWNER cannot refute this finding with evidence that would substantiate the property interest in legal proceedings, the OWNER shall provide confirmation in writing that OWNER will reimburse the DEPARTMENT for any adjustment or relocations necessary; and an agreement will be prepared and executed in accordance with the DEPARTMENT’S “Utility Accommodation Policy and Standards Manual”. If the OWNER chooses to perform it own relocations and the OWNER holds no property interest as stated above; the OWNER shall confirm in writing that the OWNER will relocate its own facilities at no cost to the DEPARTMENT and the CONTRACTOR.

4. All construction engineering and contract supervision shall be the responsibility of the DEPARTMENT and the CONTRACTOR to assure that all utility work included in the project contract is accomplished in accordance with plans and specifications and to consult with the OWNER before authorizing any changes or deviations which affect the OWNER’s facility.

5. For Utility work included in the contract, the OWNER or the OWNER’s Consultant shall have the right to visit and inspect the work at any time and advise the CONTRACTOR and the DEPARTMENT’S Engineer of any observed discrepancies or potential problems. The DEPARTMENT agrees to notify the OWNER when all utility work is completed and ready for final inspection by the OWNER.

6. Upon completion of the utility work included in the contract and upon certification by the DEPARTMENT’S Engineer that the work has been completed in accordance with the plans and specifications, the OWNER will accept the adjusted, relocated, and additional facilities and will thereafter operate and maintain said facilities located within public right of way subject to the DEPARTMENT’S “Utility Accommodation Policy and Standards, 1988 edition” and any agreements in effect without further cost to the DEPARTMENT or it’s CONTRACTOR.

7. For the purpose of utility coordination, relocation and reimbursement matters, the OWNER shall cooperate with the CONTRACTOR in the same manner as if directly with the DEPARTMENT in accordance with the
laws of Georgia, the DEPARTMENT'S "Utility Accommodation Policy and Standards, 1988 edition" and any agreements in effect between the DEPARTMENT and OWNER. The OWNER agrees to cooperate in good faith with the CONTRACTOR and to respond to all requests for information or meetings required to reach a resolution of any disputed items.

The Memorandum of Understanding will be incorporated into the project contract by reference or Exhibit.

APPROVED FOR THE OWNER BY:

[Signature]

(Date)

[Title]

APPROVED FOR THE DEPARTMENT BY:

[Signature]

(Date)

STATE UTILITIES ENGINEER
AMENDMENT TO ADVERTISED CONTRACT

CONTRACT I.D. NUMBER: B13668-10-000-0

GEORGIA PROJECT NUMBER: CSNHS-0006-00(043)

PCN: 0006043.01000

COUNTY: CHEROKEE

AMENDMENT NUMBER: 2

LETTING DATE: MARCH 19, 2010

LETTING NUMBER: 001

THE FOLLOWING CHANGES ARE HEREBY MADE TO THIS CONTRACT. THE BIDDER IS RESPONSIBLE FOR MAKING ANY NECESSARY CHANGES IN INK IN THE PROPOSAL. BIDDER SHALL ACKNOWLEDGE THIS AMENDMENT BY CHECKING THE APPROPRIATE SPACE ON THE PROPOSAL SIGNATURE PAGE.

*****************************************************************************

1. Proposal Page 561, Special Provision Section 999 Design-Build, Subsection 999.1.C.7; Revise to read as follows:

“The conceptual sound barrier locations are shown in the costing plans. The final locations of sound barrier shall be adjusted accordingly to meet site conditions, sound reduction requirements and NEPA commitments”

2. Proposal Page 562, Special Provision Section 999 Design-Build, Subsection 999.1.C.23; Revise to read as follows:

The Contractor shall install two pre-stressed concrete strain poles, and to each attach a CCTV camera with a wireless cellular communications and video connection to GDOT NaviGAtor. CCTV Installation shall include cabinet, conduit, and all necessary hardware/software for wireless connection in accordance with applicable GDOT specifications, and current design manuals. Equipment may include, but not limited to, Strain Pole TP IV; Pull Box TP 4s; Conduit, nonmetal, TP 2 - Power Service; CCTV System TP B; Video Encoder TP B; Field Switch, TP X; and Type D Cabinet. Labeling on plans shall include Station numbers and all equipment being proposed at that location. The Contractor shall coordinate and engineer power and wireless connections to these poles. All wireless equipment information and specifications can be obtained from vendors. One pole shall be located in the southeast quadrant of the Ridgewalk Pkwy/SB Ramp intersection. Another pole shall be located in the northwest quadrant of the Ridgewalk Pkwy/NB Ramp intersection.”
AMENDMENT TO ADVERTISED CONTRACT (continued):

3. Proposal Page 562, Special Provision Section 999 Design-Build, Subsection 999.1.C.13; Revise to read as follows:

“Overhead signs along Interstate 575 shall be installed in accordance with Department and MUTCD design guidelines that are current at time of letting. In addition, Interstate exit signs for the new interchange shall be installed at the 1 mile and ½ mile locations for the south bound off-ramp.

4. Proposal Page 563, Special Provision Section 999 Design-Build, Subsection 999.1.C.24; Revise to read as follows:

“6 foot chain link fence shall be installed on top of retaining walls that are adjacent to pedestrian facilities unless otherwise noted in the costing plans as handrail.”

5. Proposal Page 563, Special Provision Section 999 Design-Build, Subsection 999.1.C.25; Revise to read as follows:

“Asphaltic curb shall be installed behind guardrail when the fill height exceeds 10 feet. When asphaltic curb is installed along shoulder behind guardrail, the Contractor shall use concrete spillways with concrete flumes to drain.”

6. Proposal Page 563, Special Provision Section 999 Design-Build, Subsection 999.1.C; Add the following:

36. The Contractor shall add polymer-modified liquid AC in all SMA and OGFC asphaltic mixes.

37. The stream numbers shown in the Costing Plans are shown for information only. The Contractor is responsible for verifying, and revising stream numbers per the revised ecology addendum, as necessary.

7. Proposal Page 563, Special Provision Section 999 Design-Build, Subsection 999.1.E; Revise to read as follows:

1. The NEPA document for this project is an EA/FONSI which has been approved. The Department has prepared a draft reevaluation of the document based on the construction limits presented in the Costing Plans. The Department will update the reevaluation upon receipt of the Contractor’s preliminary plans.

2. The Contractor shall provide the preliminary plans, preliminary utility relocation plans and anticipated construction schedule to the Department as per Table 4-1. The Contractor shall provide the proposed impacts to stream and wetlands which shall include impacts that result from utility relocations, and temporary and/or permanent impacts that result from the construction of the project.

3. The Contractor shall prepare the Section 404 Nationwide Permit application to the Department’s satisfaction. The Department will transmit the Section 404 Nationwide Permit to the US Army Corps of
4. Engineers. The Contractor shall be responsible for satisfactorily addressing the US Army Corps of Engineer’s comments. It is anticipated that approximately 90 days will be required from the time the Department transmits an acceptable Section 404 Nationwide Permit application to receipt of agency approval. The Contractor is required to satisfactorily address the US Army Corp of Engineer’s comments within 14 calendar days of receipt. If any additional impacts result from the Contractor’s proposed design versus those in the Costing Plans and revised ecology addendum, then the Department will perform special studies which will require 90 additional days prior to the Department’s submittal of permit documentation to the appropriate agency.

5. The Contractor is responsible for identifying the need for any Stream Buffer Variances on this project. The Contractor shall also be responsible for the necessary design and construction needed to avoid or mitigate for the Stream Buffer(s) impact. If a Stream Buffer Variance is identified then the Contractor is responsible for notifying the Department no later than the time of the preliminary plans submittal to the Department. The Contractor shall prepare the Stream Buffer Variance application to the Department’s satisfaction. The Department will transmit the Stream Buffer Variance application to Georgia’s Environmental Protection Division. The Contractor shall be responsible for satisfactorily addressing Georgia’s Environmental Protection Division comments. It is anticipated that approximately 120 days will be required from the time the Department transmits an acceptable Stream Buffer Variance application to receipt of agency approval. The Contractor is required to satisfactorily address the Georgia Environmental Protection Division’s comments within 14 calendar days of receipt. The SBV cannot be granted prior to issuance of the Section 404 Nationwide permit.

6. Once the Department receives an approved Section 404 Nationwide Permit from the US Army Corps of Engineers, the Department will issue written notification to the Contractor that the Contractor shall then acquire all mitigation credits in the name of the Department as required under the approved permit. All mitigation credits obtained by the Contractor and applied to the project shall be approved by the US Army Corps of Engineers. Upon satisfactory receipt of the Contractor’s credit purchase, the Department will provide written authorization to work in jurisdictional Waters of the US in accordance with the permit conditions.

7. Upon adequate completion of NEPA related tasks (the reevaluation and all pre-construction commitments including, but not limited to, receipt of permits, variances, and the purchase of the mitigation credits), the Department will issue a NEPA recertification. No land disturbing activities shall take place until the Department issues the NEPA recertification.

8. Proposal Page 570, Special Provision Section 999 Design-Build, Subsection 999.3.A.3; Revise the first paragraph to read as follows:
AMENDMENT TO ADVERTISED CONTRACT (continued):


9. Proposal Page 581, Special Provision Section 999 Design-Build, Subsection 999.3.B.1.b.4; Revise to read as follows:

“Manual on Uniform Traffic Control Devices (MUTCD) by the U.S. Department of Transportation, Federal Highway Administration “FHWA” – current version at time of letting”

10. Proposal Page 584, Special Provision Section 999 Design-Build, Subsection 999.3.B.1.t; Revise the first paragraph to read as follows:

“Signing and Marking and Signalization Requirements: Prepare signing, signalization and marking plans in accordance with the Manual of Uniform Traffic Control Devices (MUTCD), and any applicable AASHTO or Department standards and guidelines that are current at time of letting. Prepare plan sheets to show all permanent roadway signs and pavement markings as they appear upon completion of the project. Place emphasis on designing clear directional signage and coordinating sign placement with roadway features, structures, sight distances and driver awareness. All signs that are impacted by the project shall be replaced and shall meet current standards.”

11. Delete Proposal Pages 474 and 475 from the proposal.

12. Add the attached Special Provision Section 107-Legal Regulations and Responsibilities to the Public, 1 page, dated March 2, 2010, to the proposal.

CSNHS-0006-00(043), PLANS

1. Plan Sheet 231, DESIGN DATA; Revise the note to read “(DESIGNED FOR SEISMIC PERFORMANCE CATEGORY B)”.

DAVID E. HOGE
STATE TRANSPORTATION OFFICE ENGINEER
December 21, 2009
March 2, 2010 Updated
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA
SPECIAL PROVISION

SECTION 107 – LEGAL REGULATIONS
AND RESPONSIBILITIES TO THE PUBLIC

CSNHS-0006-00(043) Cherokee County
P.I. No.: 0006043
I-575 @ Rope Mill Connector/Ridgewalk Pkwy. – New Interchange

The status of the parcels on which condemnations have been filed and which the Department does not presently have possession of, the status is as follows:

Subsection 107.18, “Acquisition of Right-of-Way” is modified by the addition of the following:

<table>
<thead>
<tr>
<th>Parcel No.</th>
<th>Date Filed</th>
<th>Possession Expected</th>
</tr>
</thead>
<tbody>
<tr>
<td>10</td>
<td>02/18/2010</td>
<td>*No/03/18/2010</td>
</tr>
<tr>
<td>11</td>
<td>02/18/2010</td>
<td>*No/03/18/2010</td>
</tr>
<tr>
<td>12</td>
<td>02/18/2010</td>
<td>*No/03/18/2010</td>
</tr>
<tr>
<td>19</td>
<td>02/18/2010</td>
<td>*No/03/18/2010</td>
</tr>
</tbody>
</table>

*These are anticipated dates. This project will not be awarded to construction until all parcels have been acquired and possession has been obtained in accordance with F.H.W.A. guidelines. The Contractor is to verify possession through the Department before entering upon this property for any reason.

Physical construction may proceed when authorization is granted, but the contractor will take no action that will result in unnecessary inconvenience, disproportionate injury or any action coercive in nature to occupants of residences, businesses, farms, or nonprofit organizations which have not been moved from the right of way.

Depending on construction sequencing, the lack of possession and use of the above referenced parcels could interfere with construction operations and the contractor’s ability to perform as specified.

The Contractor shall not commence work on any occupied parcel until the Department has title and possession of parcels.
AMENDMENT TO ADVERTISED CONTRACT

CONTRACT I.D. NUMBER: B13668-10-000-0
GEORGIA PROJECT NUMBER: CSNHS-0006-00(043)
PCN: 0006043.01000
COUNTY: CHEROKEE
AMENDMENT NUMBER: 3
LETTING DATE: MARCH 19, 2010
LETTING NUMBER: 001

THE FOLLOWING CHANGES ARE HEREBY MADE TO THIS CONTRACT. THE BIDDER IS RESPONSIBLE FOR MAKING ANY NECESSARY CHANGES IN INK IN THE PROPOSAL. BIDDER SHALL ACKNOWLEDGE THIS AMENDMENT BY CHECKING THE APPROPRIATE SPACE ON THE PROPOSAL SIGNATURE PAGE.

1. **Delete** Proposal Pages 478 through 486, and 572 from the proposal, and **Substitute** the attached revised/added page 572 in the proposal.

2. **Add** the following attached Special Provisions to the proposal:

   A. Section 108-Prosecution and Progress, 9 pages, dated March 18, 2010.

   B. Section 109-Measurement and Payment, 3 pages, with a first use date of March 19, 2010.

DAVID E. HOGE
STATE TRANSPORTATION OFFICE ENGINEER
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

SPECIAL PROVISION

PROJECT NO. CSNHS-0006-00 (043) CHEROKEE COUNTY
P. I. NO. 0006043

Section 108.03—Prosecution and Progress

Delete section 108.03 and substitute the following:

A. General Requirements

The Contractor shall provide a project schedule using the critical path method (CPM).

The schedule shall show the various activities of work in sufficient detail to demonstrate a reasonable and workable plan to complete the work in the specified contract time.

The Contractor shall be responsible for ensuring that all work sequences are logical and that the schedule indicates a coordinated plan. The schedule shall indicate the order and interdependence of activities and the sequence for accomplishing the work.

The schedule shall include sufficient detail to allow the Engineer to readily identify the work and evaluate the progress of each activity. The schedule shall include activities for all work to be performed by the Contractor, subcontractors and suppliers. The schedule shall also include activities specific to the project to be performed by the Department, other governmental bodies, regulatory agencies, utilities and any others necessary to complete the work. The schedule shall reflect the scope of work, design, construction phasing, maintenance of traffic requirements, environmental requirements, utility and railroad coordination, coordination with other contractors, intermediate completion dates, the contract completion date, and any other work or milestones established in the contract. The schedule shall include activities for working and shop drawing preparation, Contractor submittals, submittal review time by the Department, material procurement and fabrication, and the delivery of materials, and equipment and other similar activities.

Failure by the Contractor to include any element of work or to accurately reflect the relationships among the work activities required for performance of the Contract does not excuse the Contractor from completing all work within the specified time at no additional time or cost to the Department, notwithstanding the acceptance of the schedule by the Department.

The Contractor shall provide sufficient materials, equipment, and labor to guarantee the completion of the Project in accordance with the Plans and Specifications within the time set forth in the proposal and in strict conformance with the CPM of record. The CPM of record shall be the Contractor’s work plan for completing the entire Contract as specified in the Contract Documents.

At least 48 hours before commencing The Work, the Contractor shall notify the Engineer of his intention to begin so that proper inspection may be provided. Should the prosecution of The Work be discontinued for any reason, the Contractor shall notify the Engineer at least 24 hours in advance of resuming operations.
B. Definitions

The following definitions shall be applied to the terms used in this specification and shall not be taken to modify in any way the definitions in 101.01 through 101.83.

**Activity** - A discrete, identifiable task or operation that takes time, has a definable start and stop date, furthers the work’s progress, and can be used to plan, schedule, and monitor a project.

**Activity Calendar** - A set of days assigned to a specific activity on which work for the activity may be scheduled.

**Activity Calendar Day** - A day on which work is scheduled to be performed on a specific activity.

**Activity Identification (ID) Number** - A unique, alphanumeric, identification code assigned to a specific activity.

**Activity Network Diagram** - A graphic representation of a CPM schedule, including a timescale, which shows the relationships among activities.

**As-built Schedule** - The last monthly update schedule depicting the actual start and finish dates for each activity.

**Bar Chart** - A graphic representation of a schedule without relationship lines displayed. A timescale appears along the horizontal axis.

**Baseline Schedule** - A baseline CPM schedule showing the original plan to complete the entire project.

**Calendar Day** - As defined in 101.12.

**Calendar ID** - An alphanumeric identification code assigned to an activity calendar.

**Constraint** - A restriction imposed on the start or finish dates of an activity that modifies or overrides the activity’s logic relationships.

**Contract Completion Date** - The original date or revised date specified in the contract for completion of the project.

**Controlling Activity** - Activity or work item having the least amount of float, which if not completed as scheduled would result in delaying the scheduled completion date of the project.

**CPM of Record** - A baseline, monthly update, or a revised schedule accepted by the Department

**Critical Activity** - Any activity on the critical path.

**Critical Path** - The longest path of activities which determines the scheduled completion date of the project.

**Data Date** - The first day in an Initial or a Baseline Schedule and the first day for performance of the work remaining in a Monthly Update Schedule or a Revised Schedule.

**Free Float** - The amount of time an activity can be delayed and not delay a successor.

**Initial Schedule** - A schedule showing the original plan for the first 60 calendar days of work.

**Intermediate Completion Date** - A date specified in the contract for completion of an interim milestone, phase or other portion of the project.

**Milestone** - An activity with no duration that is typically used to represent the beginning or end of the project or an interim phase. Includes, but is not limited to, intermediate completion dates and the contract completion date.

**Monthly Update Schedule** - A schedule produced by incorporating the project’s actual progress into the baseline schedule or a revised schedule.

**Open End** - The condition that exists when an activity has either no predecessor or no successor, or when an activity’s only predecessor relationship is a finish-to-finish or only successor relationship is a start-to-start.

**Original Duration** - The estimated time, expressed in activity calendar days, required to perform an activity.

**Predecessor** - An activity that is defined by schedule logic to precede another activity. A predecessor may control the start or finish date of its successor.

**Relationship** - The interdependence among activities. Relationships link an activity to predecessors and successors.

**Remaining Duration** - The estimated time, expressed in activity calendar days, required to complete an activity.
Revised Schedule – A revised schedule that includes a significant modification to the schedule’s logic or activity durations.

Schedule – A Critical Path Method (CPM) schedule showing activities organized by relationships to depict the plan for execution of a project.

Scheduled Completion Date – The completion date forecast by the CPM schedule. The schedule may also forecast intermediate completion dates for milestones, phases, or other portions of the project.

Successor – An activity that is defined by schedule logic to succeed another activity. The start or finish date of a successor may be controlled by its predecessor.

Total Float – The amount of time an activity can be delayed and not delay the contract completion date.

C. Submissions

1. Initial Schedule

The Contractor shall submit an initial schedule to the Engineer for acceptance no later than the day of the pre-construction conference. The submittal shall include a detailed schedule for the first 60 calendar days of work in accordance with 108.03.D and a summary of activities for the balance of the contract. The schedule shall show milestones for intermediate and contract completion dates no later than those specified in the contract. All specified closure or restriction periods, non-work periods or any other time restrictions in the contract shall be addressed in the initial schedule. No work within the project limits shall be performed until the initial schedule is submitted.

The Engineer will provide notice within 14 calendar days after receipt of the initial schedule that the schedule is either accepted or rejected or that additional information is requested. If the schedule is rejected, the Engineer will identify the reasons for rejection or what additional information is requested, and within 7 calendar days, the Contractor shall submit a corrected initial schedule that addresses the reasons for rejection and provides any additional information requested.

2. Baseline, Monthly Update and Revised Schedules

The Contractor shall submit a baseline schedule in accordance with 108.03.D no later than 45 calendar days after Notice to Proceed.

If a baseline schedule is not submitted within the allotted time, the Engineer may withhold all Contract payments until a baseline schedule is submitted. When the Engineer notifies the Contractor that the baseline schedule has been accepted, it will become the CPM of record.

The Contractor shall submit a monthly update to the baseline schedule in accordance with 108.03.D no later than 7 calendar days after the end of each month during the contract beginning with the month after approval of the baseline schedule. The update schedule shall depict the status of the project at the end of the month just completed. The update schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work. If a monthly update schedule submittal is more than 30 days past due, the Engineer may withhold pay estimates until monthly update schedule submittals are current. When the Engineer notifies the Contractor that the update to the baseline schedule has been accepted, it will become the CPM of record for the period between its data date and the data date of the next accepted update or revision.

The Contractor shall submit a revised schedule when any of the following occur:

(a) The critical path is more than 7 calendar days behind schedule.

(b) There is a significant difference, as determined by the Engineer, between the scheduled and actual work sequence.

The Engineer will notify the Contractor in writing when a revised schedule is required due to any of the reasons listed above. The Contractor shall submit a revised schedule in accordance with 108.03.D no later than 14 calendar days after notification by the Engineer. The revised schedule shall depict the status of the project on the date of the Engineer’s notification for a revised schedule. The schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work.

The Engineer may request a revised schedule due to changes in the work. The Engineer will notify the Contractor in writing when a revised schedule is requested. Changes that may initiate a request for a revised schedule include, but are not limited to:
(a) An increase or decrease in the amount of work due to extra work or force account work.

(b) An approved increase or decrease in contract time.

(c) A change in the phasing or contract time of the project.

When a revised schedule is requested by the Engineer, the Contractor shall submit a revised schedule in accordance with 108.03.D no later than 14 calendar days after notification by the Engineer. The revised schedule shall depict the status of the project on the date of the Engineer’s request for a revised schedule. The schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work.

If a revised schedule is not submitted within 30 days of written notification, the Engineer may withhold all Contract payments until the revised schedule is submitted.

The Contractor may request to submit a revised schedule to reflect changes in the plan to perform the work. When a revised schedule is proposed by the Contractor, the Contractor shall provide the Engineer with a written notice 7 calendar days prior to submitting the revised schedule. The notice shall include a description of the changes to be made and the reason for the changes. The revised schedule shall be submitted in accordance with 108.03.D and shall depict the status of the project on the date of the Contractor’s written notice to the Engineer. The schedule shall reflect a new data date, work performed up to, but not including the new data date, and the plan for completing the remaining work.

A revised schedule submitted at the request of the Contractor will not be considered to be the CPM of record until accepted by the Engineer.

3. As-Built Schedule

The Contractor shall submit an as-built schedule in accordance with 108.03.D no later than 30 calendar days after completion of contract work. The as-built schedule shall depict the actual start and finish dates for each activity.

The as-built schedule shall contain a written certificate signed by the Contractor’s project manager stating “To my knowledge, the enclosed as-built schedule reflects the actual start and finish dates of the actual activities for the project contained herein.”

4. Acceptance of Schedules

The Engineer will provide written notice within 14 calendar days of receipt of a baseline, monthly update, revised or as-built schedule that the schedule is either accepted or rejected or that additional information is requested. The Engineer’s written notice will identify the reasons for rejection or what additional information is requested. Within 14 calendar days of the Engineer’s written notice, the Contractor shall submit a corrected baseline, monthly update, revised or as-built schedule that addresses the reasons for rejection and provides any additional information requested. If the Engineer does not respond in writing to the submittal of a schedule within the allotted time, the schedule shall be considered to be accepted.

The Engineer’s review of a submitted schedule will be for compliance with the specifications. Acceptance by the Engineer does not relieve the Contractor of responsibility for the accuracy or feasibility of the schedule. Acceptance of a schedule does not constitute a modification of the contract or endorsement or validation of the Contractor’s logic, activity durations, or assumptions in creating the schedule. Acceptance of a schedule does not guarantee that the project can be performed or completed as scheduled. Omissions and errors in a schedule shall be corrected by submittal of a revised schedule and will not entitle the Contractor to additional compensation or contract time.

5. Float and Scheduled Completion

Total float belongs to the project and shall be a shared commodity between the Contractor and the Department. Float is not for the exclusive use or benefit of either party. Either party has full use of float until it is depleted.

Float generated during the course of the project due to the efficiencies of either party shall be considered to be part of total float.

Negative float generated during the course of the project will not be a basis for requesting time extensions. Time extension requests shall be developed in accordance with Section 108.07.E.

A schedule may indicate a scheduled completion date in advance of the contract completion date. However, the Department shall not be liable in any way for the Contractor’s failure to complete the project prior to the contract completion date. Any costs incurred by the Contractor as a result of such a failure shall be the responsibility of the Contractor. The Contractor will not be entitled to claim or recover any such cost from the Department.
An accepted schedule that indicates a scheduled completion date in advance of the contract completion date will be considered to have total float equal to the time between the scheduled completion date and the contract completion date.

When a contract has an incentive/disincentive date for all or a portion of the work, and the Contractor elects to show a completion date in advance of the contract completion date, the Contractor shall show an activity in the schedule for accruing incentive. The duration of this activity shall be the length of time, in calendar days, between the Contractor’s scheduled completion date and the contract completion date. The Contractor may assign the activity a duration up to and including the number of days necessary to earn the specified maximum incentive. Any time between a scheduled completion date earlier than the maximum incentive date and the maximum incentive date will be considered to be part of total float.

D. Schedule Technical Requirements

The schedule shall be generated using software completely compatible with MS Project 2007 or later version.

The schedule shall at a minimum include codes for project phase, location of work and responsibility for each activity. Additional coding fields may be used if approved by the Engineer.

Each schedule submittal shall include the following:

1. A letter of transmittal identifying the schedule submission.

2. A narrative report in accordance with 108.03.E

3. A compact disc (CD) containing an electronic file of the schedule in MS Project 2007 or later version format that is completely compatible with and may be directly imported into MS Project 2007 and later version without any loss or modification of data or need for any conversion or other software. Any electronic schedule file submitted by the Contractor that is not completely compatible with the Department’s MS Project 2007 or later version software will be rejected.

4. The critical path in a bar chart format created using the longest path sort, grouped (banded) by phase and sorted by early start dates.

5. A full color plot of the activity network diagram on 11 x 17 or 22 x 34 paper in landscape layout in a time scale suitable to provide sufficient detail. Other formats may be accepted with prior approval by the Engineer. Arrows shall be used to show the relationships between activities on the activity network diagram. The critical path and critical activities shall be plotted in red.

6. Each page of the schedule plot shall at a minimum include a column for each of the following:
   a. Activity ID
   b. Activity description
   c. Original duration
   d. Remaining duration
   e. Early start
   f. Early finish
   g. Late start
   h. Late finish
   i. Total Float
   j. Percent Complete

7. Each page of the schedule plot shall include a legend with the following information:
   a. Contract number
   b. District
   c. Name of the Contractor

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d. Project location

e. Notice to Proceed date

f. Original contract completion date

g. Revised contract completion date (as applicable)

h. Data date

8. A predecessor/successor report with the following information for each activity:

a. Activity ID and description

b. Original duration

c. Remaining duration

d. Calendar ID

e. Early start date

f. Early finish date

g. Late start date

h. Late finish date

i. Total float

j. Relationship type

k. Lags

l. Constraints

Each activity shown on the CPM schedule shall meet the following requirements:

1. Each activity shall have a unique activity identification number which shall not be modified or re-assigned to different work activities once assigned to an activity in the network logic diagram.

2. Each activity shall have a unique description specifying the work type and location.

3. Each activity description shall describe work associated with only one operation.

4. All non-procurement activities shall have an original duration not to exceed 20 calendar days unless approved by the Engineer.

5. Activities for submittal reviews by the Department shall be 30 calendars days unless stated otherwise in the contract.

6. Each activity, except the first activity, shall have at least one predecessor.

7. Each activity, except the last activity, shall have at least one successor.

8. Activity relationships shall be finish-to-start, finish-to-finish or start-to-start with no leads or lags.

9. The use of lags with a negative value shall not be allowed on any activity relationship type.

10. All activities shall have their start and finish tied to the logic of the schedule.

11. Activities shall not have constraints unless approved by the Engineer, with the exception of, when used for “Project Start”, and “Project Completion”.

12. Each activity shall have an activity calendar identified. A project may have more than one activity calendar, but only one activity calendar shall be applicable to each activity.

13. Seasonal weather conditions shall be considered and included in the planning of all work.

The CPM schedule shall only be calculated using retained logic. Use of Progress Override option will not be allowed. Schedule duration shall be contiguous. Total float shall be calculated based on finish dates.
E. Narrative Reports

The Contractor shall submit a narrative report with each schedule submission pertaining to the work identified in the schedule. For the initial and baseline schedule submittals, the narrative report shall include the following:

1. An explanation of the overall plan to complete the project, including where the work will begin and how the work and crews will progress through the project.

2. An explanation of the use and application of the workdays per week, number of shifts per day, number of hours per shift, holidays observed and how the schedule accommodates adverse weather days for each month.

3. Description of the work to be completed each season for multi-year projects.

4. A description of any problems or issues.

5. A description of any delays, including identification of any delayed activities, the type of delay, the cause of the delay, the effect of the delay on other activities and project milestones and identification of actions required to mitigate the delay.

6. A description of the critical path.

7. An explanation of the use of any allowed constraints, including the reason and purpose for each constraint.

8. A statement describing the status of any required permits.

9. A statement describing the reason for the use of each lag.

For each monthly update and revised schedule submittal, the narrative report shall include the following:

1. A description of the status of the scheduled completion date, focusing on any changes since the previous submission.

2. An explanation if the scheduled completion date is projected to occur after the contract completion date.

3. An explanation if any contract milestone dates are projected to occur after the dates set out in the contract.

4. A description of the work performed since the last schedule update.

5. A description of unusual labor, shift, equipment or material conditions or restrictions encountered.

6. A description of any problems encountered or anticipated since the last schedule update.

7. A statement that identifies any current and anticipated delays. A discussion of delays in the narrative report does not constitute notice in accordance with 105.13.B.9. The statement should include identification of the delayed activity, the type of delay, the cause of the delay, the effect of the delay on other activities and project milestones and identification of actions required to mitigate the delay.

8. A description of the critical path.

9. A description of changes in the critical path and any contract milestone dates from the last schedule update.

10. A report that identifies the changes made between the previous schedule update and the current proposed schedule, including, but not limited to:
   a. Data date
   b. Completion date
   c. Activity code assignments
   d. Schedule calculation options
   e. Activity descriptions
   f. Added activities
   g. Deleted activities
   h. Added activity relationships
i. Deleted activity relationships
j. Activity original durations
k. Activity remaining durations
l. Activity actual start and finishes
m. Percent complete
n. Constraints
o. Activity coding for phase, location and assigned contractor

11. A statement providing status of pending items, including, but not limited to:
   a. Permits
   b. Change orders
   c. Time extension requests

**F. Extension of Contract Time**

Any request for an extension of time made pursuant to Specification 108.07.E shall include the contractor's own analysis, using a method approved by the Engineer, of the requested time using the CPM of Record and as-built information of work actually performed. This information must be submitted in addition to any information required by Specification 108.07.E to allow the Department to perform an analysis by comparing the CPM of Record at the time with as-built information of the work actually performed.
G. Measurement
Initial schedule, baseline schedule, as-built schedule, monthly update schedules, and revised schedules will not be measured separately for payment.

H. Payment
All costs necessary to provide initial, baseline and as-built CPM schedules, including the cost of the project scheduler, shall be included in the cost of the pay item for CONSTRUCTION COMPLETE.

All costs necessary to provide monthly update CPM schedules shall be included in the cost of the pay item for CONSTRUCTION COMPLETE.

All costs necessary to provide revised CPM schedules requested by the Engineer shall be included in the cost of the pay item for CONSTRUCTION COMPLETE.
DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA
Special Provision

PROJECT NO. CSNHS-0006-00 (043) CHEROKEE COUNTY
P. I. NO. 0006043

Section 109—MEASUREMENT AND PAYMENT

Add the following:

109.11 Price Adjustments

A. For Road Construction:

Asphalt price adjustments will be made on payments due the Contractor for certain roadway contract items.

1. “Monthly Asphalt Cement Price”: The Department will determine the “Monthly Asphalt Cement Price” based on the following formulas:

   \[ \text{Monthly Asphalt Cement Price} = \text{100\% Georgia Base Asphalt Price} \]

   Where;

   \[ \text{GBAP} = \text{“Georgia Base Asphalt Price”}, \text{ (in dollars/ton)} \]

   is based on the arithmetic average posted price of PG asphalt cement as specified in Section 820, from the Department’s monthly survey obtained from approved asphalt cement suppliers of bituminous materials to the Department projects F.O.B. the suppliers terminal. However, the highest price and the lowest price are excluded from the calculation of price, GBAP.

2. “Asphalt Cement Quantity Calculation”: The calculation of asphalt cement quantity for each mix type will be based on the asphalt cement content \( AC\% \) of the approved Job Mix Formula (JMF) as specified in Subsection 400.1.03.C. The following calculation formula will be used to determine asphalt cement quantity:
Asphalt Cement Quantity = Hot Mix Asphalitic Concrete monthly total in tons (megagrams) per mix type certified for the payment x \( AC \) (%) 

The Total Monthly Tonnage (TMT) of asphalt cement computed by the Engineer will be calculated as follows:

\[ TMT = \text{Sum of all asphalt cement quantities, including polymer modified asphalt binder and non-modified asphalt cement, based on the Hot Mix Asphalitic Concrete of the various mix types per ton (megagram)/ Sum of all asphalt cement quantities used as bituminous tack coat converted from gallons to tons (megagrams)/ Sum of all asphalt cement quantities used for bituminous surface treatment (total gallons of asphalt emulsion used, as measured from distributors, will be multiplied by a factor of 0.65 to determine the quantity in gallons of asphalt cement used) converted from gallons to tons (megagrams) by the Engineer certified for payment.} \]

Asphalt Cement Price for the Month (APM) will be adjusted monthly. Price adjustments (PA) will be made monthly and all calculations for Price Adjustments shall be performed by the Engineer as specified in SOP-39 “Determination of Asphalt Cement Index and Asphalt Cement Price Adjustment”.

\[ FPA = \left\{ \left( \frac{(FPM - FPL)}{FPL} \right) \cdot 0.10 \right\} \times (QF/1000) \times F \}

\[ FPL \]

B. **Price Adjustment Trigger:** No price adjustment will be made on any project with less than 366 Calendar Days from the Contract Letting Date to the specified completion date. If the original Contract contains 366 Calendar Days or more, the Price Adjustment shall be made on quantities placed from the Contract Letting Date to the specified completion date. A fuel price adjustment shall not be made until the FPM is greater than 10% above or below the FPL. An asphalt cement price adjustment shall not be made until the APM is greater than 5% above or below the APL.

C. “Monthly Asphalt Price”: The Department will publish a “Monthly Asphalt Cement Price” based on the formula contained within this specification.

D. “Other Restrictions”:

1. No asphalt cement price adjustment will be made for cut-back, and emulsified asphalt when used for bituminous tack coat with Hot Mix Asphalitic Concrete Construction.

2. There is a cap of 125% above the FPL/APL for any price adjustment.

3. Unless specifically provided for by Supplemental Agreement or Contract Amendment, no positive Price Adjustments for Asphalt Cement resulting in a payment to the Contractor will be made after the original Contract Time has expired. Irrespective of any other provisions in the Contract, for purposes of this specification, “Contract Time” does not include any time extensions or
Supplemental Agreements which affect the completion of the Contract. Negative Price Adjustments for Asphalt Cement for any work placed after the original Contract Time expires resulting in a return of funds to the Department will be made and shall be computed based on the Monthly Asphalt Cement Price at the time the Contract Time has expired or the Monthly Asphalt Cement Price at the time the Contract was let, whichever is less.

E. **Final Adjustment:** If there are differences between the final audited quantities and the sum of the quantities used to determine the asphalt cement adjustment, the Engineer will make a pro-rated increase or decrease in the price adjustment.

Payment for Fuel Price Adjustment will be made under:

<table>
<thead>
<tr>
<th>Item No. 109</th>
<th>Price Adjustment - Asphalt Cement</th>
<th>$ (+/-)</th>
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<tbody>
<tr>
<td>Submittal Description</td>
<td>Format</td>
<td>Quantity</td>
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<tr>
<td>Basis of Design</td>
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</tr>
<tr>
<td>Schedule – including review times</td>
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<tr>
<td>QC/QA Plan</td>
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<tr>
<td>Worksite Control Supervisor</td>
<td>HC, PDF</td>
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<tr>
<td>Qualifications</td>
<td>HC, PDF</td>
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<tr>
<td>Traffic Control Supervisor</td>
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<td>Qualifications</td>
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<tr>
<td>Construction Traffic Control Plan</td>
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<tr>
<td>Traffic Management Plan</td>
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<tr>
<td>Preliminary Plans (including all roadway plan components, erosion control plans, signing and marking, ITS, and signal plans)</td>
<td>HS, PDF</td>
<td>6,1</td>
</tr>
<tr>
<td>Final Plans (including all roadway plan components, erosion control plans, signing and marking, ITS, and signal plans)</td>
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<td>3,1</td>
</tr>
<tr>
<td>Notice of Intent (NOI) with final/signed Erosion Control Plans</td>
<td>HS, PDF</td>
<td>3,1</td>
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</table>

Comment: The Department will submit NOI Package to EPD. The Contractor shall be responsible for addressing any plan changes requested by EPD to EPD’s satisfaction.